

# **Transportation Impact Study**

# Liberty Cannabis Dispensary

304 Somerville Avenue Somerville, MA

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# **TABLE OF CONTENTS**

INTRODUCTION	
Project Description	
STUDY METHODOLOGY	3
STUDY AREA INTERSECTIONS	3
EXISTING CONDITIONS	4
ROADWAY NETWORK	4
STUDY AREA INTERSECTIONS	5
Public Transportation	6
FIELD REVIEW	8
EXISTING CONDITION VOLUMES	
CRASH SUMMARY	18
CURRENT-YEAR BUILD CONDITIONS	19
SITE-GENERATED TRAFFIC	19
PROJECT TRIP DISTRIBUTION AND ASSIGNMENT	21
2022 BUILD VOLUMES	22
FUTURE YEAR CONDITIONS	30
FUTURE ROADWAY IMPROVEMENTS	30
BACKGROUND TRAFFIC GROWTH	30
2027 DESIGN YEAR BUILD TRAFFIC VOLUMES	30
TRANSPORATION OPERATIONS ANALYSIS	35
VEHICULAR CAPACITY ANALYSIS	35
PEDESTRIAN ANALYSIS	37
BICYCLE ANALYSIS	39
Transit Analysis	39
SITE ACCESS AND CIRCULATION	41
SIGHT DISTANCE	41
PROJECT MITIGATION	41
TRANSPORTATION DEMAND MANAGEMENT	42
CONCLUSIONS	43

# **LIST OF TABLES**

Table 1: Bus Route Frequency Summary	8
Table 2: Automatic Traffic Recorder Data	10
Table 3: Mode Share Summary	11
Table 4: Estimated ITE Trip Generation	19
Table 5: Delivery Schedule	20
Table 6: Estimated Change in Trips	21
Table 7: Estimated Change in Trips	21
Table 8: Signalized Intersection Capacity Analysis	36
Table 9: Unsignalized/Pedestrian Crossing Intersections	36
Table 10: Signalized Crosswalks	38
Table 11: Sight Distance Requirements	41
LIST OF FIGURES	2
Figure 1: Site Location Map	
Figure 2: Existing Transit Facilities	
Figure 3: 2022 Existing Weekday Morning Peak Hour Vehicle Volumes Figure 4: 2022 Existing Weekday Afternoon Peak Hour Vehicle Volumes	
Figure 4: 2022 Existing Weekday Afternoon Peak Hour Vehicle VolumesFigure 5: 2022 Existing Saturday Midday Peak Hour Vehicle Volumes	
Figure 5: 2022 Existing Saturday Midday Peak Hour Vehicle Volumes	
Figure 5: 2022 Existing Weekday Morning Peak Hour Pedestrian & Bicycle Volumes Figure 7: 2022 Existing Weekday Afternoon Peak Hour Pedestrian & Bicycle Volumes	
Figure 8: 2022 Existing Weekday Afternoon Feak Hour Pedestrian & Bicycle Volumes	
Figure 9: Directions of Arrivals and Departures	
Figure 10: Weekday Morning Peak Hour New Project Vehicle Trips	
Figure 11: Weekday Afternoon Peak Hour New Project Vehicle Trips	
Figure 12: Saturday Midday Peak Hour New Project Vehicle Trips	
Figure 13: 2022 Build Weekday Morning Peak Hour Vehicle Volumes	
Figure 14: 2022 Build Weekday Afternoon Peak Hour Vehicle Volumes	
Figure 15: 2022 Build Saturday Midday Peak Hour Vehicle Volumes	
Figure 16: 2027 Build Weekday Morning Peak Hour Traffic Volumes	
Figure 17: 2027 Build Weekday Afternoon Peak Hour Volumes	
Figure 18: 2027 Build Saturday Midday Peak Hour Volumes	
Figure 19: Pedestrian & Bicycle Levels of Traffic Stress	

# **LIST OF APPENDICES**

Appendix A: Traffic Count Data
Appendix B: Traffic Projection Model

Appendix C: Crash Data

Appendix D: Journey-to-Work Data

Appendix E: 2022 Existing Capacity/Level-of-Service Analysis Appendix F: 2022 Build Capacity/Level-of-Service Analysis

Appendix G: 2027 Design Year Build Capacity/Level-of-Service Analysis

Appendix H: Capacity/Level-of-Service Analysis Summary

#### INTRODUCTION

McMahon Associates has completed a review of the existing transportation operations and potential impacts associated with the proposed conversion of the Liberty Cannabis dispensary located at 304 Somerville Avenue from medical-only to adult retail sales. The purpose of this impact study is to evaluate existing and projected transportation operations in the area surrounding the project site.

The assessment documented in this traffic impact study is based on a review of existing traffic volumes and the anticipated traffic generating characteristics of the proposed project. The study examines existing and projected traffic operations at key intersections in the vicinity of the project site. The study area was selected based on a review of the surrounding roadway network and estimated trip generating characteristics of the proposed project. This study provides a detailed analysis of traffic operations during the weekday morning, weekday afternoon, and Saturday midday peak hours when the combination of adjacent roadway volumes and project trips would be expected to be the greatest.

Based on the analysis presented in this study, the traffic projected to be generated by the proposed redevelopment is shown to have a negligible effect on the area roadways and intersections. The following report documents these findings.

# **Project Description**

The project site is located at 304 Somerville Avenue in Union Square, Somerville, Massachusetts. As shown in Figure 1, the site is bounded by Somerville Avenue to the north and existing buildings to the south, east, and west. The site is currently a medical cannabis dispensary operating as an appointment-only facility with five points of sale. The site is primarily accessed via Somerville Avenue with a small driveway located just west of the building. The driveway provides access to a small space behind a security gate that is used for parking and deliveries.

The proposed project would convert the existing medical-only dispensary to an adult retail dispensary with an additional five points of sale. The building would not be changed, and the site would continue to operate on an appointment-only basis. Access to the site would not be changed as a result of the proposed project. Four bicycle parking spaces would be added — two behind the security gate for employees and two in front of the security gate for customers.

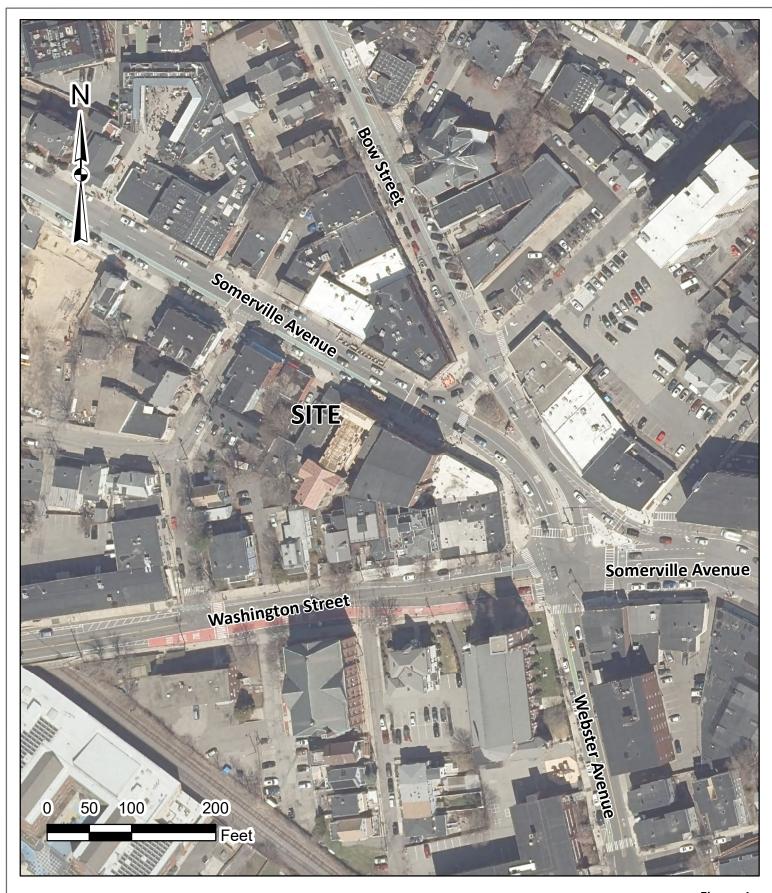




Figure 1 Site Location Map Liberty Cannabis Somerville, Massachusetts

# Study Methodology

This transportation impact study evaluates existing and projected transportation operations within the study area for the weekday morning, weekday afternoon, and Saturday midday peak hour conditions, when the combination of the adjacent roadway volumes and estimated project trips would be expected to be the greatest.

The study was conducted in three steps. The first step consisted of an inventory of existing transportation conditions within the project study area. As part of this inventory, turning movement counts (TMCs) and automatic traffic recorder (ATR) counts were collected at key locations, confirmed by the City of Somerville's Mobility Division. A field visit was completed to document intersection and roadway geometries and inventory existing pedestrian, bicycle, and public transportation infrastructure.

The second step of the study built upon the data collected in the first step of the study to establish the basis for evaluating potential impacts associated with the projected build conditions, under Existing and Design Year conditions. During this second step, the projected traffic demands associated with the proposed adult retail sales dispensary and other planned future developments that could influence traffic volumes at the study area intersections were assessed. Consistent with City of Somerville transportation impact study guidelines, the 2022 Build traffic volumes were forecasted to the future year 2027 to establish 2027 Design Year Build conditions.

The third step of this study determined if measures were necessary to improve future traffic operations and safety, minimize potential traffic impacts, and provide safe and efficient access to the proposed project site.

### **Study Area Intersections**

Based on a review of the anticipated traffic generating characteristics of the proposed project and a review of the adjacent roadways serving the project site, the following study area intersections were selected for analysis:

- Somerville Avenue at Bow Street/Webster Avenue & Washington Street
- Bow Street at Warren Avenue
- Somerville Avenue at Warren Avenue
- Somerville Avenue at Hawkins Street

This report documents existing and future traffic conditions for the study area intersections noted above.

#### **EXISTING CONDITIONS**

An accurate assessment of the potential transportation impacts associated with the proposed redevelopment requires a comprehensive understanding of the existing conditions within the project study area. The existing conditions assessment included in this study consists of an inventory of intersection and roadway geometries, pedestrian and bicycle infrastructure, public transportation infrastructure, and the collection of traffic volumes. The existing conditions in the vicinity of the project site are summarized below. Additional information on the pedestrian and bicycle conditions is provided in the pedestrian and bicycle levels of traffic stress sections (PLTS and BLTS, respectively).

# Roadway Network

### Somerville Avenue

Somerville Avenue generally extends in a northwest-southeast direction through the City of Somerville and is classified as an urban principal arterial under City of Somerville jurisdiction. For the purposes of the analysis contained within this report, Somerville Avenue is described as an east/west roadway. Within the study area, Somerville Avenue is one-way eastbound and provides access to a mix of commercial and residential land uses. In the vicinity of the project site, Somerville Avenue provides one 11-foot-wide travel lane and one 12-foot-wide travel lane. Parking is provided on both sides of the roadway, and a five-foot-wide bike lane extends along the southern side of the roadway between the parking lane and travel lanes. The sidewalk on the north side of the roadway measures approximately 12 feet wide with an effective width of approximately six feet. On the south side of the roadway, the sidewalk measures approximately 14 feet wide with an effective width of approximately five feet. Both sidewalks are in good condition. No speed limits are posted on Somerville Avenue in the vicinity of the site. As such, the City of Somerville prevailing speed limit of 25 miles per hour (mph) would apply.

#### **Bow Street**

Bow Street is an urban minor arterial under City of Somerville jurisdiction which for the purposes of this report is considered an east/west roadway. Bow Street is one-way westbound, forming a one-way pair with Somerville Avenue on the western side of Union Square, and provides access to both commercial and residential land uses. In the vicinity of its intersection with Somerville Avenue, Washington Street, and Webster Avenue, no on-street parking is provided. West of its intersection with Warren Avenue, parallel parking is provided on the south side of the roadway, and reverse angle parking is provided on the east side of the roadway. Near the intersection of Somerville Avenue at Bow Street/Webster Avenue & Washington Street, a sidewalk in good condition and measuring approximately 16 feet wide with an effective width of approximately eight feet runs along the east side of Bow Street. West of Warren Avenue, approximately ten-foot-wide sidewalks extend along both sides of the roadway. The sidewalks west of Warren Avenue is generally in good condition and is primarily concrete with some brick segments. A safety zone speed limit of 20 mph is posted on Bow Street just west of its intersection with Warren Avenue.

# Washington Street

Washington Street generally extends in the east-west direction and is classified as an urban minor arterial under City of Somerville jurisdiction. Approaching Union Square, Washington Street provides one travel lane in each direction, with an exclusive bus and bike lane in the eastbound direction and an on-street bike lane in the westbound direction. A raised median separates the eastbound and

westbound directions of travel. No on-street parking is provided on Washington Street in the vicinity of the study area. The sidewalk on the north side of the roadway measures approximately 13 feet with an effective width of approximately six feet. On the south side, the sidewalk measures between six and ten feet wide. The sidewalks along Washington Street are generally in good condition with some brick segments. A speed limit of 20 mph is posted on Washington Street.

### Webster Avenue

Webster Avenue runs in a generally north-south direction in the City of Somerville and is classified as an urban minor arterial under City of Somerville jurisdiction. Webster Avenue provides one approximately 11-foot-wide travel lane in each direction. Sharrows exist in the northbound direction, and a dedicated bicycle lane measuring approximately five feet wide extends along the west side of the southbound travel lane. Parking is provided on the western side of the roadway. Sidewalks on both sides of the street measure between eight and nine feet and are in good condition. A safety zone speed limit of 20 mph is posted on Webster Avenue between Washington Street and Newton Street.

#### Warren Avenue

Warren Avenue is a local road which runs in the northeast-southwest direction between Somerville Avenue and Columbus Avenue. Warren Avenue measures approximately 26 feet wide and accommodates two-way travel. In the vicinity of its intersection with Bow Street, parking is prohibited on the east side of the roadway, and metered parking exists on the west side of the roadway. Sidewalks extend along both sides of the roadway measuring approximately seven feet wide. The sidewalks are in generally good condition, though some minor cracking exists on the western sidewalk. A speed limit of 20 mph is posted on Warren Avenue in the vicinity of the study area.

#### **Study Area Intersections**

# Somerville Avenue at Bow Street/Webster Avenue and Washington Street

The intersection of Somerville Avenue at Bow Street/Webster Avenue and Washington Street is a four-leg signalized intersection under City of Somerville jurisdiction. The northbound Webster Avenue approach to the intersection consists of one general purpose lane. The southbound Somerville Avenue approach consists of one through/right-turn lane and one dedicated left-turn lane. The eastbound Washington Street approach to the intersection consists of one left-turn/through lane and one dedicated right-turn lane. The westbound Somerville Avenue approach consists of one dedicated left-turn lane, one dedicated through lane, and one channelized right-turn lane. Crosswalks exist across all four legs of the intersection as well as the westbound channelized right-turn, and all crosswalks are accompanied by pedestrian signals and curb ramps which include elements to generally comply with the Americans with Disabilities Act (ADA). Dedicated bicycle lanes continue through the intersection from the southbound Somerville Avenue approach and the westbound Somerville Avenue approach. Departing the intersection, dedicated bicycle lanes continue eastbound on Somerville Avenue, northbound on Bow Street, and southbound on Webster Avenue.

The signal at the intersection currently provides three primary phases: one for eastbound and westbound traffic, one for northbound traffic, and one for southbound traffic. The channelized westbound right-turn movement operates as an overlap during the southbound traffic phase.

Concurrent pedestrian phasing with 5-second Leading Pedestrian Intervals (LPIs) are provided for all crosswalks.

### Bow Street at Warren Avenue

The intersection of Bow Street at Warren Avenue is a four-leg intersection under City of Somerville jurisdiction. The westbound Bow Street approach consists of one general traffic lane and one dedicated bicycle lane and is under signal control. The northbound and southbound Warren Avenue approaches each consist of one general traffic lane and are under stop control. Crosswalks are provided across the westbound, northbound, and southbound approaches. The crosswalk across the Bow Street approach is accompanied by an actuated pedestrian signal, and all three crosswalks are accompanied by curb ramps which include elements to generally comply with ADA. The westbound bicycle lane continues through the intersection and along Bow Street to the west of the intersection.

# Somerville Avenue at Warren Avenue

The intersection of Somerville Avenue at Warren Avenue is a three-leg intersection under City of Somerville jurisdiction. The eastbound Somerville Avenue approach consists of one through lane and one left-turn/through lane. The Somerville Avenue approach is under pedestrian-actuated signal control and is otherwise free-flowing. Crosswalks exist across the eastern and western legs of the intersection and are accompanied by curb ramps which include elements to generally comply with ADA. A dedicated bicycle lane extends along the southern side of Somerville Avenue and continues through the intersection towards Washington Street.

# Somerville Avenue at Hawkins Street

The intersection of Somerville Avenue and Hawkins Street is a three-leg intersection. The northbound Hawkins Street approach consists of a single right-turn lane under stop control, and the eastbound Somerville Avenue approach consists of two free-flowing through lanes. Crosswalks exist across the western and southern legs of the intersection, and they are accompanied by curb ramps which include elements to generally comply with ADA. The curb ramp across the western Somerville Avenue leg is also accompanied by a flashing advanced-warning sign. A dedicated bicycle lane extends along the southern side of Somerville Avenue and continues through the intersection.

# **Public Transportation**

The project is served by numerous Massachusetts Bay Transportation Authority (MBTA) bus routes as well as the future MBTA Green Line Extension Union Square station. A summary of the public transportation within a half mile of the site is provided below, and the average frequencies for the bus routes serving the study area is provided in Table 1. The locations and average number of boardings and alightings at the key bus stops in the vicinity of the site are shown in Figure 2.

#### Route 69

MBTA Route 69 operates between Harvard Square and Lechmere. The closest stop pair to the site is on Cambridge Street at Norfolk Street, which is an approximately 11-minute and 0.5-mile walk. Neither stop has a bus shelter.

# Route 80

MBTA Route 80 operates between Arlington Center and Lechmere. The closest stop pair is on McGrath Highway. Both stops are an approximately 11-minute and 0.5 mile walk from the project

site. The outbound stop has a shelter and is located on an on-ramp to the McGrath Highway overpass. The inbound stop is located on a three-lane section of the road.

#### Route 85

MBTA Route 85 operates between Spring Hill and Kendall Square. The inbound route operates on Somerville Avenue directly by the site. The closest inbound stop is on Somerville Avenue at Union Square. The closest outbound stop is on Bow Street at Warren Avenue. Both stops are under a one-minute walk from the site and neither has a shelter.

#### Route 86

MBTA Route 86 operates between Sullivan Square and Cleveland Circle. The closest pair of stops to the site are one block away on Washington Street and each stop is an approximately two-minute walk. Neither stop has a shelter.

#### Route 87

MBTA Route 87 operates between Arlington and Lechmere via Somerville Avenue. The inbound route operates on Somerville Avenue directly by the site. The closest inbound stop is on Somerville Avenue at Union Square. The closest outbound stop is on Bow Street at Warren Avenue, just outside Union Square. Both stops are under a one-minute walk from the site and neither has a shelter.

#### Route 88

MBTA Route 88 operates between Arlington and Lechmere via Highland Avenue. The closest stop pair to the site is on Highland Avenue at Walnut Street, which is an approximately 11-minute and 0.5-mile walk. Both stops have shelters.

#### Route 90

MBTA Route 90 operates between Davis Square and Assembly Row. Its closest stop pair to the site is on Highland Avenue at Walnut Street, which is an approximately 11-minute and 0.5-mile walk. Both stops have shelters.

#### Route 91

MBTA Route 91 operates between Central Square and Sullivan Square. Its closest inbound (to Central Square) stop to the site is on Somerville Avenue at Stone Avenue and is an approximately two-minute walk. This stop has a shelter. The closest outbound stop to the site is an approximately five-minute and 0.3-mile walk on Prospect Street at Bennett Court. This stop does not have a shelter.

## Route CT2

MBTA Route CT2 operates between Sullivan Square and Ruggles. Its closest outbound (to Sullivan Square) stop is an approximately five-minute and 0.3-mile walk on Prospect Street at Bennett Court. This stop does not have a shelter. Its closest inbound (to Ruggles) stop to the site is on Somerville Avenue at Stone Avenue and is an approximately two-minute walk. This stop has a shelter.

# Green Line

There is currently no rapid transit service within a half mile of the project site. However, MBTA anticipates opening the Union Square extension of the Green Line D Branch in March 2022. This new service will allow for access to the new dispensary from most of the MBTA's rapid transit network. Green Line service to Union Square is anticipated to have same frequencies as the existing Green Line service. As of January 2022, the D Branch of the Green Line has a weekday peak frequency of

eight to nine minutes and a weekday off-peak and weekend frequency of ten to twelve minutes. The new Union Square station will be located on Prospect Street, which is an approximately six-minute and 0.3-mile walk from the project site.

**Table 1: Bus Route Frequency Summary** 

	Frequency by Route (Minutes)												
Period	Rt. 69	Rt. 80	Rt. 85	Rt. 86	Rt. 87	Rt. 88	Rt. 90	Rt. 91	Rt. CT2				
AM Peak	12	24	38	11	18	18	35	24	23				
Midday	25	26	n/a	33	25	25	65	50	58				
PM Peak	20	26	47	13	20	20	35	25	40				
Evening	40	56	n/a	35	29	30	60	45	n/a				
Saturday	27	40	n/a	31	30	30	61	46	n/a				
Sunday	40	68	n/a	32	41	40	70	45	n/a				

n/a Not applicable

### Field Review

Field reviews of the existing study area were conducted on Tuesday, January 25, 2022, and Friday, March 5, 2022. These reviews included the documentation of:

- Lane widths
- Traffic control devices
- Existing pedestrian and bicycle infrastructure
- Sight distance from the existing driveway on Somerville Avenue
- Posted speed limits
- General traffic operations

Each of these elements from the field reviews have been incorporated into the vehicular capacity, pedestrian, bicycle, and transit analyses documented within this report.





Figure 2
Existing Transit Facilities
Liberty Cannabis
Somerville, Massachusetts

# **Existing Condition Volumes**

# **Existing Volume Data Collection**

To assess peak hour traffic conditions, manual turning movement counts (TMCs) were conducted at the study area intersections between 6:00 AM and 8:00 PM on Tuesday, February 15, 2022, and between 10:00 AM and 2:00 PM on Saturday, February 12, 2022. In addition, two 72-hour automatic traffic recorder (ATR) counts were conducted, one on Somerville Avenue west of Hawkins Street and one on Bow Street west of Warren Avenue.

The results of the turning movement counts are tabulated by 15-minute periods and are provided in Appendix A of this report. The four highest consecutive 15-minute intervals during each of these count periods constitute the peak hours that are the basis of the traffic analysis provided in this report. Based on a review of the peak period traffic data, the weekday morning peak hour at the study area intersections occurs between 7:45 AM and 8:45 AM, the weekday afternoon peak hour occurs between 5:00 PM and 6:00 PM, and the Saturday midday peak hour occurs between 12:45 PM and 1:45 PM.

The ATR counts were conducted from Thursday, March 3, 2022, through Saturday, March 5, 2022, to avoid Somerville school breaks. The results of the ATR are summarized in Table 2 below and provided in Appendix A.

# **Seasonal Variation**

Based on MassDOT's 2019 Weekday Seasonal Factors, February traffic volumes on urban principal arterial roadways like Somerville Avenue are approximately three percent lower than an average month. To account for this variation, the counted traffic volumes were adjusted upward by three percent to reflect an average month.

# **Existing Transportation Volumes**

The resulting seasonally adjusted 2022 Existing condition vehicle volumes are shown in Figure 3, Figure 4, and Figure 5 for the weekday morning, weekday afternoon, and Saturday midday peak hours, respectively, and are documented in the traffic projection model provided in Appendix B of this report. The counted volume of pedestrians and bicyclists under 2022 Existing conditions are shown in Figure, Figure 7, and Figure 8, respectively.

A summary of the daily ATR count data is provided below.

**Table 2: Automatic Traffic Recorder Data** 

	Weekday <sup>1</sup>	Saturday <sup>2</sup>	,	85th % <sup>4</sup>
Location	ADT	ADT	HV% <sup>3</sup>	Speed
Somerville Avenue EB	7,710	7,210	2.5%	24
Bow Street WB	9,350	9,340	3.3%	20

- 1 Average daily traffic on Thursday, March 3, 2022
- 2 Average daily traffic on Saturday, March 5, 2022
- 3 Heavy vehicle percentage
- 4 85th percentile speed in miles per hour

# **Existing Site Trips**

To understand the actual number of trips generated by the existing medical-only dispensary, entering and exiting pedestrian counts were conducted on Thursday, January 27, 2022, from 6:00 AM to 8:00 PM. During these counts, the total number of person trips to the site between 6:00 AM and 10:00 AM was two, both of which were entering trips. Overall person trips to and from the existing dispensary generally increase beginning at 10:30 AM, fluctuating throughout the day with a peak around 5:00 PM. The highest hourly volume of person trips was identified to occur from 5:00 PM to 6:00 PM. During this weekday afternoon peak hour, a total of 23 trips (nine entering trips and 14 exiting trips) were observed.

#### U.S. Census Data

To assess the typical modes of transportation used for trips in the area of the site, data from the U.S. Census American Community Survey (ACS) was reviewed for Census tract 3512.04. Table 3 presents the resulting mode share for trips within the project's Census tract based on the 2015-2019 ACS data. The "work from home" and "other means" categories of the data were distributed proportionally between each of the modes displayed below.

**Table 3: Mode Share Summary** 

Mode	Mode Share
Drove alone	37.8%
Carpooled	1.5%
Public transportation	27.9%
Bicycle	15.2%
Walked	17.6%

Additionally, the average vehicle occupancy for the Census tract containing the project site (Census tract 3512.04) was calculated based on single occupancy and carpooling data available from the ACS for the years 2015 to 2019. The resulting vehicle occupancy rate was identified as 1.04 persons per vehicle.

The calculated mode share and vehicle occupancy have been utilized to estimate the breakdown of estimated person trips associated with the proposed project and is discussed in the trip generation section of the report in more detail.

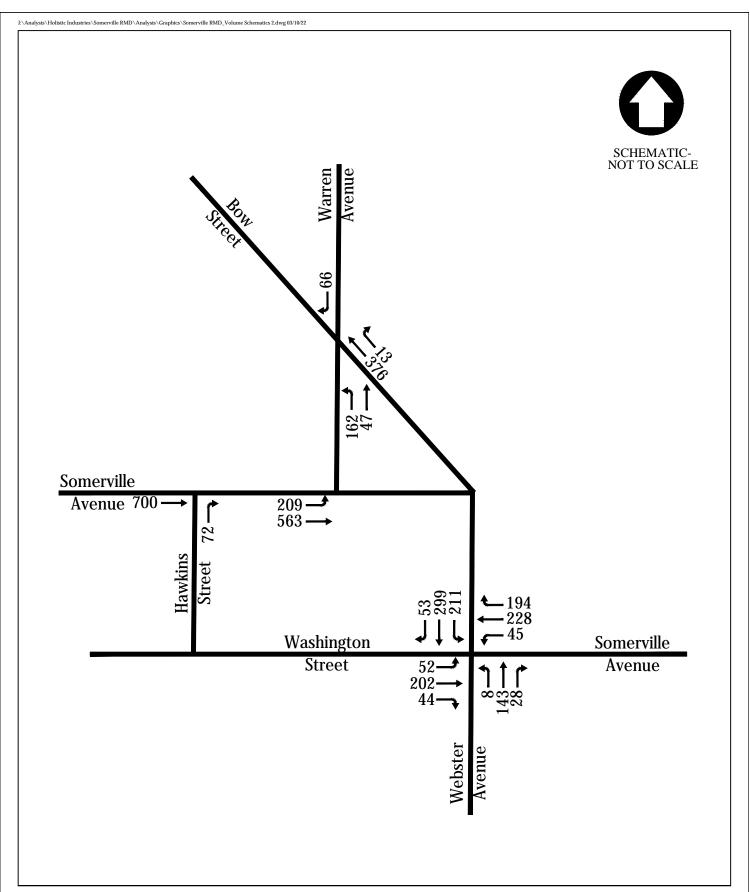




Figure 3 2022 Existing Weekday Morning Peak Hour Traffic Volumes Liberty Cannabis Somerville, Massachusetts

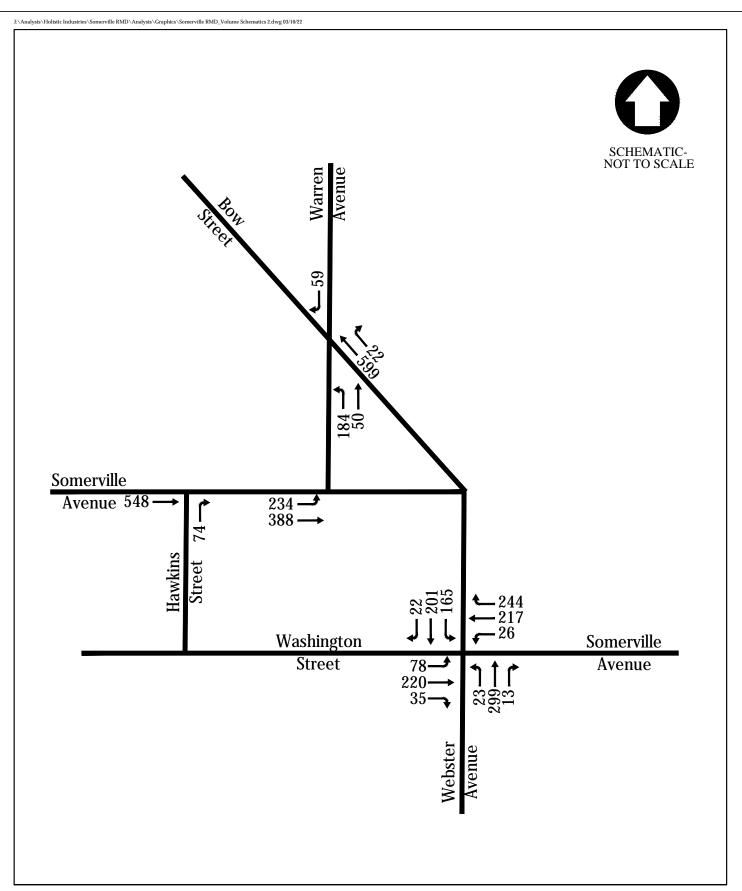




Figure 4 2022 Existing Weekday Afternoon Peak Hour Traffic Volumes Liberty Cannabis Somerville, Massachusetts

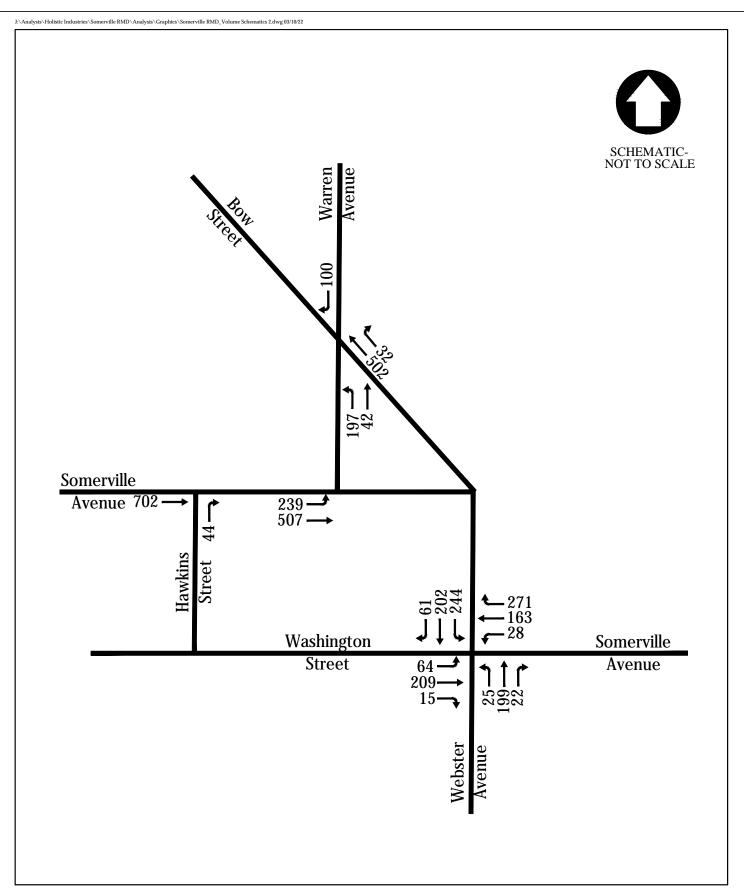
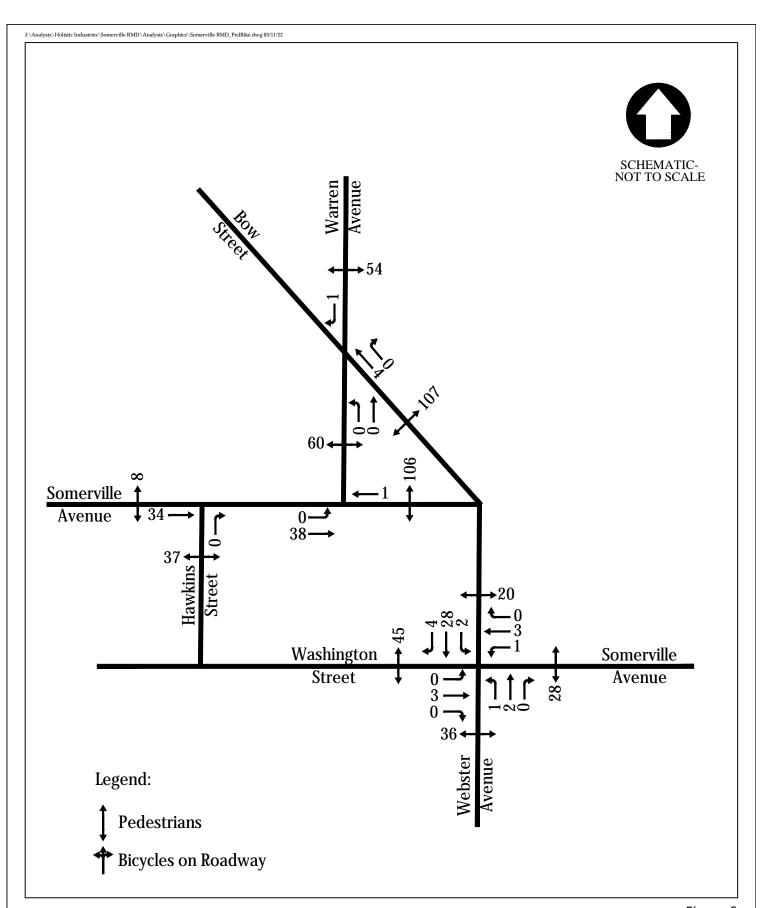




Figure 5 2022 Existing Saturday Midday Peak Hour Traffic Volumes Liberty Cannabis Somerville, Massachusetts





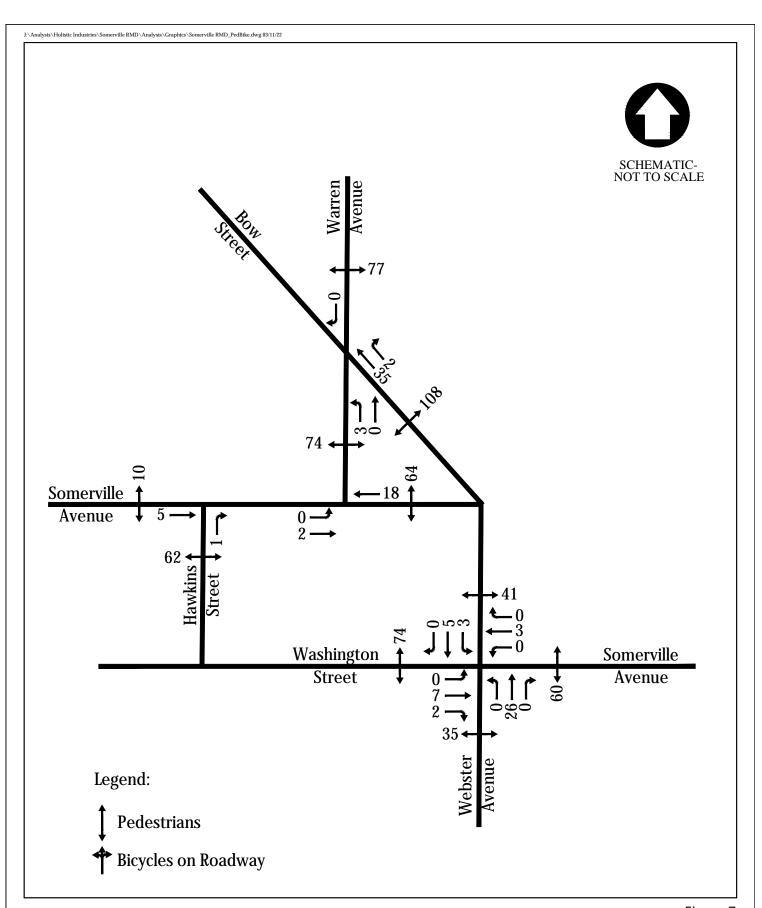




Figure 7 2022 Existing Weekday Afternoon Peak Hour Pedestrian and Bicycle Volumes Liberty Cannabis Somerville, Massachusetts

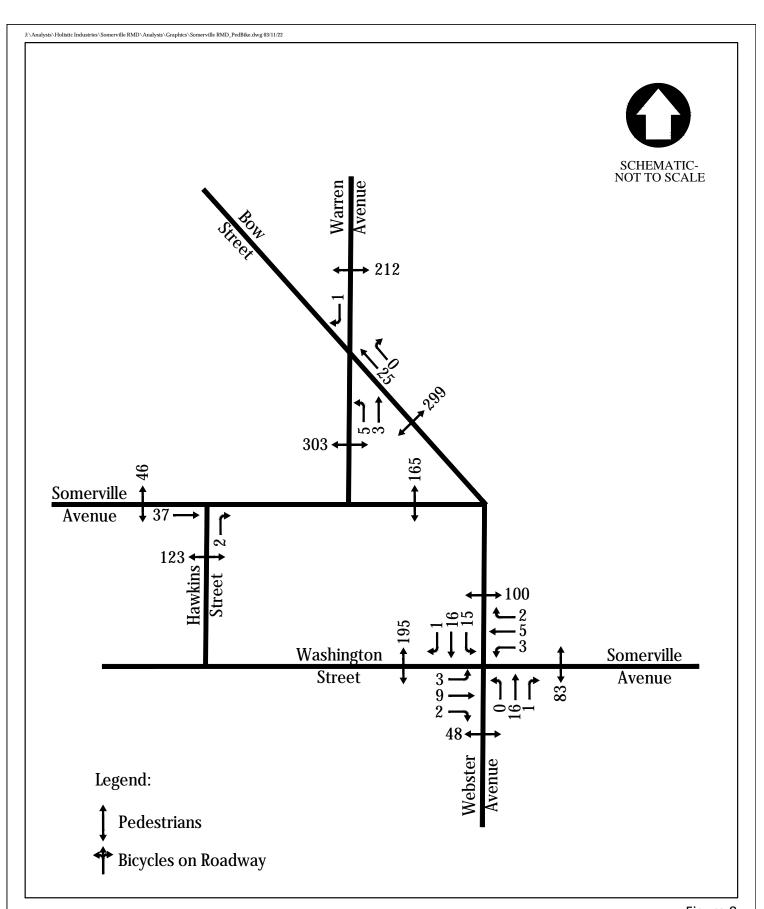




Figure 8 2022 Existing Saturday Midday Peak Hour Pedestrian and Bicycle Volumes Liberty Cannabis Somerville, Massachusetts

# **Crash Summary**

Crash data for the study area intersections was obtained from MassDOT for the most recent five-year period available. The results of the crash analysis are summarized below and are provided in Appendix C.

The MassDOT Crash Rate Worksheet calculations were used to determine whether the crash frequencies at the study area intersections were unusually high given the travel demands at each location. The MassDOT Crash Rate Worksheet calculates a crash rate expressed in crashes per million entering vehicles. The calculated rate is then compared to the average rate for signalized and unsignalized intersections statewide and within MassDOT District 4. For unsignalized intersections, the statewide and District 4 average crash rates are both 0.57 crashes per million entering vehicles. For signalized intersections the District 4 crash rate is 0.73 crashes per million entering vehicles, and the statewide average is 0.78 crashes per million entering vehicles.

During the five-year period reviewed, the intersection of Somerville Avenue at Bow Street/Webster Avenue & Washington Street is reported to have experienced a total of eight crashes resulting in a crash rate of 0.22 crashes per million entering vehicles, which is well below the statewide and District averages. Of the eight reported crashes, three were sideswipe crashes, one was an angle crash, and one was a single-vehicle crash. Two of the reported crashes involved a pedestrian. Five crashes resulted in property damage only, one crash resulted in a nonfatal injury, and the outcome of the remaining two crashes is unknown.

Between 2015 and 2019, the combined intersections of Somerville Avenue and Bow Street at Warren Avenue are reported to have experienced a total of 14 crashes, resulting in a crash rate of approximately 0.48 crashes per million entering vehicles. Five of the reported crashes were rear-end crashes, four crashes were angle crashes, two were sideswipes, two were single vehicle crashes, and one crash was of unknown type. Nine crashes resulted in property damage only, two crashes resulted in a nonfatal injury, and the outcome of three crashes is unknown.

The intersection of Somerville Avenue at Hawkins Street is reported to have experienced two crashes during the five-year period reviewed resulting in a crash rate of 0.11 crashes per million entering vehicles. One of the reported crashes was an angle crash, and the other was a rear-end crash. Both crashes resulted in property damage only.

Each of the study area intersections is encompassed by larger Highway Safety Improvement Plan (HSIP) pedestrian and bicycle crash clusters from 2010-2019 within the Union Square area. Since its identification as an HSIP intersection, Somerville Avenue at Bow Street/Webster Avenue and Washington Street has been reconstructed, which may result in a reduction in the severity of crashes at the intersection. In addition, traffic signal timing updates proposed by the City of Somerville, described in more detail later in this report, may improve the operations at the intersection.

#### **CURRENT-YEAR BUILD CONDITIONS**

# Site-Generated Traffic

#### **ITE Trip Generation**

In order to estimate the total number of vehicle trips associated with the proposed adult-use dispensary, the Institute of Transportation Engineers (ITE) publication *Trip Generation Manual, 11th Edition*, was referenced. For the proposed dispensary, Land Use Code 882 (Marijuana Dispensary) was utilized. The estimated ITE vehicle trips were then converted to person trips utilizing the vehicle occupancy rate of 1.04 persons per vehicle identified based on U.S. Census data. The number of person trips was then distributed among the various modes proportionally based on the estimated U.S. Census tract mode shares. The resulting trip generation by mode for the proposed project based on ITE data for LUC 882 is displayed in Table 4 below.

								-							
	Weekday AM		We	Weekday PM		Saturday Midday		Weekday Daily			Saturday Daily				
Description	In	Out	Total	In	Out	Total	In	Out	Total	In	Out	Total	In	Out	Total
Proposed ITE Trips <sup>1</sup>	16	15	31	28	28	56	43	43	86	317	317	634	389	389	778
Person Trips <sup>2</sup>	16	15	31	28	28	56	43	43	86	317	317	634	389	389	778
Drive alone	6	6	12	11	11	22	16	16	32	120	120	240	147	147	294
Carpool	1	0	1	0	0	0	1	1	2	5	5	10	6	6	12
Public transportation	4	4	8	8	8	16	12	12	24	88	88	176	108	108	216
Bicycle	2	2	4	4	4	8	6	6	12	48	48	96	59	59	118
Walk	3	3	6	5	5	10	8	8	16	56	56	112	69	69	138
Resulting Vehicle Trips	7	6	13	11	11	22	17	17	34	125	125	250	153	153	306

**Table 4: Estimated ITE Trip Generation** 

As shown in Table 4, based on ITE data the proposed adult-use dispensary would be estimated to generate approximately 13 vehicle trips (7 entering vehicles and 6 exiting vehicles) during the weekday morning peak hour, approximately 22 vehicle trips (11 entering vehicles and 11 exiting vehicles) during the weekday afternoon peak hour, and approximately 34 vehicle trips (17 entering vehicles and 17 exiting vehicles) during the Saturday midday peak hour. Over an average weekday, the proposed project is estimated to generate approximately 250 vehicle trips (125 entering vehicles and 125 exiting vehicles), and approximately 306 vehicle trips (153 entering vehicles and 153 exiting vehicles) during a Saturday. Based on current store hours, the dispensary would not be open during the weekday morning peak hour; however, the ITE estimates have been provided to account for potential employee arrival during those times.

ITE does not provide differentiated trip estimates for medical-only dispensaries vs. adult-use dispensaries and bases trips per square foot of the building. Because the size of the building is not proposed to change as part of the project, no changes in trips would be reflected from the existing medical use to the proposed adult retail sales utilizing ITE trip generation estimates.

# Project-Specific Trip Generation

A review of estimated trips has also been conducted by reviewing the number of points of sale, appointment length, anticipated staffing, and anticipated deliveries. The existing medical dispensary

<sup>1</sup> ITE Land Use Code 882 (Marijuana Dispensary), based on 3,000 square feet.

<sup>2</sup> Based on a vehicle occupancy rate of 1.04 persons/vehicle from Census Journey to Work data for Census Tract 3512.04

has five points of sale with 15-minute appointments per register which results in a maximum of approximately 20 customer trips entering and exiting per hour. The proposed dispensary including adult-use would include a maximum of ten points of sale with continued 15-minute appointments per register resulting in a maximum additional 20 customer trips entering and exiting per hour.

Deliveries to the project site currently occur three to five times per week and would be expected to occur approximately five times a week with the proposed adult-use in place. Deliveries are typically scheduled outside of the peak hours of the adjacent street traffic (deliveries would be scheduled between 12:00 PM and 4:00 PM) and would continue to be scheduled as such with the proposed project in place. Deliveries are expected to continue to be completed using light commercial vehicles/vans, approximately 25 feet in length, utilizing the existing driveway to access the area and building door behind the gate. The approximate number of deliveries expected to the project site during a typical weekday are presented in Table 5 below.

**Table 5: Delivery Schedule** 

Day	Period	Deliveries
Weekday	7:00AM-9:00AM	0
	9:00AM-4:00PM	1
	4:00PM-6:00PM	0
	6:00PM-7:00AM	0
Weekends	All Day	0

Because deliveries are expected to occur outside of the weekday morning, weekday afternoon, and Saturday midday peak periods, the projected peak hour trip generation for these periods does not include deliveries.

The existing dispensary has six to eight employees over all shifts on a typical day. Anticipated staffing levels for the proposed adult use would increase to approximately eight to ten employees on weekdays and ten to fourteen employees on the weekend over all shifts on a given day. With typical shifts occurring from 9:00 AM to 5:30 PM and from 2:00 PM to 8:30 PM, a portion of the existing and proposed trips to the site during the weekday morning, weekday afternoon, and Saturday midday peak hours would be attributed to employees. For the purposes of this analysis, the number of employees accessing the site has been divided between the two standard shifts.

Table 6 provides a summary of the estimated existing and proposed trips to the site using the above information for the weekday morning, weekday afternoon and Saturday midday peak hours.

**Table 6: Estimated Change in Trips** 

	We	eekday	AM	W	eekday	PM	Saturday Midday			
Description	In	Out	Total	In	Out	Total	In	Out	Total	
Existing Customer Trips	0	0	0	20	20	40	20	20	40	
Existing Employee Trips	<u>4</u>	<u>0</u>	<u>4</u>	<u>0</u>	<u>4</u>	<u>4</u>	<u>4</u>	<u>0</u>	<u>4</u>	
Total Existing Trips	4	0	4	20	24	44	24	20	44	
Proposed Customer Trips	0	0	0	40	40	80	40	40	80	
<u>Proposed Employee Trips</u> Total Proposed Trips	<u>5</u> 5	<u>0</u> 0	<u>5</u> 5	<u>0</u> 40	<u>5</u> 45	<u>5</u> 85	<u>7</u> 47	<u>0</u> 40	<u>7</u> 87	
Total New Person Trips	1	0	1	20	21	41	23	20	43	

Trip estimates based on employee scheduling and sale appointment slots.

The number of trips outlined in Table 6, above, includes the estimated maximum number of customer trips within an hour assuming all points of sale are in use and all available appointment slots are utilized. The estimated number of existing trips are shown to be above the actual observed number of trips into and out of the site described in the Existing Site Trips section above.

The increases in person trips distributed by mode are summarized in Table 7 below.

**Table 7: Estimated Change in Trips** 

	Weekday AM			We	eekday	PM	Saturday Midday			
Description	In	Out	Total	In	Out	Total	In	Out	Total	
Total New Person Trips <sup>1</sup>	1	0	1	20	21	41	23	20	43	
Drive Alone	1	0	1	8	8	16	9	7	16	
Carpool	0	0	0	0	0	0	0	0	0	
<b>Public Transportation</b>	0	0	0	6	6	12	7	6	13	
Bicycle	0	0	0	3	3	6	3	3	6	
Walk	0	0	0	3	4	7	4	4	8	
Total Vehicle Trips	1	0	1	8	8	16	9	7	16	

<sup>1</sup> Mode share based on US Census data for tract 3512.04

After applying the mode splits for Census tract 3512.04, the resulting increase in vehicle trips on the adjacent roadway network includes approximately one new vehicle trip during the weekday morning peak hour, 16 new vehicle trips (eight entering vehicles and eight exiting vehicles) during the weekday afternoon peak hour, and approximately 16 new vehicle trips (nine entering vehicles and seven exiting vehicles) during the Saturday midday peak hour.

# **Project Trip Distribution and Assignment**

The traffic estimated to be generated by the proposed development was distributed onto the study area roadways and intersections based on Census Journey-to-Work data for employees who work in Somerville. The Journey-to-Work data is included in Appendix D of this report. The resulting arrival

and departure patterns are presented in Figure 9 and the traffic projection model found in Appendix B.

The project-related vehicle traffic was then assigned to the surrounding roadway network based on the project trip distribution patterns presented in Figure 9. The resulting distributed new project vehicle trips are shown in Figure 10, Figure 11, and Figure 12 for the weekday morning, weekday afternoon, and Saturday midday peak hours, respectively.

### 2022 Build Volumes

To establish the 2022 Build peak hour traffic volumes, the distributed new project trips were added to the 2022 Existing peak hour traffic volumes to reflect the 2022 Build peak hour traffic volumes. The resulting 2022 Build weekday morning, weekday afternoon, and Saturday midday peak hour traffic volumes are presented in Figure 13, Figure 14, and Figure 15, respectively, and are documented in the traffic projection model presented in Appendix B of this report.

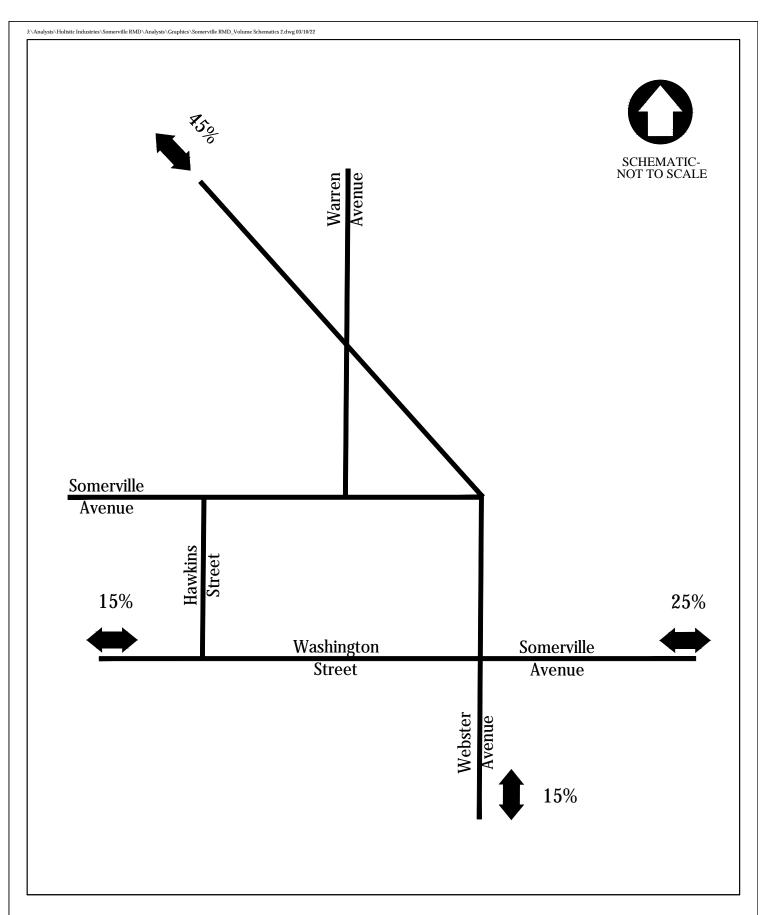




Figure 9
Direction of Arrivals and Departures
Liberty Cannabis
Somerville, Massachusetts

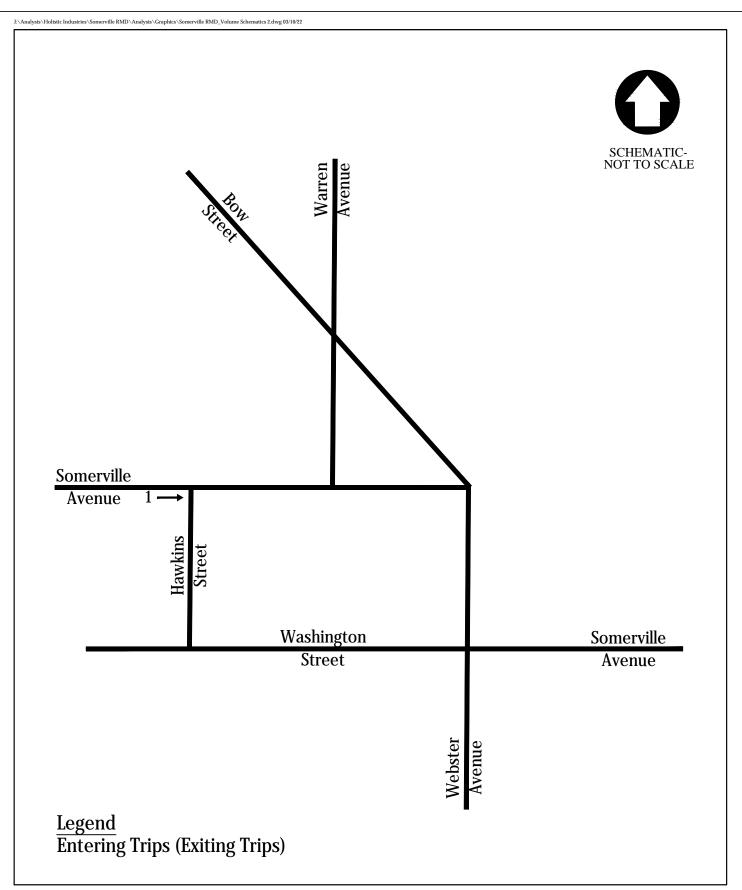




Figure 10 Weekday Morning Peak Hour New Project Trips Liberty Cannabis Somerville, Massachusetts

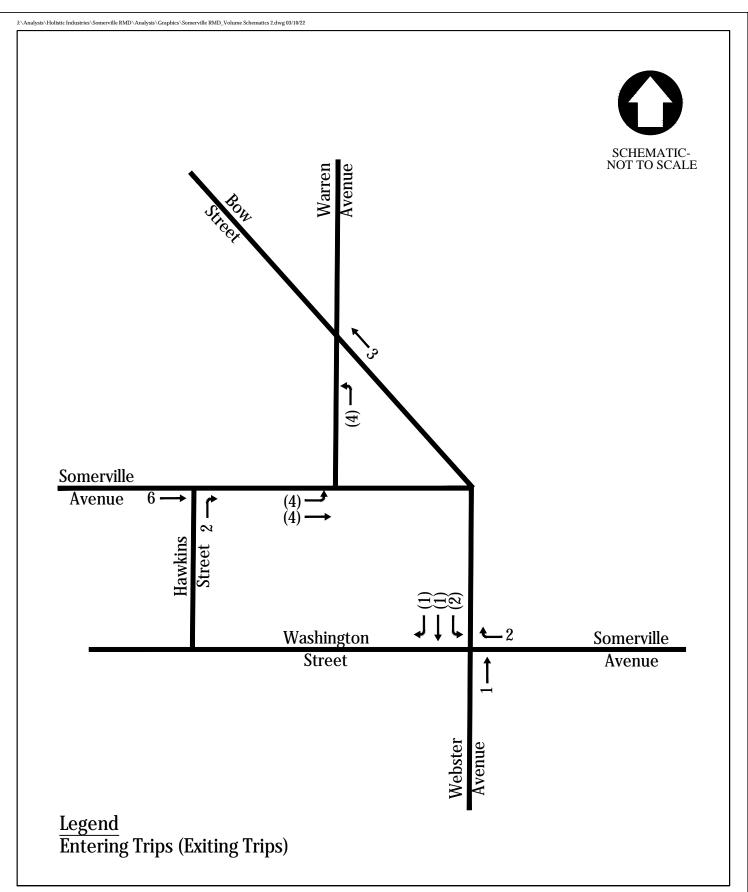




Figure 11 Weekday Afternoon Peak Hour New Project Trips Liberty Cannabis Somerville, Massachusetts

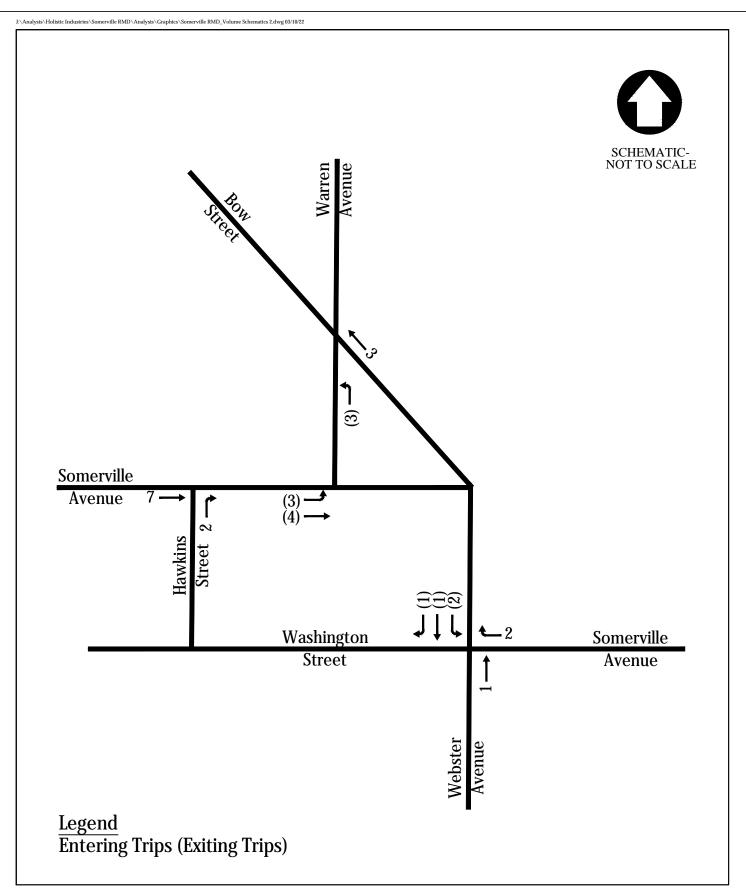




Figure 12 Saturday Midday Peak Hour New Project Trips Liberty Cannabis Somerville, Massachusetts

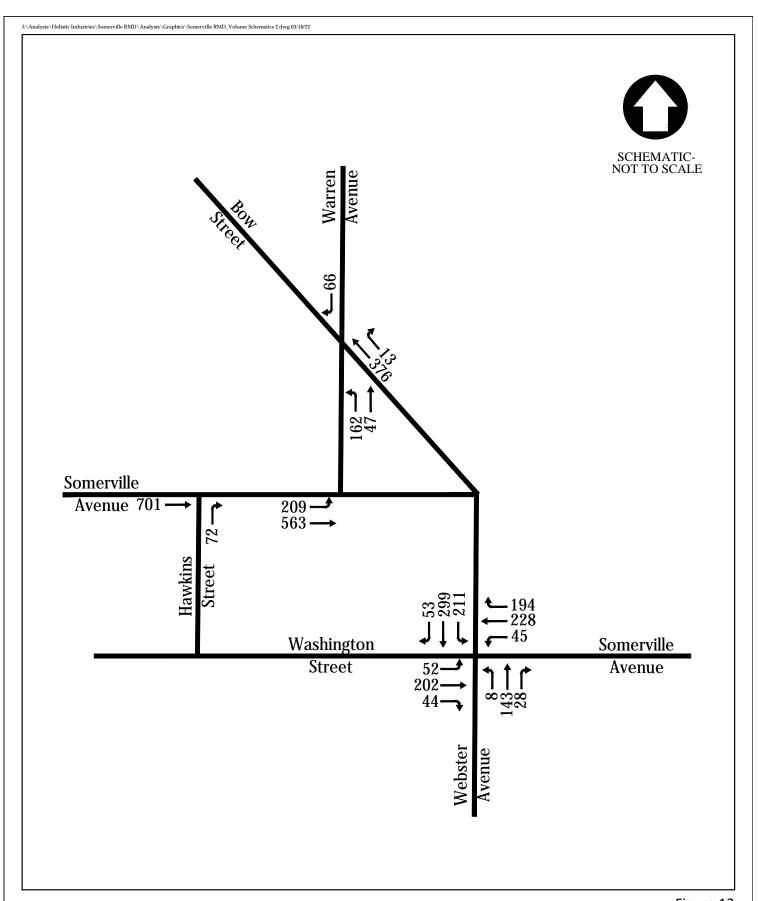




Figure 13 2022 Build Weekday Morning Peak Hour Traffic Volumes Liberty Cannabis Somerville, Massachusetts

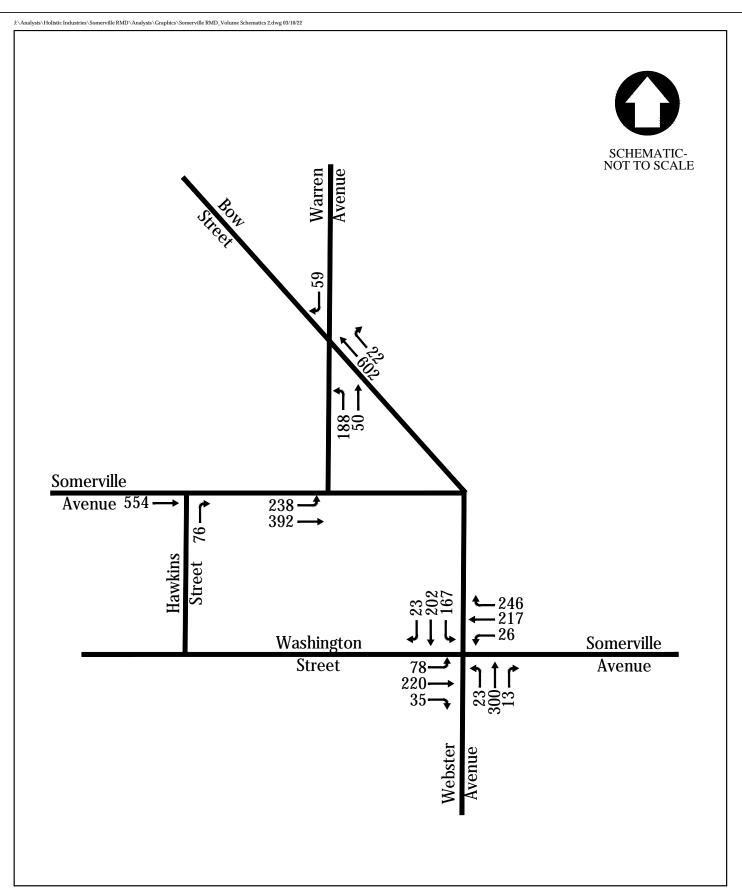




Figure 14 2022 Build Weekday Afternoon Peak Hour Traffic Volumes Liberty Cannabis Somerville, Massachusetts

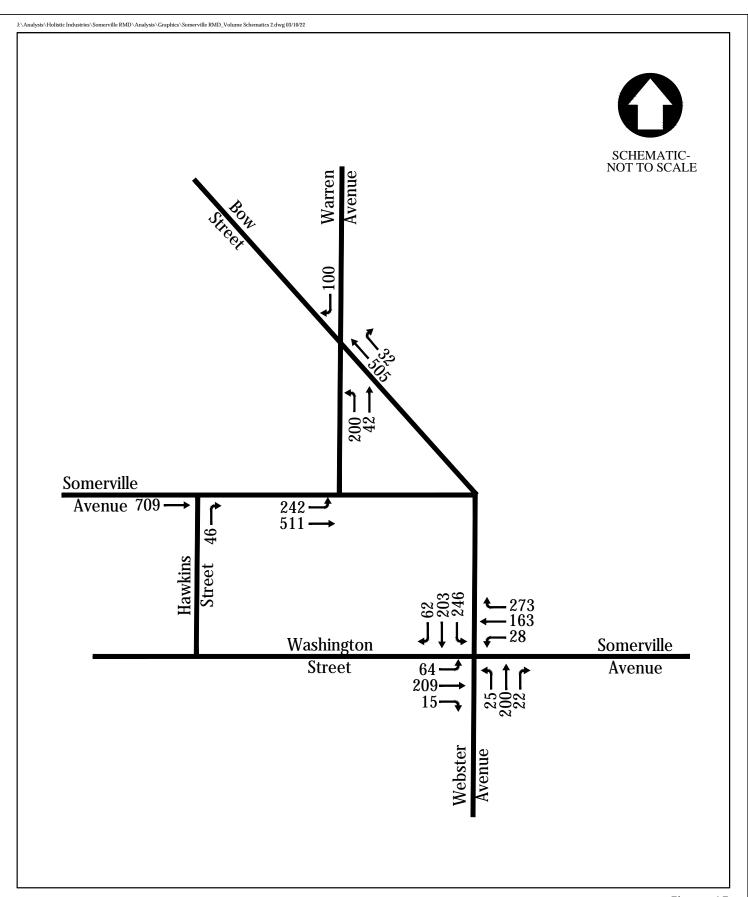




Figure 15 2022 Build Saturday Midday Peak Hour Traffic Volumes Liberty Cannabis Somerville, Massachusetts

#### **FUTURE YEAR CONDITIONS**

To determine future traffic demands on the study area roadways, the 2022 Build traffic volumes were projected to the future year 2027. Traffic volumes on the roadways in 2027 are assumed to include the 2022 Build traffic volumes as well as new traffic resulting from general growth in the study area and from other planned development projects. The potential background traffic growth unrelated to the proposed project was considered in the development of the 2027 Design Year Build peak hour traffic volume networks. A more detailed description of the development of the 2027 Design Year Build traffic volume networks is presented below.

# **Future Roadway Improvements**

Planned roadway improvement projects can affect area travel patterns and future traffic operations. Based on discussions with the City of Somerville, traffic signal phasing and timing changes were identified at the intersection of Somerville Avenue at Bow Street/Webster Avenue and Washington Street under the future condition. The changes include the addition of an exclusive pedestrian phase and yield control of the channelized right-turn lane from Somerville Avenue onto Bow Street. These proposed signal modifications were incorporated into the 2027 Design Year Build traffic analysis.

# **Background Traffic Growth**

Traffic growth is primarily a function of changes in motor vehicle use and expected land development in the region. To predict a rate at which traffic on the roadways in the vicinity of the site can be expected to grow during the five-year forecast period (2022 to 2027), both planned area developments and historic traffic growth were examined.

# Site-Specific Growth

Based on discussions with the City of Somerville, no specific planned developments were identified to be included in the projections of future growth.

# **Background Traffic Growth**

Background traffic growth accounts for changes in traffic volumes associated with general changes in population and other developments that are not known at this time. In order to establish an annual growth rate for the study area, the City of Somerville was consulted. A background growth rate of one quarter percent per year, compounded annually, was established to capture traffic growth associated with general changes in population and from other developments which may not be known at this time.

The resulting projected traffic volumes at the study area intersections are documented in the traffic projection model located in Appendix B.

# 2027 Design Year Build Traffic Volumes

The 2022 Existing peak hour traffic volumes were grown by one quarter percent per year (compounded annually) over the five-year study period (2022 to 2027), and the estimated vehicle trips from the proposed project were added to establish the 2027 Design Year Build weekday morning, weekday afternoon, and Saturday midday peak hour traffic volumes, which are illustrated

in Figure 16, Figure 17, and Figure 18, respectively, and are documented in the traffic projection model presented in Appendix B of this report.

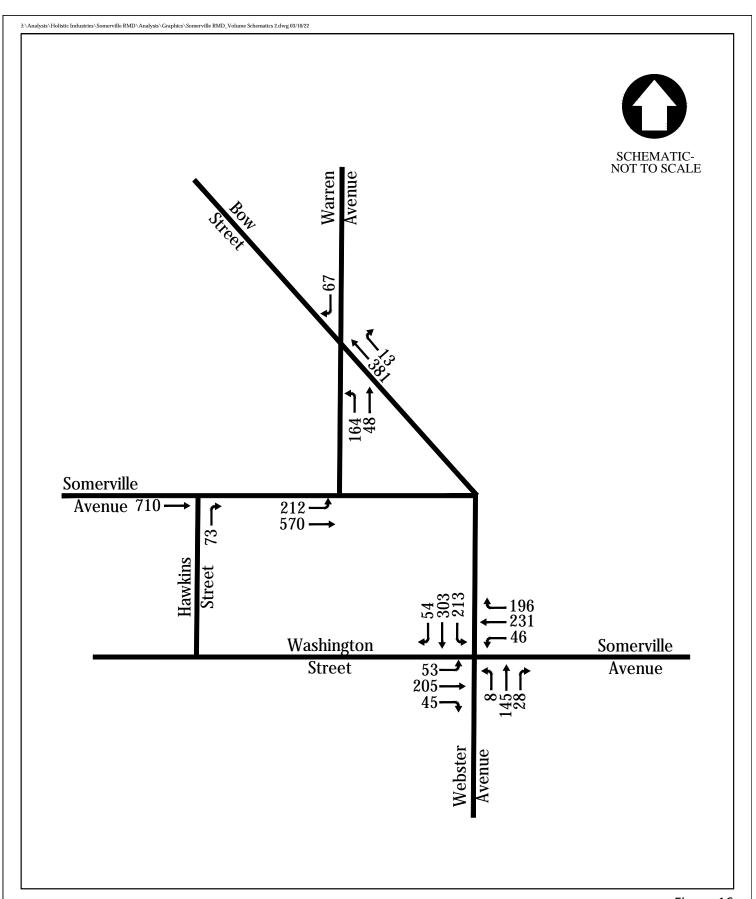




Figure 16 2027 Build Weekday Morning Peak Hour Traffic Volumes Liberty Cannabis Somerville, Massachusetts

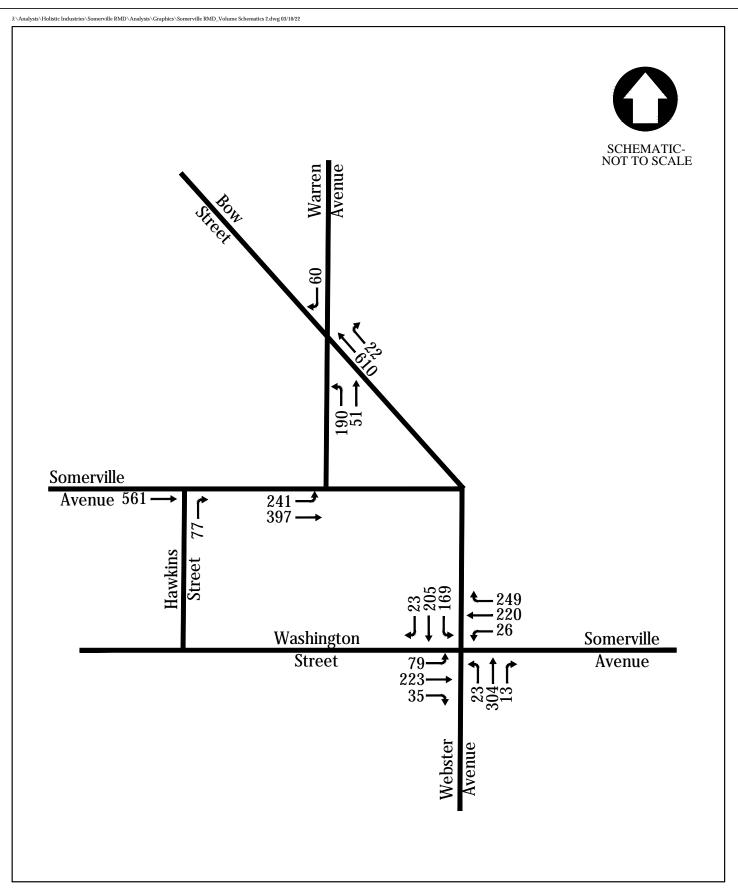




Figure 17 2027 Build Weekday Afternoon Peak Hour Traffic Volumes Liberty Cannabis Somerville, Massachusetts

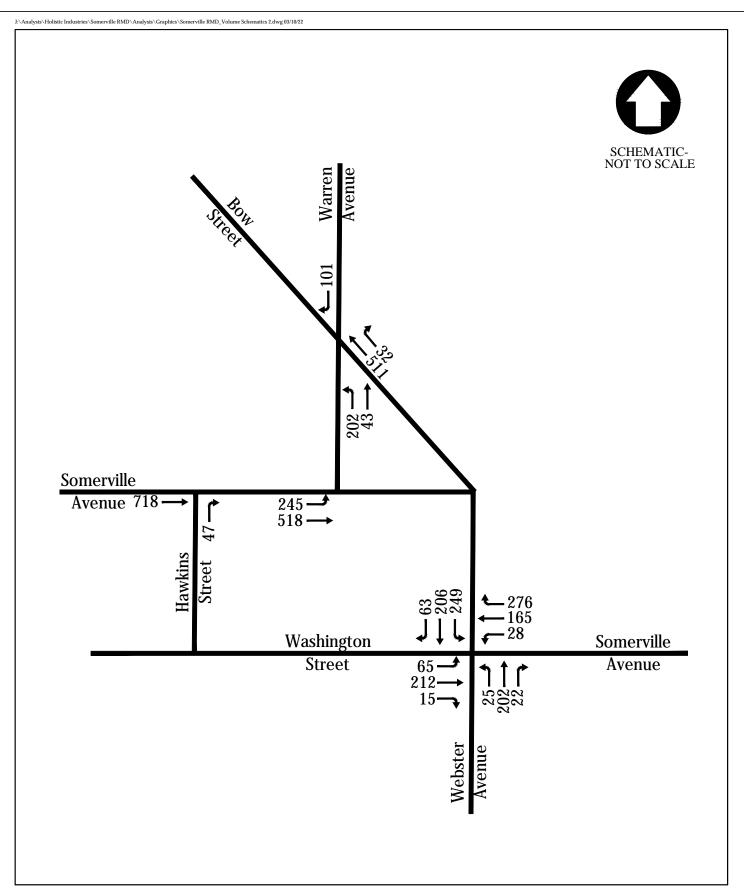




Figure 18 2027 Build Saturday Midday Peak Hour Traffic Volumes Liberty Cannabis Somerville, Massachusetts

## TRANSPORATION OPERATIONS ANALYSIS

In previous sections of this report, the quantity of pedestrians, vehicles, and bicycles at the study area intersections has been discussed. The following sections describe the overall quality of flow of the various modes through study area intersections during the weekday morning, weekday afternoon, and Saturday midday peak hours. The following section presents capacity analysis for vehicular operations, pedestrian level of traffic stress, bicycle level of traffic stress and an evaluation of potential impacts to public transit.

## **Vehicular Capacity Analysis**

As a basis for this assessment, intersection capacity analysis was conducted using the Synchro capacity analysis software at the study area intersections under the 2022 Existing, 2022 Build, and 2027 Design Year Build peak hour traffic conditions. The analysis is based on Synchro capacity analysis methodologies and procedures contained in the *Highway Capacity Manual* (HCM). In accordance with City of Somerville guidelines, the capacity analysis for the unsignalized study area intersections was performed using HCM2000 methodology, rather than the current HCM6 methodology. Average total vehicle delay is reported as level-of-service (LOS) on a scale of A to F. LOS A represents delays of 10 seconds or less and LOS F represents delays in excess of 50 seconds for unsignalized intersections and greater than 80 seconds for signalized intersections.

The Synchro model used in the capacity analysis was calibrated for the northbound Warren Street approach at its intersection with Bow Street. Based on field observations and anticipated driver behaviors in urban areas such as Somerville, the critical gap which a driver would accept in order to turn onto, or cross Bow Street from Warren Street was adjusted to lower than the default value in the HCM. The adjustment to gap acceptance is intended to better reflect the typical conditions experienced on the northbound Warren Street approach. No other calibration adjustments were made within the Synchro model as the remaining study area movements were shown to generally be representative of observed operations in the field.

The detailed Synchro capacity analysis worksheet for the 2022 Existing, 2022 Build, and 2027 Design Year Build traffic conditions are presented in Appendix E, Appendix F, and Appendix G, respectively. The capacity analysis results for the study area intersections are displayed in Table 8 and Table 9 for the signalized and unsignalized/pedestrian crossing intersections, respectively. A more detailed summary of the capacity analysis for each study area intersection is provided in Appendix H. The results of the specific capacity analysis at the study area intersections are discussed below.

**Table 8: Signalized Intersection Capacity Analysis** 

	Peak	20	22 Exist	ing	2	022 Bui	ld	2	.027 Bui	ld
Intersection	Period	LOS <sup>1</sup>	Delay <sup>2</sup>	ICU <sup>3</sup>	LOS	Delay	ICU	LOS	Delay	ICU
Somerville Avenue at	AM	E	64.3	0.99	Е	64.3	0.99	E	68.4	0.76
Bow Street/Webster Avenue	PM	Ε	68.6	0.98	Ε	68.7	0.98	Ε	66.5	0.79
& Washington Street	SAT	Ε	64.0	0.95	Ε	64.1	0.95	Ε	64.8	0.74

- 1 Level-of-Service
- 2 Average vehicle delay in seconds
- 3 Intersection capacity utilization

As shown in Table 8, the fully signalized intersection of Somerville Avenue at Bow Street/Webster Avenue & Washington Street is shown to currently operate at overall LOS E during the weekday morning, weekday afternoon, and Saturday midday peak hours. Under 2022 Build conditions with the proposed project in place, the intersection is shown to continue to operate at an overall LOS E during all three peak hours analyzed with negligible additional delay as a result of the proposed project. Under 2027 Design Year Build conditions, with the proposed signal updates incorporated and the additional background growth, the intersection is shown to continue to operate at an overall LOS E during the weekday morning, weekday afternoon, and Saturday midday peak hours.

A summary of traffic operations at the unsignalized intersections and pedestrian crossing intersections is provided in Table 9 below.

**Table 9: Unsignalized/Pedestrian Crossing Intersections** 

	Peak	Peak			22 Exist	ing	2	022 Bui	ld	2	027 Bui	ld
Intersection	Period	Moven	nent	LOS <sup>1</sup>	Delay <sup>2</sup>	V/C <sup>3</sup>	LOS	Delay	V/C	LOS	Delay	V/C
Bow Street at	AM	WB	TR	Α	4.1	0.30	Α	4.1	0.30	Α	4.1	0.30
Warren Avenue		NB	LT	С	17.8	0.45	С	17.8	0.45	С	18.2	0.46
		SB	R	В	12.2	0.15	В	12.2	0.15	В	12.3	0.15
	PM	WB	TR	Α	5.5	0.47	Α	5.6	0.47	Α	5.6	0.48
		NB	LT	F	62.0	0.90	F	65.9	0.92	F	72.8	0.95
		SB	R	C	16.7	0.21	С	16.8	0.21	С	17.0	0.21
	SAT	WB	TR	Α	6.3	0.46	Α	6.3	0.46	Α	6.3	0.47
		NB	LT	F	75.6	0.92	F	79.5	0.94	F	87.2	0.97
		SB	R	С	22.1	0.36	С	22.2	0.36	С	22.6	0.37
Somerville Avenue	AM	EB	LT	Α	9.7	0.40	Α	9.7	0.40	Α	9.7	0.40
at Warren Avenue	PM	EB	LT	Α	6.2	0.29	Α	6.2	0.29	Α	6.2	0.30
	SAT	EB	LT	Α	9.0	0.37	Α	9.4	0.38	Α	9.4	0.39
Somerville Avenue	AM	NB	R	В	11.9	0.16	В	11.9	0.16	В	12.0	0.16
at Hawkins Street	PM	NB	R	В	11.2	0.13	В	11.3	0.14	В	11.3	0.14
	SAT	NB	R	В	11.3	0.09	В	11.4	0.09	В	11.4	0.09

- 1 Level-of-Service
- 2 Average vehicle delay in seconds
- 3 Volume to capacity ratio

At the intersection of Bow Street at Warren Avenue, the westbound Bow Street approach is shown to operate at LOS A during the peak hours analyzed. The southbound Warren Avenue approach is shown to operate at LOS B during the weekday morning peak hour and at LOS C during the weekday afternoon and Saturday peak hours. The northbound Warren Avenue approach is shown to operate at LOS C during the weekday morning peak hour and at LOS F but under capacity during the weekday afternoon and Saturday midday peak hours. With the proposed project in place, under both the 2022 Build and 2027 Design Year Build conditions, all movements at the intersection are projected to continue to operate at the same LOS as under Existing conditions, with minor increases to delay focused on the northbound Warren Avenue approach. Due to limitations of the Synchro capacity analysis software, the capacity analysis reported in Table 9 for the Warren Avenue approaches does not factor in additional gaps in the Bow Street traffic due to the pedestrian crossing signal at the intersection. Based on field observations during the weekday afternoon peak hour, the reported delays and queues summarized for the northbound Warren Avenue approach are expected to be more conservative than observed operations.

The eastbound Somerville Avenue approach at its intersection with Bow Street is shown to operate at LOS A during the weekday morning, weekday afternoon, and Saturday midday peak hours, under all conditions analyzed. The vehicle trips added to the intersection due to the proposed project are not shown to substantially impact the operations at the intersection of Somerville Avenue with Bow Street.

The stop-controlled Hawkins Street approach to Somerville Avenue is projected to operate at LOS B during all peak hours and conditions. The additional vehicle trips traveling through the intersection of Somerville Avenue and Hawkins Street are not shown to have a significant impact on the operations at the intersection.

Overall, the proposed redevelopment is not projected to have a significant impact on vehicular operations at the study area intersections.

## **Pedestrian Analysis**

## Signalized Crosswalks

As part of the evaluation of the pedestrian operations within the study area, the widths, crossing distances, Walk and Flashing Don't Walk timings, and maximum pedestrian delay for the crosswalks at the signalized intersections within the study area are presented in Table 10 below.

**Table 10: Signalized Crosswalks** 

						Flashing	MUTCD <sup>1</sup>	Max.
		Crosswalk	Crosswalk	Cycle	Walk	Don't	Flashing Don't	Ped.
Intersection	Crossing	Width (ft)	Length (ft)	Length (s)	(s)	Walk (s)	Walk (s)	Delay (s)
Somerville Avenue at	Western	10	70	180	9	21	20	167
Bow Street/Webster Avenue	Eastern	10	60	180	9	19	18	167
& Washington Street	Channelized Lane	10	22	180	9	19	7	167
	Southern	10	34	180	9	22	10	167
	Northern	10	72	180	9	22	21	167
Bow Street at	Eastern	9	26	45	10	5	8	31
Warren Avenue								
Somerville Avenue at	Eastern	12	32	80	9	22	10	67
Warren Avenue	Western	10	38	80	9	22	11	67

<sup>\*</sup> Signal timings including Walk, Flashing Don't Walk, and cycle length are based on field observations

Based on the existing crosswalk lengths and the field measured Walk and Flashing Don't Walk timings, each of the signalized crosswalks within the study area, with the exception of Somerville Avenue at Warren Avenue, are shown to meet the minimum clearance interval requirements for a walking speed of 3.5 feet per second. Future changes to the intersection of Somerville Avenue at Bow Street/Webster Avenue and Washington Street would include changes to the pedestrian phasing but are not specifically known at this time.

## Pedestrian Level of Traffic Stress

Pedestrian Level of Traffic Stress (PLTS) is a measurement that analyzes segments of sidewalk based on the overall level of comfort that it provides pedestrians. PLTS is based on sidewalk widths and conditions, and the separation provided from vehicle traffic, and assigns a score from PLTS1 to PLTS4. PLTS1 represents high-quality, accessible conditions which are comfortable to walk on and provide separation from vehicles. PLTS4 represents conditions which do not provide separated infrastructure for pedestrians or are inaccessible due to the condition, width, or some other characteristic of the sidewalks. The PLTS for the nearby roadway segments serving the project site, including on the routes to critical transit stops, are presented in Figure 19.

As shown in Figure 19, the sidewalks in the surrounding area all are shown to provide PLTS2 conditions or better. This suggests that the site will be accessible by foot, as well as by transit. The proposed project would not change pedestrian access to the site, and the impact of the estimated increase in pedestrian activity associated with the project is expected to be negligible.

<sup>1</sup> MUTCD Flashing Don't Walk based on a walking speed of 3.5 feet per second

## **Bicycle Analysis**

## **Bicycle Level of Traffic Stress**

Similar to Pedestrian Level of Traffic Stress, Bicycle Level of Traffic Stress (BLTS) is a measurement that summarizes and groups the quality of different types of bicycle infrastructure based on the overall experience of cyclists who use it. BLTS combines inputs such as the type and width of bicycle infrastructure and the nature of vehicle traffic on a given roadway segment and assigns that segment a score from BLTS1 to BLTS4. BLTS1 represents conditions that are comfortable for a wide variety of cyclists including children and senior citizens, and BLTS4 represents conditions that are only comfortable for a small segment of experienced, confident cyclists. The BLTS for the nearby roadway segments serving the project site are presented in Figure 19.

As shown, the roadways surrounding the site vary from BLTS1 to BLTS3, with Somerville Avenue in front of the site shown to provide BLTS3 conditions. The impact of the estimated increase in bicycle volumes associated with the project is expected to be negligible.

## **Transit Analysis**

As discussed in previous sections of this report, the proposed project is not anticipated to generate a large number of transit trips. During the weekday morning peak hour, the proposed redevelopment is estimated to generate zero new transit trips. During the weekday afternoon peak hour, 12 trips (six entering trips and six exiting trips) are estimated to arrive and depart using public transit. During the Saturday midday peak hour, a total of 13 trips (seven entering trips and six exiting trips) are estimated to arrive and depart using public transit. These trips equate to approximately one new transit trip every five minutes, which is expected to have a negligible impact on the overall transit operations within and around the study area.

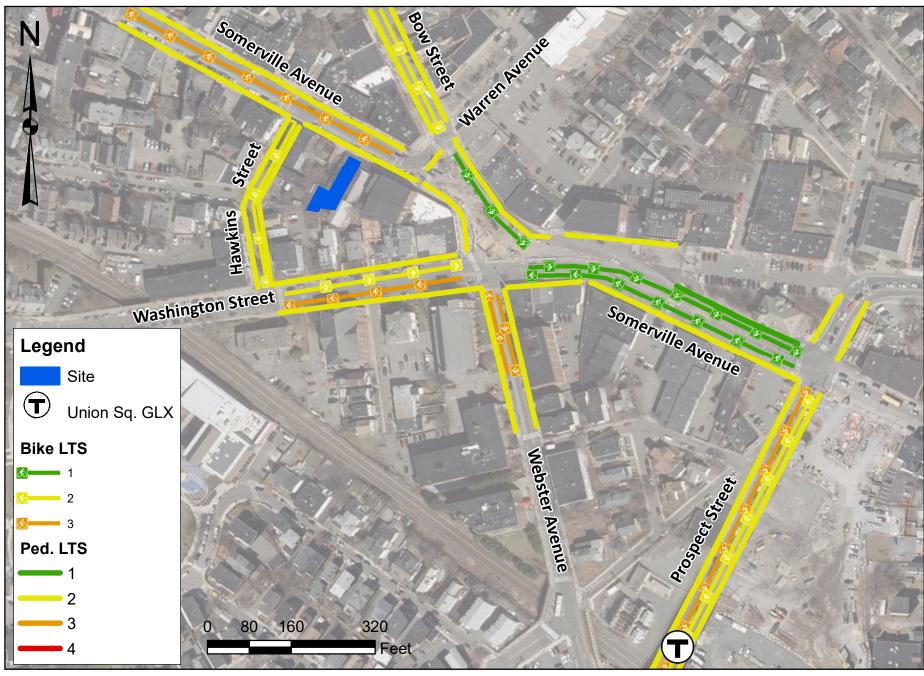




Figure 19
Pedestrian & Bicycle Levels of Traffic Stress
Liberty Cannabis
Somerville, Massachusetts

### **Site Access and Circulation**

The site is currently be accessed by vehicles via one driveway on Somerville Avenue. The majority of trips traveling to and from the site do so by foot utilizing the primary access through doors on Somerville Avenue. The proposed project would not alter the existing access and egress points, and access would continue to be provided via the existing driveway and doors on Somerville Avenue. The driveway would continue to be used for limited parking and for the loading and unloading of deliveries as outlined in the Transportation Access Plan (TAP) submitted as part of the project.

## Sight Distance

Location

A field review of sight distance was conducted at the location of the existing site driveway on Somerville Avenue. The American Association of State Highway and Transportation Officials (AASHTO) publication, A Policy on Geometric Design, 2018 Edition, defines minimum and recommended sight distances at intersections. The minimum sight distance is based on the required stopping sight distance (SSD) for vehicles traveling along the main road. The recommended sight distance allows vehicles to enter the main street traffic flow without requiring the mainline traffic to slow to less than 70% of their speed and is referred to as intersection sight distance (ISD). According to AASHTO, "If the available sight distance for an entering or crossing vehicle is at least equal to the appropriate stopping sight distance for the major road, then drivers have sufficient time to anticipate and avoid collisions." The available sight distance at the project site driveway is shown in Table 11.

SSD<sup>1</sup> ISD<sup>2</sup> 85th % Speed Meets Limit Speed Required Recommended **Sight Distance** Required **Site Driveway** Looking (mph) (mph) (ft) (ft) Measured (ft) SSD/ISD? Somerville Avenue Left (West) 25 24 155 240 490 Yes

**Table 11: Sight Distance Requirements** 

- 1 Stopping sight distance (see AASHTO equations 3-2 and 3-3) for the Citywide speed limit of 25 mph.
- 2 Intersection sight distance (see AASHTO equations 9-1 and 9-2) for the Citywide speed limit of 25 mph.

Based on the field review, the available sight distance at the existing site driveway is shown to exceed the required and recommended AASHTO sight distances for the speed limit of 25 mph and the 85<sup>th</sup> percentile speed on the roadway. The sight lines provided by the project site driveway are shown to allow for safe and efficient access to and from the site.

## **PROJECT MITIGATION**

As outlined in this report, the proposed project is not shown to have a significant impact to the transportation operations of the surrounding roadways and intersections. Additionally, changes to the intersection of Somerville Avenue at Bow Street/Webster Avenue and Washington Street including revised traffic signal timing and phasing are proposed to be implemented in the near future. Therefore, no transportation specific mitigation has been proposed as part of this project. The Transportation Demand Management section outlines the commitments of the project to help reduce the use of single occupancy vehicles traveling to and from the project site.

## TRANSPORTATION DEMAND MANAGEMENT

As part of the proposed project, Liberty Cannabis will implement a Transportation Demand Management (TDM) program to encourage the use of alternative modes of transportation and reduce single occupancy vehicles trips to the site. This TDM program would include elements such as those listed below:

- Establish TDM Coordinator Liberty Cannabis would establish a TDM coordinator to oversee site-related transportation demand management and to provide a commuter information center within the building to assist employees, including posting bus schedules for the routes serving the Union Square area in a central employee area.
- Truck Deliveries Truck deliveries and service vehicles will serve the site during the off-peak
  hours whenever possible to minimize the impacts to traffic operations on the surrounding
  roadways and intersections. The on-site parking area will continue to be utilized for all
  deliveries, minimizing the impacts of vehicle loading and unloading.
- Public Transit Bus schedules for the nearby MBTA bus routes will be provided in a central employee area.
- Bicycle Storage In addition to the existing bicycle racks within Union Square, racks
  providing parking for a total of four bicycles are proposed to be constructed on site. One rack
  will be located directly adjacent to the main entrance to the building and is intended to
  provide parking for two customer's bicycles. An additional rack with capacity for two bicycles
  will be provided for employee within the parking area located behind the fence.
- Local Hiring To minimize the total commuting distance of employees and reduce barriers to alternative transportation modes, as many of the new employees hired to provide the expanded use at the site will be located in Somerville or surrounding communities as possible.
- Appointment Purchases The appointment-only nature of the proposed dispensary will limit total travel to and from the site.

## **CONCLUSIONS**

The proposed project includes the conversion of the existing medical-only Liberty Cannabis dispensary located at 304 Somerville Avenue in Union Square, Somerville, into an adult retail sales dispensary. The proposed project would maintain the existing building footprint and current appointment-only operations and add five additional points of sale. The existing access points to the site would not be changed.

Based on the analysis presented in this transportation impact study, the proposed project is estimated to generate approximately one new vehicle trip (one entering trip and no exiting trips) during the weekday morning peak hour, approximately 16 new vehicle trips (eight entering vehicles and eight exiting vehicles) during the weekday afternoon peak hour, and approximately 16 new trips (nine entering vehicles and seven exiting vehicles) during the Saturday midday peak hour.

The capacity analysis indicates that the proposed mixed-use development is projected to have a negligible impact on the operations of the study area intersections. At the signalized intersection of Somerville Avenue at Bow Street/Webster Avenue and Washington Street, overall intersection operations are shown to be maintained at overall LOS E for all peak hours analyzed under 2022 Existing, 2022 Build and 2027 Design Year Build conditions. The key movements at all other study area intersections are projected to continue operating at the same LOS as under 2022 Existing conditions for the 2022 Build and 2027 Design Year Build conditions for all peak hours analyzed.

The site is well served by the existing pedestrian, bicycle, and public transit infrastructure of Union Square. The small number of trips anticipated to travel via each of these modes are anticipated to have a negligible impact on the transportation network within the study area.

The available sight distance at the existing site driveway would not be impacted as part of the proposed redevelopment. The sight lines at the existing site driveway meets the required sight distances for both the speed limit and 85<sup>th</sup> percentile speed on Somerville Avenue allowing for safe and efficient access to and from the site.

Based on a review of the analysis contained within this transportation impact study, the proposed site redevelopment is not shown to have a significant impact on the overall transportation operations of the study area intersections and roadways.



# Appendix for Transportation Impact Study Liberty Cannabis

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120 Water Street, 4<sup>th</sup> Floor
Boston, MA 02109
617.556.0020

Prepared for Holistic Industries, Inc.

March 2022

## **APPENDIX A**

Traffic Count Data

S: #304 Liberty Cannabis Site Trip Gen E/W: Somerville Ave. (Peds by Direction) City, State: Somerville, MA Client: McM/Zoe Dickerson

File Name: 05513A Site Code : Y1920812 Start Date : 1/27/2022

Groups Printed- Cars & Peds - Trucks & Buses - Bikes by Direction
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				Gro				Trucks		- Bikes by				*11			1
					S	omerville			#304	Liberty C		Site	S		e Avenue	•	
		From I	North			From				From S	outh			From	West		
Start Time	Right	Thru	Left	Peds	Right	Thru	Left	Ped WB SSW		Exit		Enter	Right	Thru	Left	Ped EB SSW	Int. Total
06:00 AM	0	0	0	0	0	0	0	1	0	0	0	0	0	40	0	0	41
06:15 AM	0	0	0	0	0	0	0	2	0	0	0	0	0	42	0	1	45
06:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	47	0	2	49
06:45 AM	0	0	0	0	0	0	0	2	0	0	0	0	0	69	0	0	71
Total	0	0	0	0	0	0	0	5	0	0	0	0	0	198	0	3	206
07.00.135	۱ ۵				۱ ۵							ا م		0.4			l 0 <b>=</b>
07:00 AM	0	0	0	0	0	0	0	4	0	0	0	0	0	81	0	2	87
07:15 AM	0	0	0	0	0	0	0	4	0	0	0	0	0	121	0	3	128
07:30 AM	0	0	0	0	0	0	0	4	0	0	0	0	0	137	0	4	145
07:45 AM	0	0	0	0	0	0	0	10	0	0	0	0	0	178	0	7	195
Total	0	Ü	0	0	0	0	Ü	22	0	0	0	0	0	517	0	16	555
08:00 AM	0	0	0	0	0	0	0	7	0	0	0	1	0	194	0	4	206
08:15 AM	0	0	0	0	0	0	0	5	0	0	0	0	0	195	0	4	204
08:30 AM	0	0	0	0	0	1	0	7	0	0	0	0	0	176	0	3	187
08:45 AM	0	0	0	0	0	0	0	2	0	0	0	0	0	153	0	5	160
Total	0	0	0	0	0	1	0	21	0	0	0	1	0	718	0	16	757
00.00.435	1 0	0			1 0	0	0	2	1 0					1.10	0	-	1.50
09:00 AM	0	0	0	0	0	0	0	3	0	0	0	1	0	143	0	5	152
09:15 AM	0	0	0	0	0	0	0	4	0	0	0	0	0	150	0	4	158
09:30 AM	0	0	0	0	0	0	0	5	0	0	0	0	0	139	0	3	147
09:45 AM	0	0	0	0	0	0	0	3	0	0	0	0	0	125	0	2	130
Total	0	0	0	0	0	0	0	15	0	0	0	1	0	557	0	14	587
10:00 AM	0	0	0	0	0	0	0	2	0	0	0	1	0	125	0	4	132
10:15 AM	0	0	0	0	0	0	0	3	0	0	0	0	0	130	0	3	136
10:30 AM	0	0	0	0	0	0	0	2	0	3	0	4	0	108	0	3	120
10:45 AM	0	0	0	0	0	0	0	5	0	2	0	0	0	141	0	7	155
Total	0	0	0	0	0	0	0	12	0		0	5	0	504	0	17	543
	, ,				, ,	-				-		- 1					
11:00 AM	0	0	0	0	0	0	0	8	0	1	0	1	0	147	0	5	162
11:15 AM	0	0	0	0	0	0	0	2	0	0	0	1	0	133	0	4	140
11:30 AM	0	0	0	0	0	0	0	5	0	0	0	0	0	134	0	3	142
11:45 AM	0	0	0	0	0	0	0	3	0	3	0	5	0	163	0	6	180
Total	0	0	0	0	0	0	0	18	0	4	0	7	0	577	0	18	624
12 00 PM	1 0	0			1 0	0	0		1 0	2		ء ا		151	0	2	1.55
12:00 PM	0	0	0	0	0	0	0	8	0	2	0	2	0	151	0	2	165
12:15 PM	0	0	0	0	0	0	0	6	0	0	0	1	0	156	0	6	169
12:30 PM	0	0	0	0	0	0	0	5	0	2	0	1	0	154	0	11	173
12:45 PM	0	0	0	0	0	0	0	6	0	1	0	1	0	166	0	3	177
Total	0	0	0	0	0	0	0	25	0	5	0	5	0	627	0	22	684
01:00 PM	0	0	0	0	0	0	0	4	0	1	0	1	0	141	0	5	152
01:15 PM	0	0	0	0	0	0	0	4	0	0	0	0	0	157	0	7	168
01:30 PM	0	0	0	0	0	0	0	4	0	1	0	2	0	130	0	4	141
01:45 PM	0	0	0	0	0	0	0	8	0	4	0	2	0	153	0	8	175
Total	0	0	0	0	0	0	0	20	0	6	0	5	0	581	0	24	636
02.00 53.5	1 6	0			1 6		0		1 0	2	0	2 1		160	_	1.1	100
02:00 PM	0	0	0	0	0	0	0	6	0	3	0	2	0	160	0	11	182
02:15 PM	0	0	0	0	0	0	0	8	0	1	0	1	0	174	0	5	189
02:30 PM	0	0	0	0	0	0	0	4	0	2	0	2	0	170	0	10	188
02:45 PM	0	0	0	0	0	0	0	15	0	1	0	1	0	184	0	3	204
Total	0	0	0	0	0	0	0	33	0	7	0	6	0	688	0	29	763

S: #304 Liberty Cannabis Site Trip Gen E/W: Somerville Ave. (Peds by Direction) City, State: Somerville, MA Client: McM/Zoe Dickerson

Site Code : Y1920812 Start Date : 1/27/2022 Page No : 2

File Name: 05513A

Groups Printed- Cars & Peds - Trucks & Buses - Bikes by Direction

	1			Gre						- Bikes by							1
	Somerville Avenue								#304	Liberty C		Site	S		e Avenue	•	
		From	North			From	East			From S	South			From	West	ı	
Start Time	Right	Thru	Left	Peds	Right	Thru	Left	Ped WB SSW		Exit		Enter	Right	Thru	Left	Ped EB SSW	Int. Total
03:00 PM	0	0	0	0	0	0	0	13	0	2	0	3	0	166	0	8	192
03:15 PM	0	0	0	0	0	0	0	7	0	3	0	3	0	160	0	5	178
03:30 PM	0	0	0	0	0	0	0	9	0	0	0	0	0	170	0	7	186
03:45 PM	0	0	0	0	0	1	0	12	0	1	0	0	0	170	0	8	192
Total	0	0	0	0	0	1	0	41	0	6	0	6	0	666	0	28	748
04:00 PM	0	0	0	0	0	0	0	11	0	2	0	3	0	161	0	12	189
04:15 PM	0	0	0	0	0	0	0	12	0	1	0	1	0	180	0	9	203
04:30 PM	0	0	0	0	0	0	0	8	0	1	0	2	0	147	0	7	165
04:45 PM	0	0	0	0	0	0	0	6	0	1	0	1	0	172	0	8	188
Total	0	0	0	0	0	0	0	37	0	5	0	7	0	660	0	36	745
05:00 PM	0	0	0	0	0	0	0	7	0	3	0	2	0	182	0	4	198
05:15 PM	0	0	0	0	0	2	0	11	0	4	0	3	0	190	0	7	217
05:30 PM	0	0	0	0	0	0	0	12	0	6	0	3	0	192	0	10	223
05:45 PM	0	0	0	0	0	0	0	8	0	1	0	1	0	201	0	6	217
Total	0	0	0	0	0	2	0	38	0	14	0	9	0	765	0	27	855
06:00 PM	0	0	0	0	0	0	0	8	0	0	0	0	0	165	0	8	181
06:15 PM	0	0	0	0	0	0	0	4	0	1	0	1	0	155	0	10	171
06:30 PM	0	0	0	0	0	0	0	3	0	2	0	2	0	131	0	10	148
06:45 PM	0	0	0	0	0	0	0	7	0	2	0	3	0	164	0	9	185
Total	0	0	0	0	0	0	0	22	0	5	0	6	0	615	0	37	685
07:00 PM	0	0	0	0	0	0	0	8	0	1	0	6	0	167	0	6	188
07:15 PM	0	0	0	0	0	0	0	13	0	3	0	0	0	134	0	5	155
07:30 PM	0	0	0	0	0	0	0	12	0	1	0	1	0	115	0	9	138
07:45 PM	0	0	0	0	0	0	0	5	0	3	0	2	0	80	0	6	96
Total	0	0	0	0	0	0	0	38	0	8	0	9	0	496	0	26	577
Grand Total	0	0	0	0	0	4	0	347	0	65	0	67	0	8169	0	313	8965
Apprch %	0	0	0	0	0	1.1	0	98.9	0	49.2	0	50.8	0	96.3	0	3.7	
Total %	0	0	0	0	0	0	0	3.9	0	0.7	0	0.7	0	91.1	0	3.5	
Cars & Peds	0	0	0	0	0	0	0	347	0	65	0	67	0	7732	0	313	8524
% Cars & Peds	0	0	0	0	0	0	0	100	0	100	0	100	0	94.7	0	100	95.1
Trucks & Buses	0	0	0	0	0	0	0	0	0	0	0	0	0	205	0	0	205
% Trucks & Buses	0	0	0	0	0	0	0	0	0	0	0	0	0	2.5	0	0	2.3
Bikes by Direction	0	0	0	0	0	4	0	0	0	0	0	0	0	232	0	0	236
% Bikes by Direction	0	0	0	0	0	100	0	0	0	0	0	0	0	2.8	0	0	2.6

						Somerville Avenue				#30	04 Libe	rty Car	nabis S	ite		Some	erville A	venue			
		Fı	rom No	rth			F	rom Ea	st			Fr	om So	uth			F	rom We	est		
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Ped WB SSW	App. Total		Exit		Enter	App. Total	Right	Thru	Left	Ped EB SSW	App. Total	Int. Total
Peak Hour Ana	alysis F	rom 06:	:00 AM	I to 12:	45 PM - I	Peak 1 c	of 1														
Peak Hour for	Entire I	Intersec	tion Be	gins at	07:45 AN	Л															
07:45 AM	0	0	0	0	0	0	0	0	10	10	0	0	0	0	0	0	178	0	7	185	195
08:00 AM	0	0	0	0	0	0	0	0	7	7	0	0	0	1	1	0	194	0	4	198	206
08:15 AM	0	0	0	0	0	0	0	0	5	5	0	0	0	0	0	0	195	0	4	199	204
08:30 AM	0	0	0	0	0	0	1	0	7	8	0	0	0	0	0	0	176	0	3	179	187
Total Volume	0	0	0	0	0	0	1	0	29	30	0	0	0	1	1	0	743	0	18	761	792
% App. Total	0	0	0	0		0	3.3	0	96.7		0	0	0	100		0	97.6	0	2.4		
PHF	.000	.000	.000	.000	.000	.000	.250	.000	.725	.750	.000	.000	.000	.250	.250	.000	.953	.000	.643	.956	.961
Cars & Peds	0	0	0	0	0	0	0	0	29	29	0	0	0	1	1	0	687	0	18	705	735
% Cars & Peds																					l
Trucks & Buses	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	18	0	0	18	18
% Trucks & Buses	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2.4	0	0	2.4	2.3
Bikes by Direction	0	0	0	0	0	0	1	0	0	1	0	0	0	0	0	0	38	0	0	38	39
% Bikes by Direction	0	0	0	0	0	0	100	0	0	3.3	0	0	0	0	0	0	5.1	0	0	5.0	4.9

Peak Hour Analysis From 01:00 PM to 07:45 PM - Peak 1 of 1

Peak Hour for	Entire	Interse	ction B	egins a	t 05:00 F	PM															
05:00 PM	0	0	0	0	0	0	0	0	7	7	0	3	0	2	5	0	182	0	4	186	198
05:15 PM	0	0	0	0	0	0	2	0	11	13	0	4	0	3	7	0	190	0	7	197	217
05:30 PM	0	0	0	0	0	0	0	0	12	12	0	6	0	3	9	0	192	0	10	202	223
05:45 PM	0	0	0	0	0	0	0	0	8	8	0	1	0	1	2	0	201	0	6	207	217
Total Volume	0	0	0	0	0	0	2	0	38	40	0	14	0	9	23	0	765	0	27	792	855
% App. Total	0	0	0	0		0	5	0	95		0	60.9	0	39.1		0	96.6	0	3.4		
PHF	.000	.000	.000	.000	.000	.000	.250	.000	.792	.769	.000	.583	.000	.750	.639	.000	.951	.000	.675	.957	.959
Cars & Peds	0	0	0	0	0	0	0	0	38	38	0	14	0	9	23	0	735	0	27	762	823
% Cars & Peds																					
Trucks & Buses	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	12	0	0	12	12
% Trucks & Buses	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1.6	0	0	1.5	1.4
Bikes by Direction	0	0	0	0	0	0	2	0	0	2	0	0	0	0	0	0	18	0	0	18	20
% Bikes by Direction	0	0	0	0	0	0	100	0	0	5.0	0	0	0	0	0	0	2.4	0	0	2.3	2.3

S: #304 Liberty Cannabis Site Trip Gen E/W: Somerville Ave. (Peds by Direction) City, State: Somerville, MA Client: McM/Zoe Dickerson

File Name: 05513A Site Code : Y1920812 Start Date : 1/27/2022

Groups	Printed-	Cars	&	Peds	

		г .	T .1		So	omerville	#304 I	Liberty Ca		Site	S	omerville		е			
		From N				From I		Ped WB		From So	outh			From V			
Start Time	Right	Thru	Left	Peds	Right	Thru	Left	SSW		Exit		Enter	Right	Thru	Left	Ped EB SSW	Int. Total
06:00 AM	0	0	0	0	0	0	0	1	0	0	0	0	0	39	0	0	40
06:15 AM	0	0	0	0	0	0	0	2	0	0	0	0	0	37	0	1	40
06:30 AM 06:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	43	0	2	45
	0	0	0	0	0	0	0	5	0	0	0	0	0	183	0	3	66 191
Total	U	U	U	U	0	U	U	3	U	U	U	0	U	165	U	3	191
07:00 AM	0	0	0	0	0	0	0	4	0	0	0	0	0	75	0	2	81
07:15 AM	0	0	0	0	0	0	0	4	0	0	0	0	0	108	0	3	115
07:30 AM	0	0	0	0	0	0	0	4	0	0	0	0	0	131	0	4	139
07:45 AM	0	0	0	0	0	0	0	10	0	0	0	0	0	164	0	7	181
Total	0	0	0	0	0	0	0	22	0	0	0	0	0	478	0	16	516
																	ı
08:00 AM	0	0	0	0	0	0	0	7	0	0	0	1	0	181	0	4	193
08:15 AM	0	0	0	0	0	0	0	5	0	0	0	0	0	181	0	4	190
08:30 AM	0	0	0	0	0	0	0	7	0	0	0	0	0	161	0	3	171
08:45 AM	0	0	0	0	0	0	0	2	0	0	0	0	0	141	0	5	148
Total	0	0	0	0	0	0	0	21	0	0	0	1	0	664	0	16	702
09:00 AM	0	0	0	0	0	0	0	3	0	0	0	1	0	131	0	5	140
09:15 AM	0	0	0	0	0	0	0	4	0	0	0	0	0	134	0	4	142
09:30 AM	0	0	0	0	0	0	0	5	0	0	0	0	0	129	0	3	137
09:45 AM	0	0	0	0	0	0	0	3	0	0	0	0	0	118	0	2	123
Total	0	0	0	0	0	0	0	15	0	0	0	1	0	512	0	14	542
10:00 AM	0	0	0	0	0	0	0	2	0	0	0	1	0	114	0	4	121
10:15 AM	0	0	0	0	0	0	0	3	0	0	0	0	0	118	0	3	124
10:30 AM	0	0	0	0	0	0	0	2	0	3	0	4	0	103	0	3	115
10:45 AM	0	0	0	0	0	0	0	5	0	2	0	0	0	128	0	7	142
Total	0	0	0	0	0	0	0	12	0	5	0	5	0	463	0	17	502
11:00 AM	0	0	0	0	0	0	0	8	0	1	0	1	0	136	0	5	151
11:15 AM	0	0	0	0	0	0	0	2	0	0	0	1	0	129	0	4	136
11:30 AM	0	0	0	0	0	0	0	5	0	0	0	0	0	126	0	3	134
11:45 AM	0	0	0	0	0	0	0	3	0	3	0	5	0	157	0	6	174
Total	0	0	0	0	0	0	0	18	0	4	0	7	0	548	0	18	595
12:00 PM	0	0	0	0	0	0	0	8	0	2	0	2	0	138	0	2	152
12:15 PM	0	0	0	0	0	0	0	6	0	0	0	1	0	149	0	6	162
12:30 PM	0	0	0	0	0	0	0	5	0	2	0	1	0	149	0	11	168
12:45 PM	0	0	0	0	0	0	0	6	0	1	0	1	0	158	0	3	169
Total	0	0	0	0	0	0	0	25	0	5	0	5	0	594	0	22	651
01:00 PM	0	0	0	0	0	0	0	4	0	1	0	1	0	134	0	5	145
01:00 I M	0	0	0	0	0	0	0	4	0	0	0	0	0	152	0	7	163
01:30 PM	0	0	0	0	0	0	0	4	0	1	0	2	0	121	0	4	132
01:45 PM	0	0	0	0	0	0	0	8	0	4	0	2	0	149	0	8	171
Total	0	0	0	0	0	0	0	20	0	6	0	5	0	556	0	24	611
																	1
02:00 PM	0	0	0	0	0	0	0	6	0	3	0	2	0	152	0	11	174
02:15 PM	0	0	0	0	0	0	0	8	0	1	0	1	0	167	0	5	182
02:30 PM	0	0	0	0	0	0	0	4	0	2	0	2	0	160	0	10	178
02:45 PM	0	0	0	0	0	0	0	15	0	1	0	1	0	173	0	3	193
Total	0	0	0	0	0	0	0	33	0	7	0	6	0	652	0	29	727

S: #304 Liberty Cannabis Site Trip Gen E/W: Somerville Ave. (Peds by Direction) City, State: Somerville, MA Client: McM/Zoe Dickerson

File Name: 05513A Site Code : Y1920812 Start Date : 1/27/2022

							noups Pi	inteu- C									1
					S	omerville			#304	Liberty Ca		Site	S		e Avenue	•	
		From 1	North			From 1	East			From S	outh			From	West		
Start Time	Right	Thru	Left	Peds	Right	Thru	Left	Ped WB SSW		Exit		Enter	Right	Thru	Left	Ped EB SSW	Int. Total
03:00 PM	0	0	0	0	0	0	0	13	0	2	0	3	0	161	0	8	187
03:15 PM	0	0	0	0	0	0	0	7	0	3	0	3	0	155	0	5	173
03:30 PM	0	0	0	0	0	0	0	9	0	0	0	0	0	161	0	7	177
03:45 PM		0	0	0	0	0	0	12	0	1	0	0	0	159	0	8	180
Tota	1 0	0	0	0	0	0	0	41	0	6	0	6	0	636	0	28	717
04:00 PM		0	0	0	0	0	0	11	0	2	0	3	0	157	0	12	185
04:15 PM		0	0	0	0	0	0	12	0	1	0	1	0	173	0	9	196
04:30 PM		0	0	0	0	0	0	8	0	1	0	2	0	142	0	7	160
04:45 PM		0	0	0	0	0	0	6	0	1	0	1	0	165	0	8	181
Tota	1 0	0	0	0	0	0	0	37	0	5	0	7	0	637	0	36	722
05:00 PM		0	0	0	0	0	0	7	0	3	0	2	0	176	0	4	192
05:15 PM		0	0	0	0	0	0	11	0	4	0	3	0	184	0	7	209
05:30 PM		0	0	0	0	0	0	12	0	6	0	3	0	184	0	10	215
05:45 PM		0	0	0	0	0	0	8	0	1	0	1	0	191	0	6	207
Tota	1 0	0	0	0	0	0	0	38	0	14	0	9	0	735	0	27	823
06:00 PM		0	0	0	0	0	0	8	0	0	0	0	0	159	0	8	175
06:15 PM		0	0	0	0	0	0	4	0	1	0	1	0	151	0	10	167
06:30 PM		0	0	0	0	0	0	3	0	2	0	2	0	123	0	10	140
06:45 PM		0	0_	0	0	0	0	7	0	2	0	3	0	158	0	9	179
Tota	1 0	0	0	0	0	0	0	22	0	5	0	6	0	591	0	37	661
07:00 PM		0	0	0	0	0	0	8	0	1	0	6	0	163	0	6	184
07:15 PM		0	0	0	0	0	0	13	0	3	0	0	0	129	0	5	150
07:30 PM		0	0	0	0	0	0	12	0	1	0	1	0	114	0	9	137
07:45 PM		0	0	0	0	0	0	5	0	. 3	0	2	0	77	0	6	93
Tota	1 0	0	0	0	0	0	0	38	0	8	0	9	0	483	0	26	564
Grand Total		0	0	0	0	0	0	347	0	65	0	67	0	7732	0	313	8524
Apprch %		0	0	0	0	0	0	100	0	49.2	0	50.8	0	96.1	0	3.9	
Total %	0	0	0	0	0	0	0	4.1	0	0.8	0	0.8	0	90.7	0	3.7	

		Fi	rom No	orth				rville <i>A</i> From Ea			#3		rty Car	nabis S uth	ite			rville A			
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Ped WB SSW	App. Total		Exit		Enter	App. Total	Right	Thru	Left	Ped EB SSW	App. Total	Int. Total
Peak Hour An	alysis F	rom 06	:00 AM	I to 12:	45 PM - I	Peak 1	of 1														
Peak Hour for	Entire 1	Intersec	tion Be	gins at	07:45 Al	M															
07:45 AM	0	0	0	0	0	0	0	0	10	10	0	0	0	0	0	0	164	0	7	171	181
08:00 AM	0	0	0	0	0	0	0	0	7	7	0	0	0	1	1	0	181	0	4	185	193
08:15 AM	0	0	0	0	0	0	0	0	5	5	0	0	0	0	0	0	181	0	4	185	190
08:30 AM	0	0	0	0	0	0	0	0	7	7	0	0	0	0	0	0	161	0	3	164	171
Total Volume	0	0	0	0	0	0	0	0	29	29	0	0	0	1	1	0	687	0	18	705	735
% App. Total	0	0	0	0		0	0	0	100		0	0	0	100		0	97.4	0	2.6		
PHF	.000	.000	.000	.000	.000	.000	.000	.000	.725	.725	.000	.000	.000	.250	.250	.000	.949	.000	.643	.953	.952

S: #304 Liberty Cannabis Site Trip Gen E/W: Somerville Ave. (Peds by Direction) City, State: Somerville, MA Client: McM/Zoe Dickerson

File Name: 05513A Site Code : Y1920812 Start Date : 1/27/2022

		Fı	rom No	ırth				rville A			#30	)4 Libe	rty Car		Site			rville A			
Start Time	Right	Thru	Left	Peds	I	Right	Thru	Left	Ped WB			Exit	OIII SO	Enter		Diaht	Thru	Left	Ped EB		I . T . 1
Start Time	Kigiii	Tillu	Len	Peus	App. Total	Kigiii	Tillu	Len	SSW	App. Total		EXIL		Elitei	App. Total	Right	Tillu	Leit	SSW	App. Total	Int. Total
Peak Hour An	alysis F	rom 01	:00 PM	to 07:4	45 PM - F	Peak 1 c	of 1														
Peak Hour for	Entire 1	Intersec	tion Be	gins at	05:00 PN	Л															
05:00 PM	0	0	0	0	0	0	0	0	7	7	0	3	0	2	5	0	176	0	4	180	192
05:15 PM	0	0	0	0	0	0	0	0	11	11	0	4	0	3	7	0	184	0	7	191	209
05:30 PM	0	0	0	0	0	0	0	0	12	12	0	6	0	3	9	0	184	0	10	194	215
05:45 PM	0	0	0	0	0	0	0	0	8	8	0	1	0	1	2	0	191	0	6	197	207
Total Volume	0	0	0	0	0	0	0	0	38	38	0	14	0	9	23	0	735	0	27	762	823
% App. Total	0	0	0	0		0	0	0	100		0	60.9	0	39.1		0	96.5	0	3.5		
PHF	.000	.000	.000	.000	.000	.000	.000	.000	.792	.792	.000	.583	.000	.750	.639	.000	.962	.000	.675	.967	.957

S: #304 Liberty Cannabis Site Trip Gen E/W: Somerville Ave. (Peds by Direction) City, State: Somerville, MA Client: McM/Zoe Dickerson

File Name: 05513A Site Code : Y1920812 Start Date : 1/27/2022

Groups Printed-	Trucks &	Buses
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					So	omerville			#304 1	Liberty Ca		Site	S	omerville		e	
		From N	North			From I	East			From S	outh			From	West		
Start Time	Right	Thru	Left	Peds	Right	Thru	Left	Ped WB SSW		Exit		Enter	Right	Thru	Left	Ped EB SSW	Int. Total
06:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	1
06:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	3	0	0	3
06:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	3	0	0	3
06:45 AM	0	0	0	0	0	0	0_	0	0	0	0	0	0	3	0	0	3_
Total	0	0	0	0	0	0	0	0	0	0	0	0	0	10	0	0	10
	1 .				1 .							. 1					1 -
07:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	3	0	0	3
07:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	6	0	0	6
07:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	0	2
07:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	6 17	0	0	6
Total	0	U	U	U	0	U	U	U	U	U	U	0	U	17	U	0	17
08:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	3	0	0	3
08:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	4	0	0	4
08:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	5	0	0	5
08:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	0	2
Total	0	0	0	0	0	0	0	0	0	0	0	0	0	14	0	0	14
Total	, 0	U	U	U	, 0	U	U	0	U	U	U	0	U	14	U	U	14
09:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	0	2
09:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	8	0	0	8
09:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	6	0	0	6
09:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	4	0	0	4
Total	0	0	0	0	0	0	0	0	0	0	0	0	0	20	0	0	20
	'				'												
10:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	6	0	0	6
10:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	7	0	0	7
10:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	0	2
10:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	9	0	0	9
Total	0	0	0	0	0	0	0	0	0	0	0	0	0	24	0	0	24
	1 .				1 .							. 1					1 _
11:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	7	0	0	7
11:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	1
11:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	5	0	0	5
11:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	0	2
Total	0	0	0	0	0	0	0	0	0	0	0	0	0	15	0	0	15
12:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	6	0	0	6
12:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	3	0	0	3
12:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	3	0	0	3
12:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	5	0	0	5
Total	0	0	0	0	0	0	0	0	0	0	0	0	0	17	0	0	17
	, ,		-				-		_			- 1				_	
01:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	6	0	0	6
01:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	4	0	0	4
01:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	6	0	0	6
01:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	4	0	0	4_
Total	0	0	0	0	0	0	0	0	0	0	0	0	0	20	0	0	20
	ı				ı							ı					ı
02:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	6	0	0	6
02:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	5	0	0	5
02:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	5	0	0	5
02:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	8	0	0	8
Total	0	0	0	0	0	0	0	0	0	0	0	0	0	24	0	0	24

S: #304 Liberty Cannabis Site Trip Gen E/W: Somerville Ave. (Peds by Direction) City, State: Somerville, MA Client: McM/Zoe Dickerson

File Name: 05513A Site Code : Y1920812 Start Date : 1/27/2022

						2	omerville	Oups PHII Avenue	ileu- IIu		Liberty C	'annahis	Site	2	omerville	e Avenue	a .	
			From I	North			From			11304	From S		Dite		From			
	Start Time	Right	Thru	Left	Peds	Right	Thru	Left	Ped WB SSW		Exit		Enter	Right	Thru	Left	Ped EB SSW	Int. Total
	03:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	0	2
	03:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	1
	03:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	4	0	0	4
	03:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	4	0	0	4
	Total	0	0	0	0	0	0	0	0	0	0	0	0	0	11	0	0	11
	04:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	0	2
	04:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	3	0	0	3
	04:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	0	2
	04:45 PM	0	0	0	0	0	0_	0	0	0	0	0	0	0	1	0	0	11
	Total	0	0	0	0	0	0	0	0	0	0	0	0	0	8	0	0	8
	05:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	0	2
	05:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	3	0	0	3
	05:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	0	2
	05:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	5	0	0	5
	Total	0	0	0	0	0	0	0	0	0	0	0	0	0	12	0	0	12
	06:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	0	2
	06:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	1
	06:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	4	0	0	4
	06:45 PM	0	0	0	0	0	0_	0	0	0	0	0	0	0	1_	0	0	11_
	Total	0	0	0	0	0	0	0	0	0	0	0	0	0	8	0	0	8
	07:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	0	2
	07:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	3	0	0	3
	07:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	07:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	Total	0	0	0	0	0	0	0	0	0	0	0	0	0	5	0	0	5
C	Grand Total	0	0	0	0	0	0	0	0	0	0	0	0	0	205	0	0	205
	Apprch %	0	0	0	0	0	0	0	0	0	0	0	0	0	100	0	0	
	Total %	0	0	0	0	0	0	0	0	0	0	0	0	0	100	0	0	

		Fi	om No	rth				rville <i>A</i> From Ea			#30		rty Car	nabis S	ite			erville A			
Start Time	Right	Thru	Left		App. Total	Right	Thru	Left	Ped WB	App. Total		Exit	om so	Enter	App. Total	Right	Thru	Left	Ped EB	App. Total	Int. Total
Peak Hour An	alysis F	rom 06	:00 AM	I to 12:	45 PM - I	Peak 1	of 1		3511										5511		
Peak Hour for	Entire 1	Intersec	tion Be	gins at	10:15 AM	M															
10:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	7	0	0	7	7
10:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	0	2	2
10:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	9	0	0	9	9
11:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	7	0	0	7	7
Total Volume	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	25	0	0	25	25
% App. Total	0	0	0	0		0	0	0	0		0	0	0	0		0	100	0	0		
PHF	000	.000	.000	.000	.000	.000	.000	000	.000	.000	.000	.000	.000	.000	.000	000	694	000	.000	694	694

S: #304 Liberty Cannabis Site Trip Gen E/W: Somerville Ave. (Peds by Direction) City, State: Somerville, MA Client: McM/Zoe Dickerson

File Name: 05513A Site Code : Y1920812 Start Date : 1/27/2022

		E.	rom No	erth				rville A	venue		#30		rty Car	nabis S	Site			rville A			
		1.1	OIII INC	nui			I.	TOIII Le	ısı			1.1	0111 50	uui			1	TOTTI W	est		
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Ped WB SSW	App. Total		Exit		Enter	App. Total	Right	Thru	Left	Ped EB SSW	App. Total	Int. Total
Peak Hour An	alysis F	rom 01	:00 PM	to 07:4	45 PM - F	Peak 1 o	f 1														
Peak Hour for	Entire 1	Intersec	tion Be	gins at	02:00 PN	1															
02:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	6	0	0	6	6
02:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	5	0	0	5	5
02:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	5	0	0	5	5
02:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	8	0	0	8	8
Total Volume	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	24	0	0	24	24
% App. Total	0	0	0	0		0	0	0	0		0	0	0	0		0	100	0	0		
PHF	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.750	.000	.000	.750	.750

S: #304 Liberty Cannabis Site Trip Gen E/W: Somerville Ave. (Peds by Direction) City, State: Somerville, MA Client: McM/Zoe Dickerson

File Name: 05513A Site Code : Y1920812 Start Date : 1/27/2022

Groups Printed-	Bikes by	Direction
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	1							ed- Bike									i
					S	omerville		:	#304 ]	Liberty Ca		Site	So	omerville		е	
		From	North			From I	East			From So	outh			From	West		
Start Time	Right	Thru	Left	Peds	Right	Thru	Left	Ped WB SSW		Exit		Enter	Right	Thru	Left	Ped EB SSW	Int. Total
06:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
06:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	0	2
06:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	1
 06:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	0	22
Total	0	0	0	0	0	0	0	0	0	0	0	0	0	5	0	0	5
07:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	3	0	0	3
07:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	7	0	0	7
07:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	4	0	0	4
 07:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	8	0	0	8_
Total	0	0	0	0	0	0	0	0	0	0	0	0	0	22	0	0	22
08:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	10	0	0	10
08:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	10	0	0	10
08:30 AM	0	0	0	0	0	1	0	0	0	0	0	0	0	10	0	0	11
 08:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	10	0	0	10
Total	0	0	0	0	0	1	0	0	0	0	0	0	0	40	0	0	41
09:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	10	0	0	10
09:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	8	0	0	8
09:30 AM	ő	0	0	0	0	0	0	0	0	0	0	0	0	4	0	0	4
09:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	3	0	0	3
Total	0	0	0	0	0	0	0	0	0	0	0	0	0	25	0	0	25
40.00.43.5	۱ ۵				۱ ۵									_			
10:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	5	0	0	5
10:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	5	0	0	5
10:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	3	0	0	3
 10:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	4	0	0	4
Total	0	0	0	0	0	0	0	0	0	0	0	0	0	17	0	0	17
11:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	4	0	0	4
11:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	3	0	0	3
11:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	3	0	0	3
11:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	4	0	0	4
 Total	0	0	0	0	0	0	0	0	0	0	0	0	0	14	0	0	14
12:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	7	0	0	7
12:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	4	0	0	4
12:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	0	2
12:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	3	0	0	3
Total	0	0	0	0	0	0	0	0	0	0	0	0	0	16	0	0	16
01:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	1
01:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	1
01:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	3	0	0	3
01:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	0	0	0	0	0	0	0	0	0	0	0	0	0	5	0	0	5
02:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	0	2
02:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	0	2
02:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	5	0	0	5
02:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	3	0	0	3
Total	0	0	0	0	0	0	0	0	0	0	0	0	0	12	0	0	12
1000	,		•	<u> </u>				9			0	٠ ,			9	3	

S: #304 Liberty Cannabis Site Trip Gen E/W: Somerville Ave. (Peds by Direction) City, State: Somerville, MA Client: McM/Zoe Dickerson

File Name: 05513A Site Code : Y1920812 Start Date : 1/27/2022

Groups Printed- Bikes by Direct	ion
---------------------------------	-----

							ips Printe	ea- Bike									7
					S	omerville			#304	Liberty Ca		Site	S	omerville		e	
	<u></u>	From 1	North			From 1	East			From So	outh			From	West		
Start Time	Right	Thru	Left	Peds	Right	Thru	Left	Ped WB SSW		Exit		Enter	Right	Thru	Left	Ped EB SSW	Int. Total
03:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	3	0	0	3
03:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	4	0	0	4
03:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	5	0	0	5
03:45 PM	0	0	0	0	0	1	0	0	0	0	0	0	0	7	0	0	8
Total	0	0	0	0	0	1	0	0	0	0	0	0	0	19	0	0	20
	1 .				1 -												1 .
04:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	0	2
04:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	4	0	0	4
04:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	3	0	0	3
04:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	6	0	0	6
Total	0	0	0	0	0	0	0	0	0	0	0	0	0	15	0	0	15
05.00 73.5	1 0				1 0			ا م									
05:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	4	0	0	4
05:15 PM	0	0	0	0	0	2	0	0	0	0	0	0	0	3	0	0	5
05:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	6	0	0	6
05:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	5	0	0	5
Total	0	0	0	0	0	2	0	0	0	0	0	0	0	18	0	0	20
06:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	4	0	0	4
06:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	3	0	0	3
06:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	4	0	0	4
06:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	5	0	0	5
Total	0	0	0	0	0	0	0	0	0	0	0	0	0	16	0	0	16
Total	1 0	O	Ü	Ü	, 0	Ü	O	0	· ·	O	O	O	Ü	10	Ü	Ü	10
07:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	0	2
07:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	0	2
07:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	1
07:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	3	0	0	3
Total	0	0	0	0	0	0	0	0	0	0	0	0	0	8	0	0	8
	•																•
Grand Total	0	0	0	0	0	4	0	0	0	0	0	0	0	232	0	0	236
Apprch %	0	0	0	0	0	100	0	0	0	0	0	0	0	100	0	0	
Total %	0	0	0	0	0	1.7	0	0	0	0	0	0	0	98.3	0	0	

		Fı	om No	rth				rville <i>A</i> rom Ea			#30		rty Can	nabis S uth	ite			erville A			
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Ped WB SSW	App. Total		Exit		Enter	App. Total	Right	Thru	Left	Ped EB SSW	App. Total	Int. Total
Peak Hour An	alysis F	rom 06	:00 AM	to 12:	45 PM - I	Peak 1 o	of 1														
Peak Hour for	Entire 1	Intersec	tion Be	gins at	08:00 AM	M															
08:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	10	0	0	10	10
08:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	10	0	0	10	10
08:30 AM	0	0	0	0	0	0	1	0	0	1	0	0	0	0	0	0	10	0	0	10	11
08:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	10	0	0	10	10
Total Volume	0	0	0	0	0	0	1	0	0	1	0	0	0	0	0	0	40	0	0	40	41
% App. Total	0	0	0	0		0	100	0	0		0	0	0	0		0	100	0	0		
PHF	.000	.000	.000	.000	.000	.000	.250	.000	.000	.250	.000	.000	.000	.000	.000	.000	1.00	.000	.000	1.00	.932

S: #304 Liberty Cannabis Site Trip Gen E/W: Somerville Ave. (Peds by Direction) City, State: Somerville, MA Client: McM/Zoe Dickerson

File Name: 05513A Site Code : Y1920812 Start Date : 1/27/2022

							Some	rville A	venue		#30	4 Libe	ty Can	nabis S	Site		Some	rville A	venue		
		Fı	rom No	rth			F	rom Ea	st			Fr	om So	uth			F	rom We	est		
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Ped WB SSW	App. Total		Exit		Enter	App. Total	Right	Thru	Left	Ped EB SSW	App. Total	Int. Total
Peak Hour An	alysis F	rom 01	:00 PM	to 07:4	15 PM - F	Peak 1 o	f 1					•									
Peak Hour for	Entire 1	Intersec	tion Be	gins at	04:45 PN	Л															
04:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	6	0	0	6	6
05:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	4	0	0	4	4
05:15 PM	0	0	0	0	0	0	2	0	0	2	0	0	0	0	0	0	3	0	0	3	5
05:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	6	0	0	6	6
Total Volume	0	0	0	0	0	0	2	0	0	2	0	0	0	0	0	0	19	0	0	19	21
% App. Total	0	0	0	0		0	100	0	0		0	0	0	0		0	100	0	0		
PHF	.000	.000	.000	.000	.000	.000	.250	.000	.000	.250	.000	.000	.000	.000	.000	.000	.792	.000	.000	.792	.875

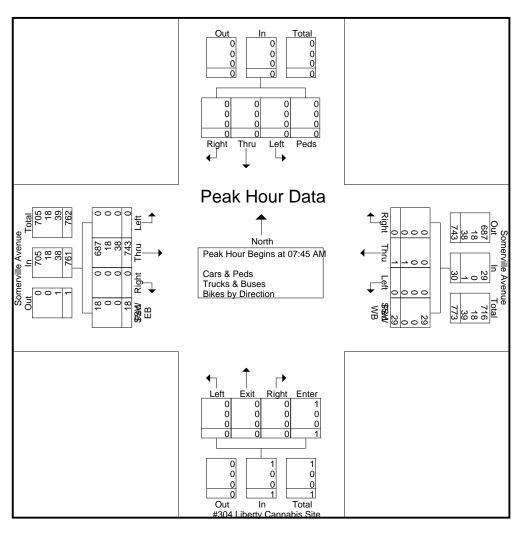
## Transportation Data Corporation

Mario Perone, mperone1@verizon.net tel (781) 587-0086 cell (781) 439-4999

S: #304 Liberty Cannabis Site Trip Gen E/W: Somerville Ave. (Peds by Direction)

City, State: Somerville, MA Client: McM/Zoe Dickerson File Name: 05513A Site Code: Y1920812 Start Date: 1/27/2022

							Some	rville A	venue		#30	)4 Libe	rty Car	nabis S	Site		Some	rville A	venue		
		Fı	om No	rth			F	rom Ea	ıst			Fı	om So	uth			F	rom We	est		
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Ped WB SSW	App. Total		Exit		Enter	App. Total	Right	Thru	Left	Ped EB SSW	App. Total	Int. Total
Peak Hour An	alysis F	rom 06:	:00 AM	to 12:4	45 PM - I	Peak 1 o	of 1														
Peak Hour for	Entire I	ntersec	tion Be	gins at	07:45 Al	M															
07:45 AM	0	0	0	0	0	0	0	0	10	10	0	0	0	0	0	0	178	0	7	185	195
08:00 AM	0	0	0	0	0	0	0	0	7	7	0	0	0	1	1	0	194	0	4	198	206
08:15 AM	0	0	0	0	0	0	0	0	5	5	0	0	0	0	0	0	195	0	4	199	204
08:30 AM	0	0	0	0	0	0	1	0	7	8	0	0	0	0	0	0	176	0	3	179	187
Total Volume	0	0	0	0	0	0	1	0	29	30	0	0	0	1	1	0	743	0	18	761	792
% App. Total	0	0	0	0		0	3.3	0	96.7		0	0	0	100		0	97.6	0	2.4		
PHF	.000	.000	.000	.000	.000	.000	.250	.000	.725	.750	.000	.000	.000	.250	.250	.000	.953	.000	.643	.956	.961_
Cars & Peds	0	0	0	0	0	0	0	0	29	29	0	0	0	1	1	0	687	0	18	705	735
% Cars & Peds																					
Trucks & Buses	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	18	0	0	18	18
% Trucks & Buses	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2.4	0	0	2.4	2.3
Bikes by Direction	0	0	0	0	0	0	1	0	0	1	0	0	0	0	0	0	38	0	0	38	39
% Bikes by Direction	0	0	0	0	0	0	100	0	0	3.3	0	0	0	0	0	0	5.1	0	0	5.0	4.9



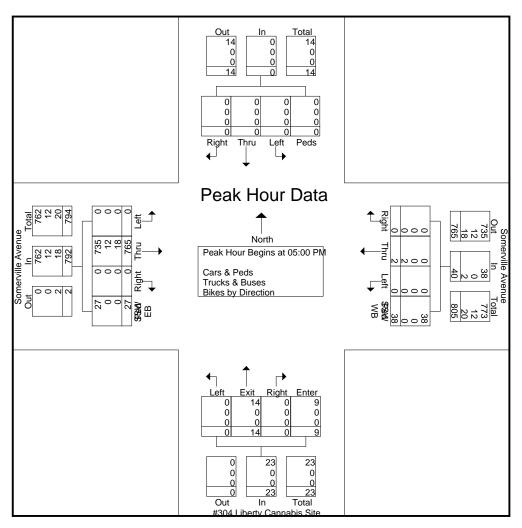
# Transportation Data Corporation

Mario Perone, mperone1@verizon.net tel (781) 587-0086 cell (781) 439-4999

S: #304 Liberty Cannabis Site Trip Gen E/W: Somerville Ave. (Peds by Direction)

City, State: Somerville, MA Client: McM/Zoe Dickerson File Name: 05513A Site Code: Y1920812 Start Date: 1/27/2022

								rville A			#30	04 Libe	rty Car	nabis S	Site		Some	rville A	venue		
		Fr	om No	rth			F	rom Ea	st			Fr	om So	uth			F	rom W	est		
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Ped WB SSW	App. Total		Exit		Enter	App. Total	Right	Thru	Left	Ped EB SSW	App. Total	Int. Total
Peak Hour Ana	alysis F	rom 01:	00 PM	to 07:4	5 PM - F	eak 1 o	f 1														
Peak Hour for	Entire I	ntersec	tion Be	gins at	05:00 PN	1															
05:00 PM	0	0	0	0	0	0	0	0	7	7	0	3	0	2	5	0	182	0	4	186	198
05:15 PM	0	0	0	0	0	0	2	0	11	13	0	4	0	3	7	0	190	0	7	197	217
05:30 PM	0	0	0	0	0	0	0	0	12	12	0	6	0	3	9	0	192	0	10	202	223
05:45 PM	0	0	0	0	0	0	0	0	8	8	0	1	0	1	2	0	201	0	6	207	217
Total Volume	0	0	0	0	0	0	2	0	38	40	0	14	0	9	23	0	765	0	27	792	855
% App. Total	0	0	0	0		0	5	0	95		0	60.9	0	39.1		0	96.6	0	3.4		
PHF	.000	.000	.000	.000	.000	.000	.250	.000	.792	.769	.000	.583	.000	.750	.639	.000	.951	.000	.675	.957	.959
Cars & Peds	0	0	0	0	0	0	0	0	38	38	0	14	0	9	23	0	735	0	27	762	823
% Cars & Peds																					
Trucks & Buses	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	12	0	0	12	12
% Trucks & Buses	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1.6	0	0	1.5	1.4
Bikes by Direction	0	0	0	0	0	0	2	0	0	2	0	0	0	0	0	0	18	0	0	18	20
% Bikes by Direction	0	0	0	0	0	0	100	0	0	5.0	0	0	0	0	0	0	2.4	0	0	2.3	2.3



Client: Zoe Dickerson
Project #: 864\_013\_MM
BTD #: Location 1
Location: Somerville, MA
Street 1: Somerville Avenue/Boy

Street 1: Somerville Avenue/Bow Street Street 2: Webster Avenue/Washington Street

Count Date: 2/12/2022
Day of Week: Saturday
Weather: Clouds & Sun, 40°F



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## PASSENGER CARS & HEAVY VEHICLES COMBINED

			r Avenue bound		Son		enue/Bow Sabound	treet		•	ton Street bound				le Avenue bound	
Start Time	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
10:00 AM	0	4	29	4	0	48	43	10	0	16	23	1	0	3	38	50
10:15 AM	0	2	32	2	0	47	34	2	0	8	43	3	0	9	26	66
10:30 AM	0	8	29	1	0	58	58	3	0	8	26	0	0	4	42	55
10:45 AM	0	6	43	4	0	43	48	6	0	13	47	7	0	11	43	60
11:00 AM	0	2	38	3	0	60	53	7	0	11	38	5	0	6	36	81
11:15 AM	0	5	54	1	0	58	43	5	0	13	41	5	0	4	44	72
11:30 AM	0	6	54	9	0	40	38	8	0	10	36	4	0	8	45	59
11:45 AM	0	3	35	6	0	57	39	6	0	14	52	7	0	9	43	69
12:00 PM	0	4	35	5	0	69	58	13	0	7	42	6	0	7	44	83
12:15 PM	0	6	45	8	0	60	38	7	0	17	55	3	0	11	44	60
12:30 PM	0	5	46	6	0	57	50	10	0	17	44	9	0	5	64	54
12:45 PM	0	6	51	6	0	58	40	17	0	16	52	3	0	7	40	67
1:00 PM	0	4	42	5	0	57	60	8	0	13	53	6	0	6	32	62
1:15 PM	0	6	47	6	0	62	58	15	0	12	41	2	0	8	35	72
1:30 PM	0	8	53	4	0	60	38	19	0	21	57	4	0	6	51	62
1:45 PM	0	3	45	9	0	48	61	9	0	14	51	4	0	11	45	58

MID P	PEAK HOUR		Webster	r Avenue		Son	nerville Ave	nue/Bow St	reet		Washing	ton Street			Somervill	e Avenue	
1:	2:45 PM		North	bound			South	bound			Easth	oound			West	bound	
	to	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
1	1:45 PM	0	24	193	21	0	237	196	59	0	62	203	15	0	27	158	263
	PHF		0.	92			0.	91			0.	85			0.	94	
	HV %	0.0%	0.0%	0.0%	4.8%	0.0%	2.5%	0.0%	0.0%	0.0%	0.0%	4.4%	0.0%	0.0%	3.7%	4.4%	3.0%

Client: Zoe Dickerson
Project #: 864\_013\_MM
BTD #: Location 1
Location: Somerville, MA

Street 1: Somerville Avenue/Bow Street Street 2: Webster Avenue/Washington Street

Count Date: 2/12/2022
Day of Week: Saturday
Weather: Clouds & Sun, 40°F



PO BOX 1723, Framingham, MA 01701 Office: 978-746-1259 DataRequest@BostonTrafficData.com www.BostonTrafficData.com

## **HEAVY VEHICLES**

				r Avenue bound		Son	nerville Ave South	nue/Bow St	treet		·	ton Street bound				e Avenue bound	
S	Start Time	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
•	10:00 AM	0	0	0	0	0	1	0	0	0	0	1	0	0	0	3	1
•	10:15 AM	0	0	1	0	0	4	0	0	0	0	2	0	0	1	3	2
•	10:30 AM	0	0	0	0	0	2	2	0	0	0	0	0	0	0	2	2
•	10:45 AM	0	1	1	0	0	0	0	0	0	0	2	0	0	1	1	0
•	11:00 AM	0	0	0	0	0	1	0	0	0	0	1	0	0	1	0	1
•	11:15 AM	0	0	0	0	0	0	0	0	0	0	3	0	0	0	3	4
	11:30 AM	0	0	0	0	0	2	0	0	0	0	0	0	0	0	1	0
•	11:45 AM	0	0	0	0	0	0	0	0	0	0	1	0	0	1	1	3
	12:00 PM	0	0	0	0	0	2	0	0	0	1	3	0	0	0	1	1
•	12:15 PM	0	0	0	1	0	2	0	0	0	0	1	0	0	1	1	0
•	12:30 PM	0	0	0	0	0	1	0	0	0	0	2	0	0	1	5	2
	12:45 PM	0	0	0	0	0	2	0	0	0	0	2	0	0	0	1	1
	1:00 PM	0	0	0	1	0	2	0	0	0	0	3	0	0	0	2	0
	1:15 PM	0	0	0	0	0	1	0	0	0	0	2	0	0	0	2	5
	1:30 PM	0	0	0	0	0	1	0	0	0	0	2	0	0	1	2	2
	1:45 PM	0	0	0	0	0	0	1	0	0	0	4	0	0	0	2	1

MID PEAK HOUR		Webster	r Avenue		Son	nerville Ave	nue/Bow St	treet		Washing	on Street			Somerville	e Avenue	
12:30 PM		North	bound			South	bound			Easth	ound			Westb	ound	
to	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
1:30 PM	0	0	0	1	0	6	0	0	0	0	9	0	0	1	10	8
PHF		0.	25			0.	75			0.	75			0.9	59	

Client: Zoe Dickerson
Project #: 864\_013\_MM
BTD #: Location 1
Location: Somerville, MA

Street 1: Somerville Avenue/Bow Street Street 2: Webster Avenue/Washington Street

Count Date: 2/12/2022
Day of Week: Saturday
Weather: Clouds & Sun, 40°F



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## PEDESTRIANS & BICYCLES

		Webster North	· Avenue bound		Son	nerville Ave South	nue/Bow St bound	reet		Washing Easth	ton Street bound				e Avenue bound	
Start Time	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED
10:00 AM	0	2	1	8	0	6	0	4	0	0	0	35	0	2	2	5
10:15 AM	1	1	0	6	0	3	0	15	0	1	0	26	0	0	1	19
10:30 AM	1	1	0	11	1	4	1	29	0	1	0	31	0	2	1	13
10:45 AM	0	2	0	4	1	9	0	17	0	1	0	24	0	2	0	16
11:00 AM	0	2	1	14	2	2	0	18	0	0	1	23	0	0	3	13
11:15 AM	0	4	0	13	5	7	0	15	0	1	1	44	1	2	1	17
11:30 AM	1	2	0	16	2	6	1	21	2	2	0	31	0	1	4	20
11:45 AM	0	3	1	14	0	5	0	20	1	2	1	37	0	0	2	15
12:00 PM	1	1	0	11	4	4	0	9	0	0	1	32	0	1	3	15
12:15 PM	0	7	0	16	1	3	0	29	1	5	0	48	0	4	2	30
12:30 PM	0	3	1	11	3	5	1	17	0	1	0	47	0	1	2	16
12:45 PM	0	7	1	9	3	3	0	13	0	2	2	46	0	0	2	14
1:00 PM	0	3	0	11	7	3	1	20	1	3	0	41	3	1	0	16
1:15 PM	0	5	0	13	4	7	0	40	0	1	0	58	0	4	0	28
1:30 PM	0	1	0	15	1	3	0	27	2	3	0	50	0	0	0	25
1:45 PM	1	5	0	25	3	4	2	29	1	0	0	55	0	2	2	16

MID PEAK HOUR		Webster	Avenue		Sor	nerville Ave	enue/Bow St	reet		Washing	ton Street			Somervill	e Avenue	
12:45 PM		North	bound			South	bound			Easth	oound			West	oound	
to	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED
1:45 PM	0	16	1	48	15	16	1	100	3	9	2	195	3	5	2	83

NOTE: Peak hour summaries here correspond to peak hours identified for passenger car and heavy vehicles combined.

Client: Zoe Dickerson
Project #: 864\_013\_MM
BTD #: Location 1
Location: Somerville, MA
Street 1: Somerville Avenue/Bow Street
Street 2: Webster Avenue/Washington Street

Count Date: 2/15/2022
Day of Week: Tuesday
Weather: Clouds & Sun, 30°F



PO BOX 1723, Framingham, MA 01701 Office: 978-746-1259 DataRequest@BostonTrafficData.com www.BostonTrafficData.com

## PASSENGER CARS & HEAVY VEHICLES COMBINED

		Wehster	r Avenue		Son		nue/Bow S	treet	· · · · · · · · · · · · · · · · · · ·		ton Street			Somervil	le Avenue	
			bound		3011		bound	11001			oound				bound	
Start Time	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
6:00 AM	0	0	14	0	0	18	9	0	0	1	13	0	0	6	11	21
6:15 AM	0	1	8	2	0	17	19	1	0	0	16	0	0	6	29	19
6:30 AM	0	0	11	1	0	14	24	1	0	2	9	3	0	5	34	22
6:45 AM	0	1	10	0	0	21	22	1	0	1	27	2	0	5	57	29
7:00 AM	0	2	15	3	0	27	42	7	0	7	21	3	0	17	40	34
7:15 AM	0	4	28	3	0	25	52	3	0	4	30	6	0	24	39	25
7:30 AM	0	1	45	13	0	37	64	5	0	10	35	11	0	31	49	29
7:45 AM	0	5	43	8	0	50	58	16	0	13	35	9	0	22	51	48
8:00 AM	0	2	41	7	0	43	63	18	0	13	55	9	0	8	58	43
8:15 AM	0	1	30	7	0	61	86	8	0	8	51	15	0	7	61	50
8:30 AM	0	0	24	5	0	47	77	8	0	16	55	10	0	7	51	45
8:45 AM	0	3	32	1	0	43	84	7	0	6	37	10	0	8	44	57
9:00 AM	0	2	37	3	0	40	71	1	0	12	39	9	0	5	32	44
9:15 AM	0	3	22	0	0	36	53	3	0	11	32	3	0	3	40	47
9:30 AM	0	3	30	1	0	28	44	3	0	6	34	1	0	4	45	33
9:45 AM	0	4	20	6	0	39	50	2	0	9	34	8	0	10	33	41
10:00 AM	0	3	29	3	0	31	43	3	0	8	25	2	0	5	39	51
10:15 AM	0	1	31	4	0	32	29	8	0	6	37	1	0	6	46	42
10:30 AM	0	3	24	3	0	21	26	3	0	8	31	5	0	6	46	31
10:45 AM	0	5	38	1	0	46	29	8	0	3	34	5	0	1	49	44
11:00 AM	0	1	28	7	0	43	35	3	0	5	38	5	0	6	36	41
11:15 AM	0	4	30	3	0	36	28	5	0	9	38	2	0	11	31	37
11:30 AM	0	3	24	1	0	40	43	4	0	11	33	3	0	6	32	55
11:45 AM	0	8	16	2	0	52	40	9	0	7	24	4	0	3	34	57
12:00 PM	0	2	29	1	0	37	38	2	0		39	4	0	7	36	56
12:15 PM 12:30 PM	0	2	37 29	7	0	48 56	43 34	13 11	0	8 14	33 38	5 4	0	7 10	28 29	42 60
12:45 PM	0	5	32	5	0	50	31	5	0	12	37	3	0	4	29	52
1:00 PM	0	2	34	2	0	40	34	1	0	5	45	2	0	7	34	54
1:15 PM	0	1	37	3	0	43	22	2	0	13	32	4	0	3	34	41
1:30 PM	0	6	30	3	0	47	32	1	0	8	47	7	0	8	34	54
1:45 PM	0	4	35	6	0	41	27	10	0	9	39	5	0	11	38	44
2:00 PM	0	3	33	5	0	40	21	6	0	13	37	8	0	10	48	57
2:15 PM	0	2	35	6	0	37	40	11	0	12	32	8	0	15	35	63
2:30 PM	0	6	45	9	0	30	36	14	0	14	43	6	0	16	53	34
2:45 PM	0	2	33	6	0	45	56	9	0	19	46	8	0	14	48	50
3:00 PM	0	1	60	15	0	38	44	7	0	24	53	5	0	8	46	57
3:15 PM	0	3	57	5	0	46	29	3	0	20	44	4	0	9	36	49
3:30 PM	0	3	54	4	0	38	33	3	0	13	57	4	0	3	51	50
3:45 PM	0	5	56	3	0	44	44	7	0	22	47	5	0	8	42	58
4:00 PM	0	8	61	3	0	28	38	7	0	26	46	6	0	5	32	48
4:15 PM	0	3	56	6	0	30	34	4	0	13	66	1	0	5	51	50
4:30 PM	0	2	49	4	0	52	41	8	0	22	53	7	0	8	40	56
4:45 PM	0	6	47	6	0	36	39	5	0	24	47	5	0	13	47	57
5:00 PM	0	7	67	3	0	44	48	6	0	19	57	9	0	4	56	49
5:15 PM	0	2	77	6	0	36	57	5	0	14	53	10	0	13	48	64
5:30 PM	0	4	71	3	0	39	43	3	0	22	55	8	0	3	57	52
5:45 PM	0	9	69	1	0	41	47	7	0	20	49	7	0	5	50	67
6:00 PM	0	6	63	3	0	32	29	8	0	19	54	3	0	7	70	49
6:15 PM	0	4	64	6	0	42	48	5	0	15	54	4	0	6	56	53
6:30 PM	0	3	49	5	0	27	45	6	0	11 17	47	3	0	12	46	55 46
6:45 PM		5	51	1	0	35	39				27	3	0	3	22	
7:00 PM 7:15 PM	0	6	34 48	<u>2</u> 5	0	34 36	37 41	10 9	0	10 20	34 34	7 5	0	4	47 35	57 51
	0		48 39	0	0	29	37	5	0		34	6	0	7	27	36
7:30 PM 7:45 PM	0	6	33	1	0	42	26	4	0	18 7	28	2	0	5	35	40
7.40 FIVI	U	Ö	ు		U	42	20	4	U	- /	20		U	o o	აა	40

AM PEAK HOUR 7:45 AM			Avenue bound		Son		nue/Bow St bound	reet			ton Street bound			Somerville West	e Avenue cound	
to	U-Turn	3				Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
8:45 AM	0 8 138 27				0	201	284	50	0	50	196	43	0	44	221	186
PHF		0.	77			0.	86			0.	89			0.	93	
HV %	0.0%	0.77				6.0%	2.1%	4.0%	0.0%	4.0%	8.2%	9.3%	0.0%	13.6%	9.5%	8.1%

MID PEAK HOUR		Webster	r Avenue		Sor	nerville Ave	nue/Bow St	reet		Washing	ton Street			Somervill	e Avenue	
12:15 PM		North	bound			South	bound			Easth	oound			West	oound	
to	U-Turn	5				Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
1:15 PM	0	0 11 132 15				194	142	30	0	39	153	14	0	28	113	208
PHF		0.	86			0.	88			0.	92			0.	88	
HV %	0.00/	0.86 0.0% 0.0% 1.5% 20.0%				7.2%	1.4%	16.7%	0.0%	7.7%	9.2%	0.0%	0.0%	17.9%	3.5%	5.3%

PM PEAK HOUR 5:00 PM			Avenue		Son	nerville Ave	nue/Bow St bound	reet			on Street			Somerville		
to	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
6:00 PM	0	0 22 284 13			0	160	195	21	0	75	214	34	0	25	211	232
PHF		0.	94			0.	96			0.	95			0.	94	
HV %	0.0%	0.94 0.0% 0.0% 0.4% 0.0%				1.9%	3.1%	0.0%	0.0%	4.0%	3.7%	0.0%	0.0%	12.0%	2.4%	2.6%

Client: Zoe Dickerson 864\_013\_MM Project #: BTD #: Location 1 Somerville, MA Location: Street 1: Somerville Avenue/Bow Street Webster Avenue/Washington Street Street 2:

2/15/2022 Count Date: Day of Week: Tuesday Weather: Clouds & Sun, 30°F



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									/EU/OL E/							
			r Avenue		Son		nue/Bow St		/EHICLES	Washing	ton Street				le Avenue	
Start Time	U-Turn	North Left	bound	Diales	U-Turn	Left	bound	Dielet	U-Turn	Left	oound Thru	Dielet	U-Turn	Left	bound Thru	Dielet
6:00 AM	0-1um 0	0	Thru 1	Right 0	0-1um 0	3	Thru 1	Right 0	0-1um 0	Lent 0	1 nru	Right 0	0-1um 0	2	2	Right 4
6:15 AM	0	0	0	0	0	3	1	0	0	0	1	0	0	0	4	4
6:30 AM	0	0	2	0	0	2	0	0	0	0	0	1	0	2	3	3
6:45 AM	0	0	0	0	0	4	0	0	0	0	3	0	0	1	14	7
7:00 AM	0	0	0	0	0	4	1	0	0	0	2	0	0	2	4	8
7:15 AM	0	0	1	0	0	3	1	0	0	0	2	0	0	3	6	4
7:30 AM	0	0	1	0	0	2	4	0	0	0	2	0	0	0	3	7
7:45 AM	0	0	0	0	0	5	2	0	0	0	3	1	0	2	6	7
8:00 AM	0	0	2	0	0	2	1	0	0	1	3	0	0	1	3	4
8:15 AM	0	0	2	0	0	4	2	2	0	0	3	2	0	2	5	2
8:30 AM	0	0	1	0	0	1	1	0	0	1	7	1	Ö	1	7	2
8:45 AM	0	0	0	0	0	4	2	2	0	0	5	0	0	2	5	4
9:00 AM	0	0	1	0	0	5	1	0	0	2	4	0	0	2	4	6
9:15 AM	0	0	0	0	0	4	2	0	0	0	5	0	0	1	1	6
9:30 AM	0	1	0	0	0	4	2	0	0	0	3	0	0	1	5	5
9:45 AM	0	1	0	0	0	6	0	0	0	2	6	1	0	1	5	4
10:00 AM	0	0	1	0	0	3	0	0	0	0	5	1	0	1	5	3
10:15 AM	0	0	1	Ö	0	2	0	2	0	0	7	0	0	2	3	5
10:30 AM	0	1	0	0	0	0	2	0	0	1	4	1	0	3	3	1
10:45 AM	0	0	3	0	0	3	2	1	0	0	1	1	0	0	3	4
11:00 AM	0	1	2	0	0	6	1	0	0	1	5	0	0	2	4	4
11:15 AM	0	0	4	0	0	7	1	1	0	0	2	0	0	0	4	0
11:30 AM	0	0	1	0	0	2	1	0	0	1	3	0	0	1	3	7
11:45 AM	0	0	0	0	0	4	2	1	0	0	1	0	0	0	3	1
12:00 PM	0	0	1	0	0	2	4	1	0	2	2	0	0	2	2	7
12:15 PM	0	0	1	2	0	5	1	1	0	0	3	0	0	1	1	2
12:30 PM	0	0	1	0	0	3	0	3	0	1	5	0	0	1	1	4
12:45 PM	0	0	0	1	0	4	1	1	0	2	2	0	0	1	0	4
1:00 PM	0	0	0	0	0	2	0	0	0	0	4	0	0	2	2	1
1:15 PM	0	0	0	0	0	3	2	0	0	1	3	0	0	0	4	1
1:30 PM	0	0	2	0	0	3	0	0	0	1	5	0	0	0	3	3
1:45 PM	0	1	1	0	0	3	1	0	0	1	4	0	0	1	7	1
2:00 PM	0	0	1	1	0	1	0	0	0	1	9	1	0	1	7	5
2:15 PM	0	0	0	0	0	4	1	0	0	1	2	0	0	1	7	4
2:30 PM	0	1	2	0	0	11	2	0	0	0	6	0	0	0	5	3
2:45 PM	0	0	3	0	0	3	2	11	0	0	6	0	0	3	6	1
3:00 PM	0	0	0	0	0	3	1	0	0	2	0	2	0	0	3	2
3:15 PM	0	0	1	0	0	3	1	0	0	0	2	0	0	2	2	2
3:30 PM	0	0	0	0	0	3	2	0	0	0	4	0	0	1	1	2
3:45 PM 4:00 PM	0	0	1	0	0	3	2	0	0	1	2	0	0	0	1	2
4:00 PM 4:15 PM	0	0	0	0	0	2	3 2	0	0	0	4 5	0	0	0	1	3 4
4:15 PM 4:30 PM	0	1	0	0	0	3	0	1	0	0	4	0	0	1	1	2
4:45 PM	0	0	1	0	0	4	1	1	0	0	5	0	0	2	2	3
5:00 PM	0	0	0	0	0	1	2	0	0	0	3	0	0	0	1	0
5:15 PM	0	0	0	0	0	1	1	0	0	2	2	0	0	2	1	4
5:30 PM	0	0	1	0	0	0	2	0	0	0	1	0	0	0	1	1
5:45 PM	0	0	0	0	0	1	1	0	0	1	2	0	0	1	2	1
6:00 PM	0	0	0	0	0	1	2	0	0	0	1	0	0	1	1	2
6:15 PM	0	0	1	0	0	<del>- i</del> -	0	0	0	0	1	0	0	1	1	3
6:30 PM	0	0	1	0	0	3	1	0	0	0	4	0	0	1	2	1
6:45 PM	0	0	0	0	0	3	1	0	0	0	2	0	0	1	1	0
7:00 PM	0	0	1	0	0	1	1	0	0	0	1	0	0	1	1	0
7:15 PM	0	0	0	0	0	1	1	0	0	0	5	0	0	1	0	2
7:30 PM	0	0	0	0	0	0	0	0	0	0	1	0	0	1	1	1
7:45 PM	0	0	1	0	0	1	0	0	0	0	2	0	0	0	0	0

AM PEAK HOUR		Webster	Avenue		Son	nerville Ave	nue/Bow St	reet		Washing	ton Street			Somerville	e Avenue	
8:15 AM		North	bound			South	bound			Easth	oound			Westh	oound	
to	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
9:15 AM	0	0	4	0	0	14	6	4	0	3	19	3	0	7	21	14
PHF		0.50				0.	75			0.	69			0.8	88	

MID PEAK HOUR		Webster Avenue Northbound -Turn Left Thru Right				nerville Ave	enue/Bow St	reet		Washing	ton Street			Somervill	le Avenue	
10:15 AM						South	bound			Easth	oound			Westl	bound	
to	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
11:15 AM	0	2	6	0	0	11	5	3	0	2	17	2	0	7	13	14
PHE		0.67				0	68			0	75			0	85	

PM PEAK HOUR 2:00 PM			r Avenue bound		Son		nue/Bow St bound	reet		Washingt Eastb	ton Street bound			Somervill West	e Avenue cound	
to	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
3:00 PM	0					9	5	1	0	2	23	1	0	5	25	13
PHF		0.	.67			0.	63			0.	59			0.	83	

Client: Zoe Dickerson
Project #: 864\_013\_MM
BTD #: Location 1
Location: Somerville, MA
Street 1: Somerville Avenue/Bow Street
Street 2: Webster Avenue/Washington Street

Count Date: 2/15/2022
Day of Week: Tuesday
Weather: Clouds & Sun, 30°F



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## PEDESTRIANS & BICYCLES

			r Avenue bound		Son		enue/Bow S	treet	5 G 2.0 1	Washing	ton Street				le Avenue bound	
Start Time	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED
6:00 AM	0	0	0	2	0	0	0	0	0	0	0	2	0	0	0	0
6:15 AM	0	0	0	1	0	1	0	1	0	0	0	1	0	0	0	0
6:30 AM	0	0	0	1	1	2	0	1	0	0	0	2	1	0	0	2
6:45 AM	0	0	0	1	2	1	0	2	0	0	0	11	0	0	0	4
7:00 AM	0	0	0	1	0	0	0	2	0	0	0	5	0	0	0	0
7:15 AM	0	2	0	2	1	6	0	3	0	0	0	14	0	0	0	6
7:30 AM 7:45 AM	0	0	0	8		6	0	5 3		0	0	34	0	0	0	18
8:00 AM	0	0	0	13 16	0	9	2	5	0	2	0	13 5	1	3	0	9 5
8:15 AM	0	0	0	2	1	9	0	6	0	0	0	16	0	0	0	6
8:30 AM	1	0	0	5	1	7	0	6	0	1	0	11	0	0	0	8
8:45 AM	0	1	0	3	5	12	0	5	0	0	0	12	2	1	0	6
9:00 AM	0	2	0	3	2	6	0	3	0	0	0	7	0	0	0	7
9:15 AM	0	0	0	1	0	6	0	4	0	1	0	9	0	0	0	2
9:30 AM	0	0	0	2	0	7	0	5	0	1	0	11	0	0	0	5
9:45 AM	0	0	0	1	3	1	0	6	0	1	0	7	0	0	0	4
10:00 AM	0	0	0	4	1	3	0	3	0	0	0	11	0	0	0	4
10:15 AM	0	1	0	1	1	2	2	3	0	0	0	5	0	0	0	2
10:30 AM	0	0	0	3	1	2	0	4	0	0	0	5	0	0	0	4
10:45 AM	0	1	0	4	1	2	0	5	0	1	1	4	2	1	0	1
11:00 AM	0	0	0	3	2	2	0	4	0	0	0	5	0	0	0	2
11:15 AM	0	0	0	2	2	2	0	3	0	0	0	9	0	0	0	2
11:30 AM 11:45 AM	0	0	0	5 6	1	6	0	5 7	0	0	0	8 16	0	0	0	<u>6</u> 7
12:00 PM	0	1	0	13	0	2	0	5	0	0	0	11	0	1	0	7
12:15 PM	0	0	0	10	0	2	0	4	1	2	0	11	0	0	1	5
12:30 PM	0	0	0	10	3	0	0	9	0	0	0	8	0	2	0	8
12:45 PM	0	0	0	4	0	2	0	7	0	0	0	13	0	0	0	8
1:00 PM	0	1	0	2	1	3	0	4	0	0	0	5	0	0	0	5
1:15 PM	0	0	0	1	2	0	0	3	0	0	0	4	0	0	0	2
1:30 PM	0	2	0	5	0	2	0	2	0	1	0	8	0	3	0	10
1:45 PM	0	0	0	8	1	0	0	8	0	0	0	9	0	0	0	12
2:00 PM	0	2	0	5	0	1	0	1	0	0	0	6	0	1	0	17
2:15 PM	0	0	1	6	1	1	0	10	0	1	0	9	0	0	0	6
2:30 PM	0	1	0	18	0	2	0	18	0	0	0	10	0	0	0	3
2:45 PM	0	2	0	34	0	2	0	7	0	0	0	40	0	0	0	43
3:00 PM	0	0	0	19	1	0	0	10	0	0	0	56	0	1	1	32
3:15 PM 3:30 PM	0	3	0	13 11	0	0	0	8 10	0	0	0	17 19	0	0	0	17 14
3:45 PM	0	3	0	12	3	4	0	7	1	0	0	17	0	0	0	11
4:00 PM	0	4	0	5	1	1	0	5	0	0	0	19	0	1	1	4
4:15 PM	0	2	0	8	1	1	0	4	1	1	0	12	0	0	1	5
4:30 PM	1	4	0	6	0	1	0	5	0	1	0	15	0	0	1	5
4:45 PM	0	5	0	4	0	0	0	8	0	1	0	15	0	0	0	16
5:00 PM	0	5	0	11	1	1	0	8	0	1	1	15	0	1	0	14
5:15 PM	0	6	0	6	1	2	0	14	0	4	1	15	0	1	0	13
5:30 PM	0	9	0	9	1	1	0	3	0	2	0	21	0	0	0	13
5:45 PM	0	6	0	9	0	1	0	16	0	0	0	23	0	1	0	20
6:00 PM	0	7	1	2	0	0	0	6	0	1	0	15	0	0	0	10
6:15 PM	1	2	0	4	1	1	0	2	0	1	0	16	0	0	0	10
6:30 PM	0	4	0	5	0	1	0	7	0	1	0	22 14	0	1	1	5 4
6:45 PM	0	9	1	5	0	0		15	0					0	0	13
7:00 PM 7:15 PM	0	2	0	5 2	1 2	2	0	15 4	0	3	0	7	0	0	0	13
7:30 PM	0	3	0	4	0	3	0	6	0	1	0	13	0	0	1	4
7:45 PM	0	3	1	4	0	0	0	3	0	0	0	6	0	1	0	6
7.70 I W	<u> </u>	<u> </u>	<del></del>	<del></del>						<u> </u>	<u> </u>					

AM PEAK HOUR		Webster	Avenue		Sor	nerville Ave	nue/Bow St	reet		Washingt	on Street			Somervill	e Avenue	
7:45 AM		Northbound				South	bound			Easth	ound			West	oound	
to	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED
8:45 AM	1	2	0	36	2	28	4	20	0	3	0	45	1	3	0	28

MID PEAK HOUR			r Avenue		Son	nerville Ave		treet		Washingt	ton Street				le Avenue	
12:15 PM		North	bound			South	bound			Eastb	oound			West	bound	
to	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED
1:15 PM	0	1	0	26	4	7	0	24	1	2	0	37	0	2	1	26

PM PEAK HOU	₹ .	Webster	Avenue		Sor	nerville Ave	nue/Bow St	reet		Washingt	on Street			Somervill	e Avenue	
5:00 PM		North	bound			South	bound			Easth	ound			West	oound	
to	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED
6:00 PM	0	26	0	35	3	5	0	41	0	7	2	74	0	3	0	60

NOTE: Peak hour summaries here correspond to peak hours identified for passenger car and heavy vehicles combined.

Client: Zoe Dickerson Project #: 864\_013\_MM BTD #: Location 2 Somerville, MA Location: Street 1: Bow Street Street 2: Warren Avenue 2/12/2022 Count Date: Day of Week: Saturday Clouds & Sun, 40°F Weather:



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## PASSENGER CARS & HEAVY VEHICLES COMBINED

		Bow	Street			Bow	Street			Warren	Avenue		Warren Avenue					
	Northbound						bound			Easth	oound		Westbound					
Start Time	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right		
10:00 AM	0	0	92	3	0	0	0	0	0	35	9	0	0	0	0	15		
10:15 AM	0	0	100	5	0	0	0	0	0	39	10	0	0	0	0	12		
10:30 AM	0	0	91	1	0	0	0	0	0	37	5	0	0	0	0	15		
10:45 AM	0	0	116	3	0	0	0	0	0	27	13	0	0	0	0	18		
11:00 AM	0	0	121	4	0	0	0	0	0	35	14	0	0	0	0	16		
11:15 AM	0	0	134	6	0	0	0	0	0	48	7	0	0	0	0	15		
11:30 AM	0	0	116	8	0	0	0	0	0	39	15	0	0	0	0	17		
11:45 AM	0	0	113	6	0	0	0	0	0	55	10	0	0	0	0	15		
12:00 PM	0	0	118	4	0	0	0	0	0	46	11	0	0	0	0	20		
12:15 PM	0	0	117	9	0	0	0	0	0	50	14	0	0	0	0	21		
12:30 PM	0	0	112	4	0	0	0	0	0	58	13	0	0	0	0	19		
12:45 PM	0	0	123	10	0	0	0	0	0	39	10	0	0	0	0	29		
1:00 PM	0	0	105	6	0	0	0	0	0	49	13	0	0	0	0	19		
1:15 PM	0	0	125	8	0	0	0	0	0	50	12	0	0	0	0	25		
1:30 PM	0	0	130	7	0	0	0	0	0	53	6	0	0	0	0	24		
1:45 PM	0	0	113	7	0	0	0	0	0	50	10	0	0	0	0	17		

MID PEAK HOUR	2	Bow	Street		Bow Street					Warren	Avenue		Warren Avenue			
12:45 PM		North	oound		Southbound				Eastbound				Westbound			
to	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
1:45 PM	0	0	483	31	0	0	0	0	0	191	41	0	0	0	0	97
1:45 PM <b>PHF</b>	0	0 0.	483 94	31	0		00	0	0		41 94	0	0	0 0.	0 84	97

Client: Zoe Dickerson Project #: 864\_013\_MM BTD #: Location 2 Somerville, MA Location: Street 1: Bow Street Street 2: Warren Avenue 2/12/2022 Count Date: Day of Week: Saturday Clouds & Sun, 40°F Weather:



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## **HEAVY VEHICLES**

			Street bound				Street bound				Avenue		Warren Avenue Westbound				
Start Time	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	
10:00 AM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	
10:15 AM	0	0	3	0	0	0	0	0	0	0	0	0	0	0	0	0	
10:30 AM	0	0	2	0	0	0	0	0	0	0	0	0	0	0	0	0	
10:45 AM	0	0	1	0	0	0	0	0	0	3	0	0	0	0	0	0	
11:00 AM	0	0	1	0	0	0	0	0	0	0	1	0	0	0	0	0	
11:15 AM	0	0	3	0	0	0	0	0	0	0	0	0	0	0	0	0	
11:30 AM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	
11:45 AM	0	0	3	0	0	0	0	0	0	0	0	0	0	0	0	0	
12:00 PM	0	0	1	0	0	0	0	0	0	0	1	0	0	0	0	0	
12:15 PM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	
12:30 PM	0	0	2	0	0	0	0	0	0	0	0	0	0	0	0	1	
12:45 PM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	
1:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
1:15 PM	0	0	5	0	0	0	0	0	0	0	0	0	0	0	0	0	
1:30 PM	0	0	2	0	0	0	0	0	0	0	0	0	0	0	0	0	
1:45 PM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	

MID PEAK HOUR		Bow	Street		Bow Street					Warren	Avenue		Warren Avenue				
10:15 AM		North	bound		Southbound					Easth	oound		Westbound				
to	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	
11:15 AM	0	0	7	0	0	0	0	0	0	3	1	0	0	0	0	0	
PHF		0.	58		0.00					0.	33		0.00				

Client: Zoe Dickerson 864\_013\_MM Project #: BTD #: Location 2 Somerville, MA Location: Street 1: Bow Street Street 2: Warren Avenue 2/12/2022 Count Date: Day of Week: Saturday Clouds & Sun, 40°F Weather:



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#### PEDESTRIANS & BICYCLES

			Street bound				Street				Avenue				Avenue bound	
Start Time	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED
10:00 AM	0	5	0	34	0	0	0	4	0	0	0	32	0	0	0	23
10:15 AM	0	3	0	49	0	0	0	0	0	0	0	39	0	0	2	37
10:30 AM	0	4	0	48	0	0	0	7	2	0	0	46	0	0	0	27
10:45 AM	1	4	1	43	0	0	0	4	0	1	0	38	0	1	1	31
11:00 AM	0	7	0	28	0	0	0	1	0	0	0	20	0	0	0	31
11:15 AM	1	4	0	58	0	0	0	6	2	1	0	26	0	1	0	46
11:30 AM	2	6	1	47	0	0	0	1	0	2	0	47	0	0	1	41
11:45 AM	2	4	1	60	0	0	0	7	0	0	0	57	0	0	0	53
12:00 PM	0	7	1	56	0	0	0	2	0	0	0	54	0	0	0	55
12:15 PM	1	14	0	88	0	0	0	4	2	0	0	64	0	0	0	63
12:30 PM	1	8	0	75	0	1	0	5	2	1	0	68	0	0	0	39
12:45 PM	0	8	0	51	0	0	0	0	0	0	1	53	0	0	0	47
1:00 PM	1	7	0	100	0	0	0	3	3	2	0	90	0	0	1	65
1:15 PM	0	8	0	80	0	0	0	10	1	0	0	85	0	2	0	49
1:30 PM	0	2	0	68	0	0	0	10	1	1	1	75	0	0	0	51
1:45 PM	0	8	0	89	0	0	0	8	1	0	0	70	0	0	1	55

MID PEAK HOUR		Bow :	Street			Bow	Street			Warren	Avenue			Warren	Avenue	
12:45 PM		North	bound			South	bound			Eastl	oound			West	bound	
to	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED
1:45 PM	1	25	0	299	0	0	0	23	5	3	2	303	0	2	1	212

NOTE: Peak hour summaries here correspond to peak hours identified for passenger car and heavy vehicles combined.

Client: Zoe Dickerson 864\_013\_MM Project #: BTD #: Location 2 Somerville, MA Location: Street 1: Bow Street Warren Avenue Street 2: 2/15/2022 Count Date: Day of Week: Tuesday Weather: Clouds & Sun, 30°F



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## PASSENGER CARS & HEAVY VEHICLES COMBINED

			Street				Street				Avenue				Avenue	
Ot T'	I		bound	D: Li			bound	D: 14			bound	D: 1.			bound	D: L
Start Time	U-Turn	Left	Thru	Right												
6:00 AM	0	0	37	0	0	0	0	0	0	3	3	0	0	0	0	2
6:15 AM	0	0	26	1	0	0	0	0	0	13	1	0	0	0	0	1
6:30 AM	0	0	32	1	0	0	0	0	0	6	3	0	0	0	0	6
6:45 AM	0	0	39	1	0	0	0	0	0	9	1	0	0	0	0	7
7:00 AM	0	0	55	2	0	0	0	0	0	14	5	0	0	0	0	10
7:15 AM	0	0	61	1	0	0	0	0	0	30	6	0	0	0	0	8
7:30 AM	0	0	92	2	0	0	0	0	0	30	8	0	0	0	0	25
7:45 AM	0	0	100	3	0	0	0	0	0	42	10	0	0	0	0	16
8:00 AM	0	0	99	3	0	0	0	0	0	42	13	0	0	0	0	21
8:15 AM	0	0	87	4	0	0	0	0	0	42	9	0	0	0	0	13
8:30 AM	0	0	79	3	0	0	0	0	0	31	14	0	0	0	0	14
8:45 AM	0	0	97	0	0	0	0	0	0	30	8	0	0	0	0	10
9:00 AM	0	0	83	5	0	0	0	0	0	27	16	0	0	0	0	15
9:15 AM	0	0	82	4	0	0	0	0	0	30	7	0	0	0	0	13
9:30 AM	0	0	65	5	0	0	0	0	0	36	10	0	0	0	0	10
9:45 AM	0	0	74	2	0	0	0	0	0	20	7	0	0	0	0	11
10:00 AM	0	0	85	2	0	0	0	0	0	36	5	0	0	0	0	15
10:15 AM	0	0	79	6	0	0	0	0	0	24	8	0	0	0	0	13
10:30 AM	0	0	62	2	0	0	0	0	0	20	7	0	0	0	0	10
10:45 AM	0	0	83	6	0	0	0	0	0	32	5	0	0	0	0	16
11:00 AM	0	0	72	5	0	0	0	0	0	31	9	0	0	0	0	16
11:15 AM	0	0	74	6	0	0	0	0	0	37	8	0	0	0	0	10
11:30 AM	0	0	88	4	0	0	0	0	0	36	8	0	0	0	0	22
11:45 AM	0	0	84	4	0	0	0	0	0	33	11	0	0	0	0	19
12:00 PM	0	0	79	5	0	0	0	0	0	36	9	0	0	0	0	18
12:15 PM	0	0	99	4	0	0	0	0	0	39	7	0	0	0	0	20
12:30 PM	0	0	107	6	0	0	0	0	0	30	4	0	0	0	0	8
12:45 PM	0	0	97	8	0	0	0	0	0	40	2	0	0	0	0	16
1:00 PM	0	0	87	4	0	0	0	0	0	47	11	0	0	0	0	14
1:15 PM	0	0	92	5	0	0	0	0	0	43	17	0	0	0	0	16
1:30 PM	0	0	81	4	0	0	0	0	0	42	13	0	0	0	0	23
1:45 PM	0	0	92	2	0	0	0	0	0	31	7	0	0	0	0	12
2:00 PM	0	0	89	7	0	0	0	0	0	27	7	0	0	0	0	17
2:15 PM	0	0	110	2	0	0	0	0	0	30	14	0	0	0	0	21
2:30 PM	0	0	85	5	0	0	0	0	0	44	15	0	0	0	0	23
2:45 PM	0	0	98	8	0	0	0	0	0	59	21	0	0	0	0	17
3:00 PM	0	0	127	7	0	0	0	0	0	55	20	0	0	0	0	26
3:15 PM	0	0	126	7	0	0	0	0	0	46	13	0	0	0	0	15
3:30 PM	0	0	112	9	0	0	0	0	0	28	23	0	0	0	0	19
3:45 PM	0	0	133	6	0	0	0	0	0	32	13	0	0	0	0	12
4:00 PM	0	0	124	7	0	0	0	0	0	46	17	0	0	0	0	16
4:15 PM	0	0	116	2	0	0	0	0	0	38	11	0	0	0	0	20
4:30 PM	0	0	128	2	0	0	0	0	0	43	13	0	0	0	0	13
4:45 PM	0	0	122	8	0	0	0	0	0	46	13	0	0	0	0	15
5:00 PM	0	0	134	2	0	0	0	0	0	61	16	0	0	0	0	17
5:15 PM	0	0	143	3	0	0	0	0	0	31	9	0	0	0	0	19
5:30 PM	0	0	157	6	0	0	0	0	0	41	13	0	0	0	0	10
5:45 PM	0	0	148	10	0	0	0	0	0	46	11	0	0	0	0	11
6:00 PM	0	0	133	2	0	0	0	0	0	34	17	0	0	0	0	9
6:15 PM	0	0	127	5	0	0	0	0	0	41	17	0	0	0	0	15
6:30 PM	0	0	114	5	0	0	0	0	0	36	18	0	0	0	0	10
6:45 PM	0	0	117	6	0	0	0	0	0	40	13	0	0	0	0	9
7:00 PM	0	0	99	3	0	0	0	0	0	21	8	0	0	0	0	21
7:15 PM	0	0	98	13	0	0	0	0	0	23	10	0	0	0	0	13
7:30 PM	0	0	93	1	0	0	0	0	0	33	6	0	0	0	0	5
7:45 PM	0	0	82	1	0	0	0	0	0	34	8	0	0	0	0	10

AM PEAK HOUR 7:30 AM			Street bound				Street bound				Avenue				Avenue bound	
to	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
8:30 AM	0	0	378	12	0	0	0	0	0	156	40	0	0	0	0	75
PHF		0.	95			0.	00			0.	89			0.	.75	
HV %	0.0%	0.0%	6.6%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.6%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%

MID PEAK HOUR			Street				Street				Avenue				Avenue	
12:45 PM		North	bound			South	bound			East	oound			vvest	bound	
to	U-Turn	U-Turn Left Thru Right				Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
1:45 PM	0	0	357	21	0	0	0	0	0	172	43	0	0	0	0	69
PHF		0.	90			0.	00			0.	90			0.	75	
HV %	0.0%	0.0%	3.9%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	3.5%	4.7%	0.0%	0.0%	0.0%	0.0%	1.4%

PM PEAK HOUR		Bow :	Street			Bow \$	Street			Warren	Avenue			Warren	Avenue	
5:00 PM		North	bound			South	bound			Easth	ound			West	oound	
to	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
6:00 PM	0	0	582	21	0	0	0	0	0	179	49	0	0	0	0	57
PHF		0.	92			0.	00			0.	74			0.	75	
HV%	0.0%	0.0%	1.7%	4.8%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	7.0%

Client: Zoe Dickerson 864\_013\_MM Project #: BTD #: Location 2 Somerville, MA Location: Street 1: Bow Street Warren Avenue Street 2: 2/15/2022 Count Date: Day of Week: Tuesday Weather: Clouds & Sun, 30°F



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# HEAVY VEHICLES

			Street bound				Street bound				Avenue				Avenue	
Start Time	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
6:00 AM	0	0	5	0	0	0	0	0	0	0	0	0	0	0	0	0
6:15 AM	0	0	4	0	0	0	0	0	0	0	0	0	0	0	0	0
6:30 AM	0	0	4	0	0	0	0	0	0	1	0	0	0	0	0	0
6:45 AM	0	0	7	0	0	0	0	0	0	0	0	0	0	0	0	0
7:00 AM	0	0	8	0	0	0	0	0	0	0	0	0	0	0	0	0
7:00 AM 7:15 AM						0		0		-						0
	0	0	3	0	0		0		0	2	0	0	0	0	0	
7:30 AM	0	0	5	0	0	0	0	0	0	0	0	0	0	0	0	0
7:45 AM	0	0	8	0	0	0	0	0	0	0	0	0	0	0	0	0
8:00 AM	0	0	7	0	0	0	0	0	0	1	0	0	0	0	0	0
8:15 AM	0	0	5	0	0	0	0	0	0	0	0	0	0	0	0	0
8:30 AM	0	0	4	0	0	0	0	0	0	0	0	0	0	0	0	0
8:45 AM	0	0	5	0	0	0	0	0	0	3	0	0	0	0	0	0
9:00 AM	0	0	9	0	0	0	0	0	0	0	0	0	0	0	0	0
9:15 AM	0	0	5	0	0	0	0	0	0	1	0	0	0	0	0	1
9:30 AM	0	0	5	0	0	0	0	0	0	2	0	0	0	0	0	1
9:45 AM	0	0	5	0	0	0	0	0	0	2	0	0	0	0	0	0
10:00 AM	0	0	5	0	0	0	0	0	0	2	0	0	0	0	0	0
10:15 AM	0	0	5	0	0	0	0	0	0	0	0	0	0	0	0	0
10:30 AM	0	0	2	1	0	0	0	0	0	1	1	0	0	0	0	0
10:45 AM	0	0	7	0	0	0	0	0	0	1	0	0	0	0	0	1
11:00 AM	0	0	6	0	0	0	0	0	0	1	0	0	0	0	0	0
11:15 AM	0	0	4	0	0	0	0	0	0	5	0	0	0	0	0	0
11:30 AM	0	0	4	0	0	0	0	0	0	1	0	0	0	0	0	0
11:45 AM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0
12:00 PM	0	0	7	0	0	0	0	0	0	1	0	0	0	0	0	0
12:15 PM	0	0	3	0	0	0	0	0	0	1	0	0	0	0	0	1
12:30 PM	0	0	5	0	0	0	0	0	0	1	0	0	0	0	0	1
12:45 PM	0	0	6	0	0	0	0	0	0	1	0	0	0	0	0	0
1:00 PM	0	0	1	0	0	0	0	0	0	3	0	0	0	0	0	0
1:15 PM	0	0	3	0	0	0	0	0	0	0	1	0	0	0	0	0
1:30 PM	0	0	4	0	0	0	0	0	0	2		0	0	0	0	1
1:45 PM	0	0	3	0	0	0	0	0	0	0	0	0	0	0	0	0
2:00 PM	0	0	4	1	0	0	0	0	0	0	0	0	0	0	0	1
2:15 PM	0	0	5	1	0	0	0	0	0	1	0	0	0	0	0	1
2:30 PM	0	0	2	0	0	0	0	0	0	0	0	0	0	0	0	1
2:45 PM	0	0	3	0	0	0	0	0	0	1	0	0	0	0	0	0
3:00 PM	0	0	3	0	0	0	0	0	0	0	0	0	0	0	0	0
3:15 PM	0	0	3	0	0	0	0	0	0	0	0	0	0	0	0	0
3:30 PM	0	0	2	0	0	0	0	0	0	0	0	0	0	0	0	1
3:45 PM	0	0	4	0	0	0	0	0	0	1	0	0	0	0	0	1
4:00 PM	0	0	3	0	0	0	0	0	0	0	0	0	0	0	0	0
4:15 PM	0	0	2	0	0	0	0	0	0	0	0	0	0	0	0	0
4:30 PM	0	0	2	0	0	0	0	0	0	0	0	0	0	0	0	0
4:45 PM	0	0	2	1	0	0	0	0	0	0	0	0	0	0	0	1
5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
5:15 PM	0	0	5	1	0	0	0	0	0	0	0	0	0	0	0	2
5:30 PM	0	0	3	0	0	0	0	0	0	0	0	0	0	0	0	1
5:45 PM	0	0	2	0	0	0	0	0	0	0	0	0	0	0	0	0
6:00 PM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
6:15 PM	0	0	3	0	0	0	0	0	0	0	0	0	0	0	0	0
6:30 PM	0	0	2	0	0	0	0	0	0	1	0	0	0	0	0	0
6:45 PM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0
7:00 PM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0
7:15 PM	0	0	2	0	0	0	0	0	0	0	0	0	0	0	0	0
7:30 PM	0	0	2	0	0	0	0	0	0	1	0	0	0	0	0	0
7:45 PM	0	0	2	0	0	0	0	0	0	0	0	0	0	0	0	0
1.43 FIVI	U	U	<u> </u>		U	U		U	U		U	U	U	U	U	U

AM PEAK I	IOUR		Bow	Street			Bow	Street			Warren	Avenue			Warren	Avenue	
8:45 AN	Л		North	bound			South	bound			Easth	ound			West	bound	
to		U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
9:45 AN	Л	0	0	24	0	0	0	0	0	0	6	0	0	0	0	0	2
PHF			0 0 24 0				0.	00			0.	50			0.	50	

		_	_			_	_				_					
MID PEAK HOUR		Bow	Street			Bow	Street			Warren	Avenue			Warren	Avenue	
10:30 AM		North	bound			South	bound			Easth	oound			West	bound	
to	U-Turn					Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
11:30 AM	0					0	0	0	0	8	1	0	0	0	0	1
PHF		0 0 19 1				0	00			0	45			0	25	

PM PEAK HOUR 2:00 PM			Street bound				Street bound				Avenue			Warren Westl	Avenue	
to	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
3:00 PM	0	0 0 14 2				0	0	0	0	2	0	0	0	0	0	3
PHF		0.	.67			0.	00			0.	50			0.	75	

Client: Zoe Dickerson 864\_013\_MM Project #: BTD #: Location 2 Location: Somerville, MA Street 1: Bow Street Warren Avenue Street 2: 2/15/2022 Count Date: Day of Week: Tuesday Weather: Clouds & Sun, 30°F



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#### PEDESTRIANS & BICYCLES

			Street bound				Street bound	-STAIAN	o a bio i	Warren	Avenue				Avenue bound	
Start Time	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED
6:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2
6:15 AM	0	0	0	5	0	0	0	1	0	0	0	2	1	0	0	2
6:30 AM	0	0	0	7	0	0	0	0	0	0	0	4	0	0	0	5
6:45 AM	1	0	0	7	0	1	0	2	0	0	0	6	0	0	0	4
7:00 AM	0	1	0	4	0	0	0	1	0	0	0	3	0	0	0	6
7:15 AM	0	2	0	11	0	0	0	2	0	0	0	8	0	0	0	12
7:30 AM	0	1	0	24	0	0	0	0	0	0	0	8 12	0	0	0	9
7:45 AM 8:00 AM	0	3 1	0	26 43	0	0	0	2	0	0	0	19	0	0	0	8 22
8:15 AM	0	0	0	21	0	0	0	0	0	0	0	18	0	0	1	12
8:30 AM	0	0	0	17	0	0	0	1	0	0	0	11	0	0	0	12
8:45 AM	0	3	0	14	0	0	0	1	0	0	0	14	0	0	0	4
9:00 AM	0	4	0	11	0	0	0	1	0	0	0	8	0	0	0	6
9:15 AM	0	1	0	15	0	0	0	0	0	0	0	9	0	0	0	3
9:30 AM	0	0	0	12	0	1	0	0	0	0	0	12	1	0	0	7
9:45 AM	1	0	0	13	0	0	0	1	0	0	0	15	0	0	0	7
10:00 AM	0	2	0	23	0	0	0	2	0	0	0	17	0	0	0	14
10:15 AM	0	2	0	13	0	0	0	4	0	0	0	10	1	0	0	6
10:30 AM	0	0	0	16	0	0	0	0	0	0	0	13	0	0	0	11
10:45 AM	0	1	0	18	0	0	0	0	0	0	0	12	0	0	0	13
11:00 AM	1	0	0	11	0	0	0	0	0	0	0	6	1	0	0	8
11:15 AM 11:30 AM	0	2	0	17 9	0	0	0	3	0	0	0	11 12	0	0	0	10 9
11:45 AM	0	1	0	18	0	0	0	1	0	0	0	14	0	0	0	7
12:00 PM	0	3	0	27	0	0	0	2	0	0	0	22	0	0	0	6
12:15 PM	0	1	1	33	0	0	0	2	0	0	0	20	0	0	0	8
12:30 PM	0	0	0	22	0	0	0	2	1	0	0	17	0	0	0	3
12:45 PM	0	0	0	31	0	0	0	2	0	0	0	20	0	0	0	7
1:00 PM	0	2	0	21	0	0	0	3	0	0	0	19	0	0	0	8
1:15 PM	1	1	0	18	0	0	0	4	0	0	0	15	0	0	0	5
1:30 PM	0	2	0	14	0	0	0	0	0	0	0	13	0	0	0	5
1:45 PM	0	1	0	19	0	0	0	0	0	0	0	10	1	0	0	9
2:00 PM	0	2	0	25	0	0	0	1	0	0	0	20	0	0	0	6
2:15 PM	0	3	0	35	0	0	0	2	2	1	0	23	0	0	0	6
2:30 PM	0	0	1	43	0	0	0	2	0	1	1	26	0	0	0	18
2:45 PM 3:00 PM	0	1	0	49 58	0	0	0	0	1	0	0	12 12	0	0	0	16 21
3:15 PM	1	2	0	19	0	0	0	1	0	0	0	15	0	0	0	11
3:30 PM	0	1	0	23	0	0	0	10	0	0	0	14	0	1	0	19
3:45 PM	0	6	0	28	0	0	0	1	0	0	1	19	1	0	0	13
4:00 PM	0	5	1	19	0	0	0	3	0	0	0	16	0	0	0	8
4:15 PM	0	7	0	23	0	1	0	1	0	0	0	20	0	0	0	15
4:30 PM	0	4	1	20	0	0	0	0	0	0	0	10	0	0	0	13
4:45 PM	0	7	0	20	0	0	0	1	0	0	0	17	0	0	0	13
5:00 PM	0	4	0	38	0	0	0	1	0	1	0	25	0	0	0	18
5:15 PM	0	10	1	23	0	0	0	0	0	1	0	15	0	0	0	10
5:30 PM	0	15	1	32	0	0	0	0	0	0	0	21	0	0	0	21
5:45 PM	1	6	0	15	0	0	0	1	0	1	0	13	0	0	0	28
6:00 PM 6:15 PM	0	10 6	0	17 16	0	0	0	3	0	0	0	12 20	0	0	0	9
6:30 PM	0	6	0	27	0	0	0	0	0	0	0	16	0	0	0	16
6:30 PM 6:45 PM	0	12	1	19	0	0	0	2	0	1	0	17	0	0	0	22
7:00 PM	0	5	0	13	0	0	0	0	0	0	0	12	0	0	0	16
7:15 PM	0	6	0	15	0	0	0	0	1	0	0	12	1	0	0	15
7:30 PM	0	1	0	9	0	0	0	0	0	0	0	9	0	0	0	16
7:45 PM	0	2	0	10	0	0	0	1	0	0	0	9	0	0	0	7
-																

AM PEAK HOUR		Bow 8	Street			Bow \$	Street			Warren	Avenue			Warren	Avenue	
7:30 AM		North	bound			South	bound			Easth	ound			West	oound	
to	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED
8:30 AM	0	5	0	114	0	0	0	2	0	0	0	57	0	0	1	51

MID PEAK HOUR		Bow	Street			Bow	Street			Warren	Avenue			Warren	Avenue	
12:45 PM		North	bound			South	bound			Easth	oound			West	bound	
to	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED
1:45 PM	1	5	0	84	0	0	0	9	0	0	0	67	0	0	0	25

PM PEAK HOU	R	Bow	Street			Bow	Street			Warren	Avenue			Warren	Avenue	
5:00 PM		North	bound			South	bound			Easth	ound			West	oound	
to	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED
6:00 PM	1	35	2	108	0	0	0	2	0	3	0	74	0	0	0	77

NOTE: Peak hour summaries here correspond to peak hours identified for passenger car and heavy vehicles combined.

Client: Zoe Dickerson Project #: 864\_013\_MM BTD #: Location 3 Somerville, MA Location: Street 1: Somerville Avenue Street 2: Hawkins Street 2/12/2022 Count Date: Day of Week: Saturday Clouds & Sun, 40°F Weather:



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#### PASSENGER CARS & HEAVY VEHICLES COMBINED

							· · · · · · · · · · · · · · · · · · ·		· · · ···							
		Hawkin	s Street			Driv	eway			Somervill	e Avenue			Somervill	le Avenue	
		North	bound			South	bound			Easth	oound			West	bound	
Start Time	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
10:00 AM	0	0	0	9	0	1	0	0	0	1	128	0	0	0	0	0
10:15 AM	0	0	0	6	0	0	0	0	0	1	135	0	0	0	0	0
10:30 AM	0	0	0	10	0	1	0	0	0	1	144	0	0	0	0	0
10:45 AM	0	0	0	6	0	0	0	0	0	0	143	0	0	0	0	0
11:00 AM	0	0	0	10	0	0	0	0	0	1	152	0	0	0	0	0
11:15 AM	0	0	2	12	0	0	0	0	0	0	144	0	0	0	0	0
11:30 AM	0	0	1	8	0	2	0	0	0	1	135	0	0	0	0	0
11:45 AM	0	0	1	9	0	0	0	0	0	0	168	0	0	0	0	0
12:00 PM	0	0	0	10	0	1	0	0	0	1	178	0	0	0	0	0
12:15 PM	0	0	0	10	0	0	0	0	0	0	159	0	0	0	0	0
12:30 PM	0	0	0	9	0	0	0	0	0	0	179	0	0	0	0	0
12:45 PM	0	0	0	8	0	0	0	0	0	0	158	0	0	0	0	0
1:00 PM	0	0	0	13	0	0	0	0	0	0	181	0	0	0	0	0
1:15 PM	0	0	0	9	0	0	0	0	0	0	175	0	0	0	0	0
1:30 PM	0	0	0	12	0	1	0	0	0	1	156	0	0	0	0	0
1:45 PM	0	0	0	15	0	0	0	0	0	0	157	0	0	0	0	0

MID F	PEAK HOUR		Hawkin	s Street			Drive	eway			Somervill	e Avenue			Somervill	e Avenue	
1	2:30 PM		North	bound			South	bound			Easth	oound			West	bound	
	to	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
1	1:30 PM	0	0	0	39	0	0	0	0	0	0	693	0	0	0	0	0
	PHF	0.75					0.	00			0.	96			0.	00	
	HV %	0.0% 0.0% 0.0% 0.0%				0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	1.3%	0.0%	0.0%	0.0%	0.0%	0.0%

Client: Zoe Dickerson Project #: 864\_013\_MM BTD #: Location 3 Somerville, MA Location: Street 1: Somerville Avenue Street 2: Hawkins Street 2/12/2022 Count Date: Day of Week: Saturday Weather: Clouds & Sun, 40°F



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#### **HEAVY VEHICLES**

		Hawkin North	s Street bound			Drive South	•				e Avenue oound				le Avenue bound	
Start Time	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
10:00 AM	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0
10:15 AM	0	0	0	0	0	0	0	0	0	0	4	0	0	0	0	0
10:30 AM	0	0	0	0	0	0	0	0	0	0	4	0	0	0	0	0
10:45 AM	0	0	0	1	0	0	0	0	0	0	2	0	0	0	0	0
11:00 AM	0	0	0	0	0	0	0	0	0	0	2	0	0	0	0	0
11:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:30 AM	0	0	0	0	0	0	0	0	0	0	2	0	0	0	0	0
11:45 AM	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0
12:00 PM	0	0	0	0	0	0	0	0	0	0	2	0	0	0	0	0
12:15 PM	0	0	0	0	0	0	0	0	0	0	2	0	0	0	0	0
12:30 PM	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0
12:45 PM	0	0	0	0	0	0	0	0	0	0	2	0	0	0	0	0
1:00 PM	0	0	0	0	0	0	0	0	0	0	6	0	0	0	0	0
1:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:30 PM	0	0	0	0	0	0	0	0	0	0	2	0	0	0	0	0
1:45 PM	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0

MID PEAK HOUR		Hawkin	s Street			Drive	eway			Somervill	e Avenue			Somerville	e Avenue	
10:15 AM		Northbound				South	bound			Easth	oound			Westh	oound	
to	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
11:15 AM	0	0	0	1	0	0	0	0	0	0	12	0	0	0	0	0
PHF	0.25					0.	00			0.	75			0.	00	

Client: Zoe Dickerson 864\_013\_MM Project #: BTD #: Location 3 Somerville, MA Location: Street 1: Somerville Avenue Street 2: Hawkins Street 2/12/2022 Count Date: Day of Week: Saturday Clouds & Sun, 40°F Weather:



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#### PEDESTRIANS & BICYCLES

		Hawkin	s Street			Drive	eway			Somervill	e Avenue			Somervill	e Avenue	
		North	bound			South	bound			Eastl	oound			West	bound	
Start Time	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED
10:00 AM	0	0	0	19	0	0	0	28	0	5	1	3	0	0	0	0
10:15 AM	0	0	0	14	0	0	0	32	0	4	0	3	0	0	0	0
10:30 AM	0	0	0	26	0	0	0	47	0	8	0	8	0	0	0	3
10:45 AM	0	0	0	11	0	0	0	46	0	10	0	0	0	1	0	0
11:00 AM	0	0	0	27	0	0	0	29	0	6	1	4	0	0	0	0
11:15 AM	0	0	0	28	0	0	0	47	0	14	0	5	0	1	0	1
11:30 AM	0	0	0	25	0	0	0	44	0	7	0	3	0	0	0	0
11:45 AM	0	0	0	27	0	0	0	55	0	6	0	8	0	0	0	0
12:00 PM	0	0	0	26	0	0	0	75	0	5	0	3	0	1	0	4
12:15 PM	0	0	1	25	0	0	0	87	0	6	0	8	0	0	0	1
12:30 PM	0	0	0	39	0	0	0	95	0	18	0	9	0	1	0	4
12:45 PM	0	0	0	31	0	0	0	89	0	7	0	5	0	1	0	0
1:00 PM	0	0	2	20	0	0	0	96	0	12	0	19	0	0	0	1
1:15 PM	0	0	0	39	0	0	0	100	0	10	0	11	0	0	0	0
1:30 PM	0	0	0	33	0	0	0	138	0	8	0	11	0	0	0	0
1:45 PM	0	0	1	56	0	0	0	90	0	10	0	12	0	0	0	3

MID PEAK HOUR		Hawkin	s Street			Drive	eway			Somervill	e Avenue			Somervill	e Avenue	
12:30 PM		North	bound			South	bound			Easth	oound			West	oound	
to	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED
1:30 PM	0	0	2	129	0	0	0	380	0	47	0	44	0	2	0	5

NOTE: Peak hour summaries here correspond to peak hours identified for passenger car and heavy vehicles combined.

Client: Zoe Dickerson 864\_013\_MM Project #: BTD #: Location 3 Location: Somerville, MA Street 1: Somerville Avenue Hawkins Street Street 2: 2/15/2022 Count Date: Day of Week: Tuesday Weather: Clouds & Sun, 30°F



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## PASSENGER CARS & HEAVY VEHICLES COMBINED

						PASSEN	IGER CA	RS & HEA	AVY VEHI	CLES CO	MBINED					
		Hawkin	s Street			Driv	eway			Somervil	le Avenue			Somervil	le Avenue	
		North	bound			South	bound			East	bound			West	bound	
Start Time	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
6:00 AM	0	0	0	0	0	0	0	0	0	0	35	0	0	0	0	0
6:15 AM	0	0	0	5	0	0	0	0	0	0	43	0	0	0	0	0
6:30 AM	0	0	0	3	0	0	0	0	0	0	46	0	0	0	0	0
6:45 AM	0	0	0	2	0	1	0	0	0	0	60	0	0	0	0	0
7:00 AM	0	0	0	6	0	0	0	0	0	0	89	0	0	0	0	0
7:15 AM	0	0	0	15	0	0	0	0	0	0	109	0	0	0	0	0
7:30 AM	0	0	0	9	0	0	0	0	0	0	141	0	0	0	0	0
7:45 AM	0	0	0	20	0	0	0	0	0	0	151	0	0	0	0	0
8:00 AM 8:15 AM	0	0	0	18 24	0	0	0	0	0	0	183 168	0	0	0	0	0
8:30 AM	0	0	0	8	0	0	0	0	0	2	178	0	0	0	0	0
8:45 AM	0	0	0	5	0	0	0	0	0	0	160	0	0	0	0	0
9:00 AM	0	0	0	8	0	0	0	0	0	0	139	0	0	0	0	0
9:15 AM	0	0	0	3	0	2	0	0	0	0	119	0	0	0	0	0
9:30 AM	0	0	0	6	0	0	0	0	0	0	121	0	0	0	0	0
9:45 AM	0	0	0	10	0	0	0	0	0	0	113	0	0	0	0	0
10:00 AM	0	0	0	12	0	0	0	0	0	0	108	0	0	0	0	0
10:15 AM	0	0	0	5	0	1	0	0	0	1	94	0	0	0	0	0
10:30 AM	0	0	0	6	0	0	0	0	0	0	72	0	0	0	0	0
10:45 AM	0	0	1	6	0	0	0	0	0	0	122	0	0	0	0	0
11:00 AM	0	0	0	8	0	1	0	0	0	1	109	0	0	0	0	0
11:15 AM	0	0	0	7	0	0	0	0	0	0	108	0	0	0	0	0
11:30 AM	0	0	0	4	0	0	0	0	0	0	120	0	0	0	0	0
11:45 AM	0	0	1	8	0	0	0	0	0	0	147	0	0	0	0	0
12:00 PM	0	0	0	13	0	0	0	0	0	0	107	0	0	0	0	0
12:15 PM	0	0	0	14	0	0	0	0	0	0	134	0	0	0	0	0
12:30 PM	0	0	0	9	0	2	0	0	0	0	127	0	0	0	0	0
12:45 PM	0	0	0	7	0	0	0	0	0	0	123	0	0	0	0	0
1:00 PM	0	0	0	7	0	1	0	0	0	0	125	0	0	0	0	0
1:15 PM	0	0	0	10	0	0	0	0	0	0	120	0	0	0	0	0
1:30 PM 1:45 PM	0	0	0	8 9	0	0	0	0	0	0	124 115	0	0	0	0	0
2:00 PM	0	0	0	6	0	1	0	0	0	0	110	0	0	0	0	0
2:00 PM 2:15 PM	0	0	0	14	0	0	0	0	0	0	110	0	0	0	0	0
2:30 PM	0	0	0	18	0	0	0	0	0	0	128	0	0	0	0	0
2:45 PM	0	0	0	29	0	1	0	0	0	0	152	0	0	0	0	0
3:00 PM	0	0	0	23	0	0	0	0	0	0	141	0	0	0	0	0
3:15 PM	0	0	0	14	0	0	0	0	0	0	121	0	0	0	0	0
3:30 PM	0	0	0	17	0	1	0	0	0	0	117	0	0	0	0	0
3:45 PM	0	0	0	14	0	0	0	0	0	1	119	0	0	0	0	0
4:00 PM	0	0	1	19	0	1	0	0	0	0	116	0	0	0	0	0
4:15 PM	0	0	0	17	0	0	0	0	0	1	102	0	0	0	0	0
4:30 PM	0	0	0	11	0	0	0	0	0	0	122	0	0	0	0	0
4:45 PM	0	0	0	13	0	0	0	0	0	1	126	0	0	0	0	0
5:00 PM	0	0	0	17	0	0	0	0	0	0	157	0	0	0	0	0
5:15 PM	0	0	0	15	0	1	0	0	0	0	131	0	0	0	0	0
5:30 PM	0	0	0	22	0	0	0	0	0	1	122	0	0	0	0	0
5:45 PM	0	0	0	18	0	1	0	0	0	0	120	0	0	0	0	0
6:00 PM	0	0	0	19	0	1	0	0	0	0	116	0	0	0	0	0
6:15 PM	0	0	0	24	0	0	0	0	0	0	124	0	0	0	0	0
6:30 PM	0	0	0	17	0	0	0	0	0	0	112	0	0	0	0	0
6:45 PM	0	0	0	14	0	0	0	0	0	0	114	0	0	0	0	0
7:00 PM	0	0	2	11	0	1	0	0	0	0	98	0	0	0	0	0
7:15 PM 7:30 PM	0	0	0	6	0	0	0	0	0	0	102	0	0	0	0	0
		0	0	8	0	0	0	0	0	0	101	0	0		0	
7:45 PM	0	0	0	7	0	1	0	0	0	1	104	0	0	0	0	0

AM PEAK HOUR 7:45 AM			s Street bound				eway bound			Somervill Eastb	e Avenue oound			Somervill West	e Avenue cound	
to	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
8:45 AM	0 0 0 70			70	0	0	0	0	0	3	680	0	0	0	0	0
PHF	0.73					0.	00			0.	93			0.	00	
HV %	0.73 0.0% 0.0% 0.0% 2.9%			2.9%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	4.6%	0.0%	0.0%	0.0%	0.0%	0.0%

MID PEAK HOU 11:45 AM	R		s Street bound			Drive South	eway bound			Somervill Eastb	e Avenue oound				e Avenue bound	
to	U-Turn					Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
12:45 PM	0	0 0 1 44			0	2	0	0	0	0	515	0	0	0	0	0
PHF		0.80				0.	25			0.	88			0.	00	
HV %	0.0%				0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	7.2%	0.0%	0.0%	0.0%	0.0%	0.0%

PM PEAK HOUR 2:30 PM			s Street bound			Drive South	eway bound			Somervill Eastb	e Avenue oound			Somervill West		
to	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
3:30 PM	0	0	0	84	0	1	0	0	0	0	542	0	0	0	0	0
PHF	0.72					0.	25			0.	89			0.	00	
HV~%	0.0%				0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	3.9%	0.0%	0.0%	0.0%	0.0%	0.0%

Client: Zoe Dickerson 864\_013\_MM Project #: BTD #: Location 3 Somerville, MA Location: Street 1: Somerville Avenue Hawkins Street Street 2: 2/15/2022 Count Date: Day of Week: Tuesday Weather: Clouds & Sun, 30°F



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## HEAVY VEHICLES

								HEAVY V	/EHICLES	3						
		Hawkin	s Street			Driv	eway			Somervil	le Avenue			Somervil	le Avenue	
		North	bound			South	bound			Eastl	bound			West	bound	
Start Time	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
6:00 AM	0	0	0	0	0	0	0	0	0	0	4	0	0	0	0	0
6:15 AM	0	0	0	0	0	0	0	0	0	0	6	0	0	0	0	0
6:30 AM	0	0	0	1	0	0	0	0	0	0	2	0	0	0	0	0
6:45 AM	0	0	0	0	0	0	0	0	0	0	8	0	0	0	0	0
7:00 AM	0	0	0	0	0	0	0	0	0	0	5	0	0	0	0	0
7:15 AM	0	0	0	1	0	0	0	0	0	0	4	0	0	0	0	0
7:30 AM	0	0	0	0	0	0	0	0	0	0	7	0	0	0	0	0
7:45 AM	0	0	0	0	0	0	0	0	0	0	7	0	0	0	0	0
8:00 AM	0	0	0	0	0	0	0	0	0	0	8	0	0	0	0	0
8:15 AM	0	0	0	1	0	0	0	0	0	0	6	0	0	0	0	0
8:30 AM	0	0	0	1	0	0	0	0	0	0	10	0	0	0	0	0
8:45 AM	0	0	0	1	0	0	0	0	0	0	8	0	0	0	0	0
9:00 AM	0	0	0	0	0	0	0	0	0	0	8	0	0	0	0	0
9:15 AM	0			0	0	0	0	0	0	0	7		0	0	0	0
9:30 AM		0	0			0	0	0	0	0		0		0	0	0
9:30 AM 9:45 AM	0	0	0	0	0	0	0	0	0	0	8 9	0	0	0	0	0
				1		0										
10:00 AM	0	0	0		0	0	0	0	0	0	4	0	0	0	0	0
10:15 AM	0	0	0	0	0		0		0	1	7	0		0	0	0
10:30 AM	0	0	0	1	0	0	0	0	0	0	8	0	0	0	0	0
10:45 AM	0	0	0	1	0	0	0	0	0	0	9	0	0	0	0	
11:00 AM	0	0	0	0	0	0	0	0	0	0	10	0	0	0	0	0
11:15 AM	0	0	0	1	0	0	0	0	0	0	14	0	0	0	0	0
11:30 AM	0	0	0	0	0	0	0	0	0	0	4	0	0	0	0	0
11:45 AM	0	0	0	2	0	0	0	0	0	0	9	0	0	0	0	0
12:00 PM	0	0	0	0	0	0	0	0	0	0	8	0	0	0	0	0
12:15 PM	0	0	0	0	0	0	0	0	0	0	10	0	0	0	0	0
12:30 PM	0	0	0	2	0	0	0	0	0	0	10	0	0	0	0	0
12:45 PM	0	0	0	0	0	0	0	0	0	0	5	0	0	0	0	0
1:00 PM	0	0	0	0	0	0	0	0	0	0	9	0	0	0	0	0
1:15 PM	0	0	0	0	0	0	0	0	0	0	7	0	0	0	0	0
1:30 PM	0	0	0	0	0	0	0	0	0	0	5	0	0	0	0	0
1:45 PM	0	0	0	0	0	0	0	0	0	0	7	0	0	0	0	0
2:00 PM	0	0	0	0	0	0	0	0	0	0	6	0	0	0	0	0
2:15 PM	0	0	0	0	0	0	0	0	0	0	9	0	0	0	0	0
2:30 PM	0	0	0	1	0	0	0	0	0	0	3	0	0	0	0	0
2:45 PM	0	0	0	0	0	0	0	0	0	0	8	0	0	0	0	0
3:00 PM	0	0	0	0	0	0	0	0	0	0	6	0	0	0	0	0
3:15 PM	0	0	0	0	0	0	0	0	0	0	4	0	0	0	0	0
3:30 PM	0	0	0	1	0	0	0	0	0	0	4	0	0	0	0	0
3:45 PM	0	0	0	0	0	0	0	0	0	0	4	0	0	0	0	0
4:00 PM	0	0	0	0	0	0	0	0	0	0	3	0	0	0	0	0
4:15 PM	0	0	0	0	0	0	0	0	0	0	3	0	0	0	0	0
4:30 PM	0	0	0	0	0	0	0	0	0	0	2	0	0	0	0	0
4:45 PM	0	0	0	1	0	0	0	0	0	0	5	0	0	0	0	0
5:00 PM	0	0	0	0	0	0	0	0	0	0	2	0	0	0	0	0
5:15 PM	0	0	0	1	0	0	0	0	0	0	4	0	0	0	0	0
5:30 PM	0	0	0	0	0	0	0	0	0	0	3	0	0	0	0	0
5:45 PM	0	0	0	0	0	0	0	0	0	0	4	0	0	0	0	0
6:00 PM	0	0	0	1	0	0	0	0	0	0	2	0	0	0	0	0
6:15 PM	0	0	0	0	0	0	0	0	0	0	3	0	0	0	0	0
6:30 PM	0	0	0	0	0	0	0	0	0	0	2	0	0	0	0	0
6:45 PM	0	0	0	0	0	0	0	0	0	0	4	0	0	0	0	0
7:00 PM	0	0	0	0	0	0	0	0	0	0	2	0	0	0	0	0
7:15 PM	0	0	0	0	0	0	0	0	0	0	3	0	0	0	0	0
7:30 PM	0	0	0	0	0	0	0	0	0	0	2	0	0	0	0	0
7:45 PM	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0
1.40 FIVI	U	U	U	U	U	U	U	U	U	U		U	U	U	U	U

AM PEAK HO	UR	Hawk	ns Street			Driv	eway			Somervill	e Avenue			Somervill	e Avenue	
8:00 AM		Nort	hbound			South	bound			Easth	ound			West	bound	
to	U-Tur	n Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
9:00 AM	0	0	0	3	0	0	0	0	0	0	32	0	0	0	0	0
PHF			).75			0.	00			0.	80			0.	00	

MID PEAK HOUR	1	Hawkin	s Street			Driv	eway			Somervill	e Avenue			Somervill	le Avenue	
10:30 AM		Northbound					bound			Easth	oound			Westl	bound	
to	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
11:30 AM	0	0	0	3	0	0	0	0	0	0	41	0	0	0	0	0
PHF		0.75				0	00			0	73			0	00	

PM PEAK HOUR		Hawkin	s Street			Drive	eway			Somervill	e Avenue			Somervill	e Avenue	
2:00 PM		Northbound				South	bound			Easth	oound			Westl	oound	
to	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right
3:00 PM	0	0	0	1	0	0	0	0	0	0	26	0	0	0	0	0
PHF		0.25				0.	00			0.	72			0.	00	

Client: Zoe Dickerson 864\_013\_MM Project #: BTD #: Location 3 Location: Somerville, MA Street 1: Somerville Avenue Hawkins Street Street 2: 2/15/2022 Count Date: Day of Week: Tuesday Weather: Clouds & Sun, 30°F



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#### PEDESTRIANS & BICYCLES

			_					ES I RIAN	S & BIC T							
			s Street				eway				e Avenue				e Avenue	
			bound				bound				oound				bound	
Start Time	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED
6:00 AM	0	0	0	2	0	0	0	0	0	0	0	0	0	0	0	0
6:15 AM	0	0	0	2	0	0	0	2	0	0	0	0	0	0	0	0
6:30 AM	0	0	0	3	0	0	0	5	0	2	0	0	0	0	0	0
6:45 AM	0	0	0	8	0	0	0	6	0	1	0	2	0	1	0	0
7:00 AM	0	0	0	9	0	0	0	3	0	0	0	0	0	0	0	1
7:15 AM	0	0	0	6	0	0	0	6	0	7	0	1	0	0	0	0
7:30 AM	0	0	0	4	0	0	0	3	0	5	0	0	0	0	0	0
7:45 AM	0	0	0	6	0	0	0	4	0	9	0	0	0	0	0	0
8:00 AM	0	0	0	15	0	0	0	9	0	4	0	3	0	0	0	0
8:15 AM	0	0	0	8	0	0	0	8	0	10	0	2	0	0	0	1
8:30 AM	0	0	0	8	0	0	0	2	0	11	0	3	0	0	0	0
8:45 AM	0	0	0	8	0	0	0	15	0	15	0	1	0	0	0	0
9:00 AM	0	0	1	13	0	0	0	9	0	6	0	4	0	0	0	0
9:15 AM	0	0	0	4	0	0	0	11	0	6	0	0	0	0	0	0
9:30 AM	0	0	0	7	0	0	0	7	0	8	0	2	0	0	0	1
9:45 AM	0	0	0	9	0	0	0	15	0	4	0	2	0	1	0	0
10:00 AM	0	0	0	8	0	0	0	17	0	3	0	2	0	0	0	0
10:15 AM	0	0	0	3	0	0	0	5	0	3	0	0	0	0	0	0
10:30 AM	0	0	0	6	0	0	0	11	0	2	0	0	0	0	0	1
10:45 AM	0	0	0	7	0	0	0	12	0	3	0	1	0	0	0	2
11:00 AM	0	0	0	7	0	0	0	10	0	4	0	4	0	0	0	1
11:15 AM	0	0	0	8	0	0	0	8	0	2	0	1	0	2	0	0
11:15 AM 11:30 AM		0		6	0	0	0	13	0	4	0	0	0	0	0	0
	0		0													
11:45 AM	0	0	0	9	0	0	0	21	0	4	0	1	0	0	0	0
12:00 PM	0	0	0	3	0	0		23	0	2		4	0	0	0	0
12:15 PM	0	0	0	11	0	0	0	26	0	3	0	2	0	0	0	0
12:30 PM	0	0	0	10	0	0	0	18	0	3	0	3	0	0	0	0
12:45 PM	0	0	0	13	0	0	0	18	0	2	0	2	0	0	0	0
1:00 PM	0	0	0	14	0	0	0	17	0	0	0	1	0	0	0	2
1:15 PM	0	0	0	10	0	0	0	24	0	2	0	3	0	0	0	0
1:30 PM	0	0	0	9	0	0	0	16	0	2	0	2	0	0	0	2
1:45 PM	0	0	0	9	0	0	0	15	0	0	0	1	0	0	0	0
2:00 PM	0	0	0	14	0	0	0	16	0	2	0	2	0	0	0	0
2:15 PM	0	0	0	8	0	0	0	9	0	2	0	1	0	0	0	1
2:30 PM	0	0	0	12	0	0	0	10	0	0	0	0	0	0	0	1
2:45 PM	0	0	0	17	0	0	0	12	0	1	0	2	0	0	0	2
3:00 PM	0	0	1	6	0	0	0	17	0	0	0	1	0	0	0	0
3:15 PM	0	0	0	10	0	0	0	13	0	1	0	2	0	0	0	0
3:30 PM	0	0	0	14	0	0	0	16	0	1	0	2	0	0	0	1
3:45 PM	0	0	0	19	0	0	0	17	0	1	0	4	0	0	0	0
4:00 PM	0	0	0	18	0	0	0	13	0	0	0	4	0	1	0	2
4:15 PM	0	0	1	9	0	0	0	12	0	2	0	2	0	0	0	0
4:30 PM	0	0	0	15	0	0	0	11	0	1	0	5	0	1	0	0
4:45 PM	0	0	0	8	0	0	0	18	0	2	0	1	0	0	0	0
5:00 PM	0	0	0	18	0	0	0	21	0	1	0	0	0	0	0	0
5:15 PM	0	0	0	13	0	0	0	12	0	1	0	2	0	0	0	0
5:30 PM	0	0	1	18	0	0	0	9	0	2	0	1	0	0	0	0
5:45 PM	0	0	0	13	0	0	0	12	0	1	0	7	0	0	0	1
6:00 PM	0	0	0	14	0	0	0	11	0	0	0	5	0	0	0	0
6:15 PM	0	0	0	14	0	0	0	8	0	2	0	4	0	0	0	1
6:30 PM	0	0	0	10	0	0	0	7	0	1	0	1	0	0	0	1
6:45 PM	0	0	0	19	0	0	0	14	0	2	0	4	0	1	0	1
7:00 PM	0	0	0	12	0	0	0	10	0	1	0	1	0	0	0	0
7:15 PM	0	0	0	7	0	0	0	14	0	4	0	0	0	0	0	1
7:30 PM	0	0	0	9	0	0	0	13	0	3	0	4	0	0	0	1
7:45 PM	0	0	0	9	0	0	0	10	0	0	0	5	0	0	0	0
7.101.111					- v	·			·	·			·	·	·	

AM PEAK HOUR	1	Hawkin	s Street			Drive	eway			Somervill	e Avenue			Somervill	e Avenue	
7:45 AM		North	bound			South	bound			Easth	ound			West	bound	
to	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED
8:45 AM	0	0	0	37	0	0	0	23	0	34	0	8	0	0	0	1

MID PEAK HOUR	1	Hawkin	s Street			Drive	eway			Somervill	e Avenue			Somervill	e Avenue	
11:45 AM		North	bound			Southbound				Easth	ound			West	bound	
to	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED
12:45 PM	0	0	0	33	0	0	0	88	0	12	0	10	0	0	0	0

PM PEAK HOUR		Hawkin	s Street			Drive	eway			Somervill	e Avenue			Somervill	e Avenue	
2:30 PM		North	bound			South	bound			Easth	ound			West	oound	
to	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED	Left	Thru	Right	PED
3:30 PM	0	0	1	45	0	0	0	52	0	2	0	5	0	0	0	3

NOTE: Peak hour summaries here correspond to peak hours identified for passenger car and heavy vehicles combined.

Job 864\_013\_MM\_ATR 1

Area Somerville, MA

Location Somerville Avenue EB, between Hawkins Street & Bow Market Way

## Thursday, March 3, 2022



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Time	То	tal	E	В			Time	To	tal	Е	В		
0000	19		19		0		1200	124		124		0	
0015	13		13		0		1215	153		153		0	
0030	9		9		0		1230	111		111		0	
0045	10	51	10	51	0	0	1245	116	504	116	504	0	0
0100	10		10		0		1300	114		114		0	
0115	6		6		0		1315	115		115		0	
0130	4		4		0		1330	125		125		0	
0145	5	25	5	25	0	0	1345	141	495	141	495	0	0
0200	2		2		0		1400	138		138		0	
0215	7		7		0		1415	113		113		0	
0230	1		1		0		1430	121		121		0	
0245	6	16	6	16	0	0	1445	124	496	124	496	0	0
0300	2	. •	2		0	ŭ	1500	124		124		0	·
0315	3		3		0		1515	140		140		0	
0330	4		4		0		1530	126		126		0	
0345	4	13	4	13	0	0	1545	124	514	124	514	0	0
0400	7	. •	7		0	ŭ	1600	119	• • •	119	•	0	·
0415	6		6		0		1615	130		130		0	
0430	6		6		0		1630	113		113		0	
0445	14	33	14	33	0	0	1645	115	477	115	477	0	0
0500	8		8		0	· ·	1700	123		123		0	·
0515	22		22		0		1715	129		129		0	
0530	22		22		0		1730	136		136		0	
0545	25	77	25	77	0	0	1745	136	524	136	524	0	0
0600	33		33	• •	0	Ū	1800	102	02.	102	021	0	Ü
0615	53		53		0		1815	125		125		0	
0630	58		58		0		1830	107		107		0	
0645	76	220	76	220	0	0	1845	119	453	119	453	0	0
0700	83		83		0	ŭ	1900	96		96		0	·
0715	115		115		0		1915	113		113		0	
0730	110		110		0		1930	116		116		0	
0745	140	448	140	448	0	0	1945	91	416	91	416	0	0
0800	151		151	110	0	Ū	2000	82	110	82	110	0	Ü
0815	180		180		0		2015	73		73		0	
0830	147		147		0		2030	77		77		0	
0845	149	627	149	627	0	0	2045	75	307	75	307	0	0
0900	125	021	125	021	0	Ū	2100	82	001	82	001	0	O
0915	132		132		0		2115	47		47		0	
0930	107		107		0		2130	70		70		0	
0945	130	494	130	494	0	0	2145	58	257	58	257	0	0
1000	118	707	118	707	0	U	2200	54	201	54	201	0	O
1015	119		119		0		2215	44		44		0	
1013	114		114		0		2230	43		43		0	
1030	133	484	133	484	0	0	2245	41	182	41	182	0	0
1100	129	704	129	704	0	U	2300	30	102	30	102	0	J
1115	111		111		0		2315	27		27		0	
1130	120		120		0		2330	29		29		0	
1145	123	483	123	483	0	0	2345	28	114	28	114	0	0
1140	120	700	120	700	J	J	Total	<b>7710</b>	114	7710	117	0	J
							I Olai	7710		7710		9	

Job 864\_013\_MM\_ATR 1

Area Somerville, MA

Location Somerville Avenue EB, between Hawkins Street & Bow Market Way

# **BOSTON** TRAFFIC DATA

PO BOX 1723, Framingham, MA 01701 Office: 978-746-1259 DataRequest@BostonTrafficData.com www.BostonTrafficData.com

## Friday, March 4, 2022

	_		_	_					_			www.bost		
Time		tal		В				me		tal		В		
0000	31		31		0			200	107		107		0	
0015	20		20		0			215	89		89		0	
0030	15		15		0		12	230	126		126		0	
0045	11	77	11	77	0	0	12	245	143	465	143	465	0	0
0100	16		16		0		13	300	148		148		0	
0115	5		5		0			315	164		164		0	
0130	12		12		0			330	160		160		0	
0145	6	39	6	39	Ö	0		345	134	606	134	606	0	0
0200	2	00	2	00	0	Ū		100	143	000	143	000	0	Ŭ
0200	3		3		0			115	145		145		0	
0213														
	7	4.4	7	4.4	0	0		130	115	<b>540</b>	115	<b>540</b>	0	0
0245	2	14	2	14	0	0		145	139	542	139	542	0	0
0300	3		3		0			500	132		132		0	
0315	1		1		0			515	108		108		0	
0330	4		4		0			530	125		125		0	
0345	7	15	7	15	0	0		545	116	481	116	481	0	0
0400	6		6		0		16	600	169		169		0	
0415	6		6		0		16	315	121		121		0	
0430	5		5		0		16	330	145		145		0	
0445	11	28	11	28	0	0	16	345	145	580	145	580	0	0
0500	10		10		0		17	700	164		164		0	
0515	21		21		0			715	136		136		0	
0530	29		29		0			730	141		141		0	
0545	29	89	29	89	0	0		745	123	564	123	564	0	0
0600	40	03	40	03	0	U		300	129	304	129	304	0	U
0615	43		43		0			315	134		134		0	
0630	50	404	50	404	0	0		330	108	40.4	108	40.4	0	0
0645	58	191	58	191	0	0		345	63	434	63	434	0	0
0700	72		72		0			900	116		116		0	
0715	118		118		0			915	122		122		0	
0730	135		135		0			930	125		125		0	
0745	130	455	130	455	0	0		945	124	487	124	487	0	0
0800	158		158		0		20	000	97		97		0	
0815	151		151		0		20	)15	107		107		0	
0830	146		146		0		20	030	93		93		0	
0845	129	584	129	584	0	0	20	)45	97	394	97	394	0	0
0900	143		143		0			100	87		87		0	
0915	118		118		0			115	72		72		0	
0930	120		120		0			130	66		66		0	
0945	128	509	128	509	0	0		145	65	290	65	290	0	0
1000	106		106		0	•		200	65		65		0	Ü
1015	91		91		0			215	42		42		0	
1013	122		122		0			230			50		0	
		427		427		0			50	204		204		0
1045	108	427	108	427	0	0		245	47 55	204	47 55	204	0	0
1100	130		130		0			300	55		55		0	
1115	144		144		0			315	42		42		0	
1130	145	_	145		0			330	43		43		0	
1145	150	569	150	569	0	0		345	36	176	36	176	0	0
							To	otal	8220		8220		0	

Job 864\_013\_MM\_ATR 1

Area Somerville, MA

Location Somerville Avenue EB, between Hawkins Street & Bow Market Way

# **BOSTON** TRAFFIC DATA

PO BOX 1723, Framingham, MA 01701 Office: 978-746-1259 DataRequest@BostonTrafficData.com www.BostonTrafficData.com

## Saturday, March 5, 2022

Time													ontraincoata	com
0015	Time		otal		В			Time		tal		В		
0030														
0045         40         149         40         149         0         0         1245         83         432         83         432         0         0           0110         25         25         0         0         1315         132         132         0           0130         30         30         0         0         1330         143         143         0           0145         21         113         21         113         0         0         1430         135         135         0         0           0200         20         20         0         0         1440         135         135         0         0           0230         8         8         8         0         0         1445         109         516         0         0           0245         13         58         13         58         0         0         1445         109         516         0         0           0300         9         9         0         1500         139         139         139         0         0           0303         6         6         0         0         1545	0015	44		44		0		1215	142				0	
01100	0030	31		31		0		1230	88		88		0	
01100	0045	40	149	40	149	0	0	1245	83	432	83	432	0	0
0115				25				1300					0	
0130														
0240														
0200         20         20         0         1400         135         135         0           0215         17         17         0         1415         134         134         134         0           0230         8         8         0         0         1435         138         138         0           0245         13         58         13         58         0         0         1450         139         139         0           0300         9         9         0         1500         139         139         0           0315         4         4         4         0         1515         118         118         0           0330         6         6         6         0         0         1550         144         144         0           0345         5         24         5         24         0         0         1565         133         534         0         0           0445         6         25         6         25         0         0         1645         155         155         0         0           0430         3         3         0         <			113		113		0			502		502		0
0215         17         17         0         1415         134         134         0           0230         8         8         8         0         1430         138         138         0           0245         13         58         13         58         0         0         1445         109         516         10         0           0300         9         9         0         1500         139         139         0         0           0315         4         4         4         0         1515         118         118         0         0           0330         6         6         6         0         1530         144         144         0         0           0445         5         24         5         24         0         1645         153         534         133         534         0         0           0415         9         9         0         1615         155         155         0         0         0         1445         130         568         130         568         0         0         0         0         1430         13         13         0							Ū			002		002		Ū
0230         8         8         0         1440         138         138         0         0         1445         109         516         109         516         0														
D245														
0300         9         9         0         1500         139         139         0           0315         4         4         0         1515         118         118         0           0330         6         6         0         0         1530         144         144         0           0345         5         24         5         24         0         0         1545         133         534         13         534         0         0           04400         7         7         0         1600         150         150         0         0           0430         3         3         0         1630         133         133         0         0         1630         133         133         0			<b>50</b>		<b>50</b>		0			E40		E40		0
0315			30		30		U			310		310		U
0330         6         6         0         1530         144         144         0         0         0         1545         133         534         133         534         0														
0345         5         24         5         24         0         0         1545         133         534         133         534         0         0           04400         7         7         7         0         1600         150         150         0           0445         9         9         0         1630         133         133         0           0500         1         1         1         0         1770         139         139         0           0500         1         1         1         0         1770         139         139         0           0515         7         7         0         1715         105         105         0           0530         13         13         0         1773         101         101         0           0545         18         39         18         39         0         0         1745         120         465         120         465         0         0           0645         18         39         18         39         0         0         1745         120         465         120         465         0         0														
0400         7         7         0         1600         150         150         0           0415         9         9         0         1615         155         155         0           0430         3         3         0         1630         133         133         0           0445         6         25         6         25         0         0         1645         130         568         130         568         0         0           0500         1         1         0         1700         139         139         0         0           0515         7         7         7         0         1715         105         105         0           0530         13         13         0         1745         120         465         120         465         0           0645         18         39         18         39         0         0         1745         120         465         120         465         0         0           0600         13         13         13         0         1800         113         113         0         0         1845         113         113 </td <td></td> <td></td> <td></td> <td></td> <td><b>.</b>.</td> <td></td> <td>_</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td>_</td>					<b>.</b> .		_							_
0415         9         9         0         1615         155         155         0           0430         3         3         0         1630         133         133         0           0445         6         25         6         25         0         0         1645         130         568         130         568         0         0           0500         1         1         1         0         1700         139         139         0         0           0515         7         7         0         1715         105         105         0         0           0530         13         13         0         1730         101         101         0 <t< td=""><td></td><td></td><td>24</td><td></td><td>24</td><td></td><td>0</td><td></td><td></td><td>534</td><td></td><td>534</td><td></td><td>0</td></t<>			24		24		0			534		534		0
0430         3         3         0         1630         133         133         0 <td< td=""><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></td<>														
0445         6         25         6         25         0         0         1645         130         568         130         568         0         0           0500         1         1         0         1700         139         139         0         0           0515         7         7         0         1715         105         105         0         0           0530         13         13         0         1730         101         101         0         0           0645         18         39         18         39         0         0         1745         120         465         120         465         0         0           0645         30         30         0         1800         113         113         0         0         1815         113         113         0         0         0         1815         113         1113         0         0         0         0         1845         98         433         98         433         0         0         0         0         1845         98         433         98         433         0         0         0         1945         98		9												
0500         1         1         1         0         1700         139         139         0           0515         7         7         0         1715         105         105         0           0530         13         13         0         1730         101         101         0           0545         18         39         18         39         0         0         1745         120         465         120         465         0         0           0600         13         13         0         1800         113         113         0         0         0         0         1815         113         113         0         0         0         0         0         1815         113         113         0         0         0         0         0         1815         113         113         0         0         0         0         0         1815         113         113         0         0         0         0         0         0         0         1815         181         81         0         0         0         0         0         0         0         0         0         0         0		3		3		0		1630						
0515         7         7         0         1715         105         105         0           0530         13         13         0         1730         101         101         0           0545         18         39         18         39         0         0         1745         120         465         120         465         0         0           0600         13         13         0         1800         113         113         0         0           0615         30         30         0         0         1815         113         113         0           0645         37         106         37         106         0         0         1845         98         433         98         433         0           0700         27         27         0         1900         97         97         0         0           0715         33         33         33         0         1915         88         88         88         0           0730         47         47         0         1930         107         107         0           0745         65         172         6	0445	6	25	6	25		0	1645	130	568		568	0	0
0530         13         13         13         0         1730         101         101         0	0500	1		1		0		1700	139		139		0	
0545         18         39         18         39         0         0         1745         120         465         120         465         0         0           0600         13         13         0         1800         113         113         0         0           0615         30         30         0         1815         113         113         0         0           0630         26         26         0         1830         109         109         0         0           0645         37         106         37         106         0         0         1845         98         433         98         433         0         0           07700         27         27         0         1900         97         97         0         0         0         19715         88         88         0         0         0         1945         91         383         91         383         0         0         0         1945         91         383         91         383         0         0         0         0         0         1945         91         383         91         383         0         0	0515	7		7		0		1715	105		105		0	
0545         18         39         18         39         0         0         1745         120         465         120         465         0         0           0600         13         13         0         1800         113         113         0         0           0615         30         30         0         1815         113         113         0         0           0630         26         26         0         1830         109         109         0         0           0645         37         106         37         106         0         0         1845         98         433         98         433         0         0           07700         27         27         0         1900         97         97         0         0         0         19715         88         88         0         0         0         1945         91         383         91         383         0         0         0         1945         91         383         91         383         0         0         0         0         0         1945         91         383         91         383         0         0	0530	13		13		0		1730	101		101		0	
0600         13         13         0         1800         113         113         0           0615         30         30         0         1815         113         113         0           0630         26         26         0         1830         109         109         0           0645         37         106         0         0         1845         98         433         98         433         0           0700         27         27         0         1900         97         97         0           0715         33         33         0         1915         88         88         0           0730         47         47         0         1930         107         107         0           0745         65         172         65         172         0         1945         91         383         91         383         0         0           0800         70         70         0         2015         81         81         81         0         0         0         0         0         0         0         0         0         0         0         0         0	0545		39		39	0	0	1745	120	465	120	465	0	0
0615         30         30         0         1815         113         113         0           0630         26         26         0         1830         109         109         0           0645         37         106         37         106         0         0         1845         98         433         98         433         0         0           0700         27         27         0         1900         97         97         0						0		1800						
0630         26         26         0         1830         109         109         0           0645         37         106         37         106         0         0         1845         98         433         98         433         0         0           07700         27         27         0         1900         97         97         0         0           0715         33         33         0         1915         88         88         0         0           0730         47         47         0         1930         107         107         0         0           0745         65         172         65         172         0         0         1945         91         383         91         383         0         0           0800         70         70         0         2000         111         111         111         0									113					
0645         37         106         37         106         0         0         1845         98         433         98         433         0         0           0700         27         27         0         1900         97         97         0         0           0715         33         33         0         1915         88         88         0         0           0730         47         47         0         1930         107         107         0         0           0745         65         172         65         172         0         0         1945         91         383         91         383         0         0           0800         70         70         0         2000         111         111         0         0         111         111         0														
0700         27         27         0         1900         97         97         0           0715         33         33         0         1915         88         88         0           0730         47         47         0         1930         107         107         0           0745         65         172         65         172         0         0         1945         91         383         91         383         0         0           0800         70         70         0         2005         81         81         0			106		106		0			433		433		0
0715         33         33         0         1915         88         88         0           0730         47         47         0         1930         107         107         0           0745         65         172         65         172         0         0         1945         91         383         91         383         0         0           0800         70         70         0         2000         111         111         0							-							-
0730         47         47         0         1930         107         107         0           0745         65         172         65         172         0         0         1945         91         383         91         383         0         0           0800         70         70         0         2000         111         111         0 <td></td>														
0745         65         172         65         172         0         0         1945         91         383         91         383         0         0           0800         70         70         0         2000         111         111         0 <t< td=""><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></t<>														
0800         70         70         0         2000         111         111         0           0815         67         67         0         2015         81         81         0           0830         66         66         0         2030         38         38         0           0845         112         315         112         315         0         0         2045         39         269         39         269         0         0           0900         103         103         0         2100         67         67         0<			172		172		Λ			383		383		Λ
0815         67         67         0         2015         81         81         0           0830         66         66         0         2030         38         38         0           0845         112         315         112         315         0         0         2045         39         269         39         269         0         0           0900         103         103         0         2100         67         67         0			112		112		U			303		303		U
0830         66         66         0         2030         38         38         0           0845         112         315         112         315         0         0         2045         39         269         39         269         0         0           0900         103         103         0         2100         67         67         0         0           0915         106         106         0         2115         60         60         0         0           0930         97         97         0         2130         61         61         0<														
0845         112         315         112         315         0         0         2045         39         269         39         269         0         0           0900         103         103         0         2100         67         67         0         0           0915         106         106         0         2115         60         60         0         0           0930         97         97         0         2130         61         61         0         0           0945         117         423         117         423         0         0         2145         73         261         73         261         0         0           1000         95         95         0         2200         72         72         0         0         10														
0900       103       103       0       2100       67       67       0         0915       106       106       0       2115       60       60       0         0930       97       97       0       2130       61       61       0         0945       117       423       117       423       0       0       2145       73       261       73       261       0       0         1000       95       95       0       2200       72       72       0       0         1015       121       121       0       2215       55       55       0       0         1030       140       140       0       2230       52       52       0       0         1045       117       473       117       473       0       0       2245       51       230       51       230       0       0         1100       143       143       0       2300       55       55       0       0       0         1115       131       131       0       2315       39       39       0       0         1145       144 <td></td> <td></td> <td>245</td> <td></td> <td>245</td> <td></td> <td>0</td> <td></td> <td></td> <td>200</td> <td></td> <td>200</td> <td></td> <td>0</td>			245		245		0			200		200		0
0915       106       106       0       2115       60       60       0         0930       97       97       0       2130       61       61       0         0945       117       423       117       423       0       0       2145       73       261       73       261       0       0         1000       95       95       0       2200       72       72       0       0       1015       121       121       0       2215       55       55       0       0       1015       121       121       0       2230       52       52       0			315		315		U			269		269		U
0930         97         97         0         2130         61         61         0           0945         117         423         117         423         0         0         2145         73         261         73         261         0         0           1000         95         95         0         2200         72         72         0         0         10         10         10         10         10         22         10														
0945       117       423       117       423       0       0       2145       73       261       73       261       0       0         1000       95       95       0       2200       72       72       0       0         1015       121       121       0       2215       55       55       0       0         1030       140       140       0       2230       52       52       0       0         1045       117       473       117       473       0       0       2245       51       230       51       230       0       0         1100       143       143       0       2300       55       55       0       0       0         1115       131       131       0       2315       39       39       0       0       0       0       0       0       0       0         1130       143       143       0       2330       27       27       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0       0 <td></td>														
1000       95       95       0       2200       72       72       0         1015       121       121       0       2215       55       55       0         1030       140       140       0       2230       52       52       0         1045       117       473       117       473       0       0       2245       51       230       51       230       0       0         1100       143       143       0       2300       55       55       0       0         1115       131       131       0       2315       39       39       0       0         1130       143       143       0       2330       27       27       0       0         1145       144       561       0       0       2345       35       156       35       156       0       0			400		400		•			001		001		_
1015     121     121     0     2215     55     55     0       1030     140     140     0     2230     52     52     0       1045     117     473     117     473     0     0     2245     51     230     51     230     0     0       1100     143     143     0     2300     55     55     0       1115     131     131     0     2315     39     39     0       1130     143     143     0     2330     27     27     0       1145     144     561     0     0     2345     35     156     35     156     0     0			423		423		0			261		261		0
1030       140       140       0       2230       52       52       0         1045       117       473       117       473       0       0       2245       51       230       51       230       0       0         1100       143       143       0       2300       55       55       0														
1045     117     473     117     473     0     0     2245     51     230     51     230     0     0       1100     143     143     0     2300     55     55     0       1115     131     131     0     2315     39     39     0       1130     143     143     0     2330     27     27     0       1145     144     561     0     0     2345     35     156     35     156     0     0														
1100     143     143     0     2300     55     55     0       1115     131     131     0     2315     39     39     0       1130     143     143     0     2330     27     27     0       1145     144     561     0     0     2345     35     156     35     156     0     0														
1115     131     131     0     2315     39     39     0       1130     143     143     0     2330     27     27     0       1145     144     561     0     0     2345     35     156     35     156     0     0			473		473		0			230		230	0	0
1130     143     0     2330     27     27     0       1145     144     561     0     0     2345     35     156     35     156     0     0		143		143		0		2300	55		55		0	
1145 144 561 144 561 0 0 2345 35 156 35 156 0 0		131		131		0			39		39		0	
1145 144 561 144 561 0 0 2345 35 156 35 156 0 0	1130	143		143		0		2330	27		27		0	
		144	561		561		0	2345	35	156	35	156	0	0
								Total	7207		7207	_	0	

Job 864\_013\_MM\_ATR 2
Area Somerville, MA

Location Bow Street NB, between Warren Avenue & Walnut Street

## Thursday, March 3, 2022



PO BOX 1723, Framingham, MA 01701 Office: 978-746-1259 DataRequest@BostonTrafficData.com www.BostonTrafficData.com

Time	То	tal	N	IB			Time	То	tal	N	В		
0000	26		26	_	0		1200	125	••••	125	_	0	
0015	22		22		0		1215	154		154		0	
0030	18		18		0		1230	149		149		0	
0045	8	74	8	74	0	0	1245	156	584	156	584	0	0
0100	8		8		0		1300	109		109		0	
0115	10		10		0		1315	142		142		0	
0130	13		13		0		1330	132		132		0	
0145	4	35	4	35	0	0	1345	155	538	155	538	0	0
0200	6		6		0		1400	182		182		0	
0215	1		1		0		1415	154		154		0	
0230	7		7		0		1430	156		156		0	
0245	7	21	7	21	0	0	1445	192	684	192	684	0	0
0300	5		5		0		1500	173		173		0	
0315	9		9		0		1515	185		185		0	
0330	5		5		0		1530	173		173		0	
0345	7	26	7	26	0	0	1545	204	735	204	735	0	0
0400	6		6		0		1600	184		184		0	
0415	3		3		0		1615	187		187		0	
0430	10		10		0		1630	179		179		0	
0445	13	32	13	32	0	0	1645	200	750	200	750	0	0
0500	17		17		0		1700	191		191		0	
0515	21		21		0		1715	193		193		0	
0530	26		26		0		1730	206		206		0	
0545	39	103	39	103	0	0	1745	207	797	207	797	0	0
0600	39		39		0		1800	196		196		0	
0615	42		42		0		1815	178		178		0	
0630	45		45		0		1830	194		194		0	
0645	74	200	74	200	0	0	1845	158	726	158	726	0	0
0700	73		73		0		1900	131		131		0	
0715	84		84		0		1915	146		146		0	
0730	121		121		0		1930	137		137		0	
0745	132	410	132	410	0	0	1945	123	537	123	537	0	0
0800	151		151		0		2000	119		119		0	
0815	129		129		0		2015	118		118		0	
0830	134		134		0		2030	102		102		0	
0845	125	539	125	539	0	0	2045	90	429	90	429	0	0
0900	97		97		0		2100	96		96		0	
0915	103		103		0		2115	86		86		0	
0930	123		123		0		2130	71		71		0	
0945	125	448	125	448	0	0	2145	73	326	73	326	0	0
1000	109		109		0		2200	66		66		0	
1015	126		126		0		2215	60		60		0	
1030	123		123		0		2230	66		66		0	
1045	96	454	96	454	0	0	2245	44	236	44	236	0	0
1100	127		127		0		2300	38		38		0	
1115	122		122		0		2315	35		35		0	
1130	131		131		0		2330	28		28		0	
1145	152	532	152	532	0	0	2345	29	130	29	130	0	0
							Total	9346		9346		0	

Job 864\_013\_MM\_ATR 2
Area Somerville, MA

Location Bow Street NB, between Warren Avenue & Walnut Street

# **BOSTON** TRAFFIC DATA

PO BOX 1723, Framingham, MA 01701 Office: 978-746-1259 DataRequest@BostonTrafficData.com www.BostonTrafficData.com

## Friday, March 4, 2022

												tonTrafficData	.com
Time		tal		IB			Time		otal		В		
0000	33		33		0		1200	172		172		0	
0015	22		22		0		1215	146		146		0	
0030	19		19		0		1230	163		163		0	
0045	16	90	16	90	0	0	1245	176	657	176	657	0	0
0100	17		17		0		1300	141		141		0	
0115	12		12		0		1315	178		178		0	
0130	10		10		0		1330	158		158		0	
0145	5	44	5	44	0	0	1345	151	628	151	628	0	0
0200	4		4		0	-	1400	147		147		0	_
0215	5		5		0		1415	149		149		0	
0230	7		7		0		1430	142		142		0	
0245	7	23	7	23	0	0	1445	176	614	176	614	0	0
0300	2	23	2	23	0	U	1500	170	014	170	014	0	U
0300	9		9		0		1515	203		203		0	
								194					
0330	5	20	5	20	0	0	1530		750	194	750	0	^
0345	4	20	4	20	0	0	1545	185	752	185	752	0	0
0400	5		5		0		1600	219		219		0	
0415	4		4		0		1615	198		198		0	
0430	12		12		0		1630	182		182		0	_
0445	14	35	14	35	0	0	1645	190	789	190	789	0	0
0500	8		8		0		1700	209		209		0	
0515	24		24		0		1715	187		187		0	
0530	22		22		0		1730	188		188		0	
0545	40	94	40	94	0	0	1745	197	781	197	781	0	0
0600	36		36		0		1800	165		165		0	
0615	51		51		0		1815	159		159		0	
0630	45		45		0		1830	184		184		0	
0645	71	203	71	203	0	0	1845	186	694	186	694	0	0
0700	69		69		0		1900	165		165		0	
0715	81		81		0		1915	166		166		0	
0730	118		118		0		1930	169		169		0	
0745	140	408	140	408	0	0	1945	148	648	148	648	0	0
0800	163	400	163	400	0	U	2000	132	040	132	040	0	U
0815	155		155		0		2015	124		124		0	
0830	144		144		0		2030	118		118		0	
0845	127	589	127	589		0	2045	125	499	125	499	0	0
	127	509		309	0	U			499	116	499		U
0900			127		0		2100	116				0	
0915	118		118		0		2115	99		99		0	
0930	130	500	130	500	0	0	2130	112	440	112	440	0	0
0945	127	502	127	502	0	0	2145	91	418	91	418	0	0
1000	135		135		0		2200	73		73		0	
1015	121		121		0		2215	91		91		0	
1030	162		162		0		2230	53		53		0	
1045	109	527	109	527	0	0	2245	52	269	52	269	0	0
1100	173		173		0		2300	61		61		0	
1115	114		114		0		2315	58		58		0	
1130	142		142		0		2330	59		59		0	
1145	174	603	174	603	0	0	2345	56	234	56	234	0	0
							Total	10121		10121		0	

Job 864\_013\_MM\_ATR 2
Area Somerville, MA

Location Bow Street NB, between Warren Avenue & Walnut Street

# **BOSTON** TRAFFIC DATA

PO BOX 1723, Framingham, MA 01701 Office: 978-746-1259 DataRequest@BostonTrafficData.com www.BostonTrafficData.com

## Saturday, March 5, 2022

Tires	-	401		ID.			Time	-	4al		В		
Time		tal		IB			Time	To	tal		В	0	
0000	54		54		0		1200	166		166		0	
0015	39		39		0		1215	179		179		0	
0030	40	400	40	400	0	0	1230	169	675	169	675	0	0
0045	35	168	35	168	0	0	1245	161	675	161	675	0	0
0100	25		25		0		1300	158		158		0	
0115	26		26		0		1315	156		156		0	
0130	26	400	26	400	0	•	1330	178	0.40	178	0.40	0	•
0145	29	106	29	106	0	0	1345	156	648	156	648	0	0
0200	23		23		0		1400	156		156		0	
0215	21		21		0		1415	155		155		0	
0230	18		18		0		1430	149		149		0	
0245	16	78	16	78	0	0	1445	138	598	138	598	0	0
0300	14		14		0		1500	175		175		0	
0315	8		8		0		1515	154		154		0	
0330	8		8		0		1530	139		139		0	
0345	4	34	4	34	0	0	1545	153	621	153	621	0	0
0400	6		6		0		1600	184		184		0	
0415	7		7		0		1615	169		169		0	
0430	5		5		0		1630	154		154		0	
0445	2	20	2	20	0	0	1645	182	689	182	689	0	0
0500	8		8		0		1700	182		182		0	
0515	15		15		0		1715	169		169		0	
0530	20		20		0		1730	158		158		0	
0545	26	69	26	69	0	0	1745	156	665	156	665	0	0
0600	22		22		0		1800	154		154		0	
0615	31		31		0		1815	143		143		0	
0630	32		32		0		1830	143		143		0	
0645	32	117	32	117	0	0	1845	132	572	132	572	0	0
0700	42		42		0		1900	180		180		0	
0715	46		46		0		1915	124		124		0	
0730	61		61		0		1930	135		135		0	
0745	81	230	81	230	0	0	1945	136	575	136	575	0	0
0800	90		90		0		2000	113		113		0	
0815	80		80		0		2015	144		144		0	
0830	76		76		0		2030	107		107		0	
0845	110	356	110	356	0	0	2045	117	481	117	481	0	0
0900	120		120		0		2100	114		114		0	
0915	114		114		0		2115	89		89		0	
0930	125		125		0		2130	94		94		0	
0945	130	489	130	489	0	0	2145	88	385	88	385	0	0
1000	144		144		0		2200	74		74		0	
1015	148		148		0		2215	66		66		0	
1030	142		142		0		2230	93		93		0	
1045	148	582	148	582	0	0	2245	83	316	83	316	0	0
1100	143		143		0		2300	71		71		0	
1115	148		148		0		2315	61		61		0	
1130	146		146		0		2330	68		68		0	
1145	169	606	169	606	0	0	2345	55	255	55	255	0	0
							Total	9335		9335		0	

Job # 864\_013\_MM\_ATR 1

Area Somerville, MA

Location Somerville Avenue EB, between Hawkins Street & Bow Market Way

**Direction** Eastbound

Thursday, March 3, 2022



Time	Total	Class	Class	Class	Class	Class	Class	Class	Class	Class	Class	Class	Class	Class
		1	2	3	4	5	6	7	8	9	10	11	12	13
		Motorcycle	Passenger Car	Vans, Pick up Trucks	Bus	2 Axle 6 Tires	3 Axle Unit	4 Axles or more Unit	3 or 4 Axle Trailer	5 Axle Trailer	6 Axle or more Trailer	5 Axle or less Multi-Trailer	6 Axle Multi- Trailer	7 Axle or more Multi-Trailer
0000	51	0	48	1	2	0	0	0	0	0	0	0	0	0
0100	25	0	22	1	2	0	0	0	0	0	0	0	0	0
0200	16	0	13	3	0	0	0	0	0	0	0	0	0	0
0300	13	0	11	2	0	0	0	0	0	0	0	0	0	0
0400	33	0	21	4	0	3	4	0	0	1	0	0	0	0
0500	77	0	56	12	4	0	3	1	0	1	0	0	0	0
0600	220	0	168	38	8	0	3	0	1	2	0	0	0	0
0700	448	4	385	44	8	4	1	2	0	0	0	0	0	0
0800	627	6	573	28	7	4	3	2	2	2	0	0	0	0
0900	494	6	424	55	3	0	3	1	0	2	0	0	0	0
1000	484	0	416	48	4	8	5	0	0	2	1	0	0	0
1100	483	3	411	53	4	8	3	0	0	1	0	0	0	0
1200	504	1	438	46	6	7	3	1	0	2	0	0	0	0
1300	495	0	430	47	7	7	4	0	0	0	0	0	0	0
1400	496	0	422	55	6	10	1	1	0	1	0	0	0	0
1500	514	0	451	49	9	2	2	1	0	0	0	0	0	0
1600	477	3	425	32	5	5	2	3	1	1	0	0	0	0
1700	524	0	486	28	6	2	1	1	0	0	0	0	0	0
1800	453	1	427	16	6	1	0	1	0	1	0	0	0	0
1900	416	2	400	7	5	1	0	1	0	0	0	0	0	0
2000	307	0	296	9	2	0	0	0	0	0	0	0	0	0
2100	257	1	238	15	2	1	0	0	0	0	0	0	0	0
2200	182	0	176	4	1	0	0	1	0	0	0	0	0	0
2300	114	0	104	8	2	0	0	0	0	0	0	0	0	0
Total	7710	27	6841	605	99	63	38	16	4	16	1	0	0	0
	100.00%	0.35%	88.73%	7.85%	1.28%	0.82%	0.49%	0.21%	0.05%	0.21%	0.01%	0.00%	0.00%	0.00%

Job # 864\_013\_MM\_ATR 1

Area Somerville, MA

Location Somerville Avenue EB, between Hawkins Street & Bow Market Way

Direction Eastbound Friday, March 4, 2022

BOSTON TRAFFIC DATA PO BOX 1723, Framingham, MA 01701 Offic: 978-746-129 DataReques@BostonTrafficbata.com www.BostonTrafficbata.com

Time	Total	Class	Class	Class	Class	Class	Class	Class	Class	Class	Class	Class	Class	Class
		1	2	3	4	5	6	7	8	9	10	11	12	13
		Motorcycle	Passenger Car	Vans, Pick up Trucks	Bus	2 Axle 6 Tires	3 Axle Unit	4 Axles or more Unit	3 or 4 Axle Trailer	5 Axle Trailer	6 Axle or more Trailer	5 Axle or less Multi-Trailer	6 Axle Multi- Trailer	7 Axle or more Multi-Trailer
0000	77	0	71	4	2	0	0	0	0	0	0	0	0	0
0100	39	0	34	3	2	0	0	0	0	0	0	0	0	0
0200	14	0	13	1	0	0	0	0	0	0	0	0	0	0
0300	15	0	9	3	0	1	1	0	0	1	0	0	0	0
0400	28	0	19	4	0	3	2	0	0	0	0	0	0	0
0500	89	0	69	14	3	1	1	0	0	1	0	0	0	0
0600	191	1	146	28	8	2	2	1	0	3	0	0	0	0
0700	455	1	405	29	8	4	4	2	0	2	0	0	0	0
0800	584	1	529	31	9	5	6	1	0	2	0	0	0	0
0900	509	1	425	64	9	4	5	0	0	1	0	0	0	0
1000	427	2	374	41	6	2	2	0	0	0	0	0	0	0
1100	569	1	475	72	7	8	5	0	0	1	0	0	0	0
1200	465	1	401	50	1	6	4	0	0	2	0	0	0	0
1300	606	1	513	75	9	6	1	0	0	1	0	0	0	0
1400	542	0	482	51	8	0	1	0	0	0	0	0	0	0
1500	481	1	421	49	6	2	2	0	0	0	0	0	0	0
1600	580	0	539	31	5	5	0	0	0	0	0	0	0	0
1700	564	0	530	26	3	2	0	2	1	0	0	0	0	0
1800	434	0	420	11	2	0	0	1	0	0	0	0	0	0
1900	487	0	467	11	6	3	0	0	0	0	0	0	0	0
2000	394	0	385	8	1	0	0	0	0	0	0	0	0	0
2100	290	2	284	2	2	0	0	0	0	0	0	0	0	0
2200	204	2	195	4	2	1	0	0	0	0	0	0	0	0
2300	176	2	168	3	2	0	1	0	0	0	0	0	0	0
Total	8220	16	7374	615	101	55	37	7	1	14	0	0	0	0
	100.00%	0.19%	89.71%	7.48%	1.23%	0.67%	0.45%	0.09%	0.01%	0.17%	0.00%	0.00%	0.00%	0.00%

Job # 864\_013\_MM\_ATR 1

Area Somerville, MA

Location Somerville Avenue EB, between Hawkins Street & Bow Market Way

**Direction** Eastbound

Saturday, March 5, 2022



Time	Total	Class	Class	Class	Class	Class	Class	Class	Class	Class	Class	Class	Class	Class
		1	2	3	4	5	6	7	8	9	10	11	12	13
		Motorcycle	Passenger Car	Vans, Pick up Trucks	Bus	2 Axle 6 Tires	3 Axle Unit	4 Axles or more Unit	3 or 4 Axle Trailer	5 Axle Trailer	6 Axle or more Trailer	5 Axle or less Multi-Trailer	6 Axle Multi- Trailer	7 Axle or more Multi-Trailer
0000	149	0	143	2	2	0	0	1	0	1	0	0	0	0
0100	113	0	107	3	1	0	0	0	0	2	0	0	0	0
0200	58	0	57	1	0	0	0	0	0	0	0	0	0	0
0300	24	0	24	0	0	0	0	0	0	0	0	0	0	0
0400	25	0	22	3	0	0	0	0	0	0	0	0	0	0
0500	39	1	32	3	2	1	0	0	0	0	0	0	0	0
0600	106	0	87	12	2	1	2	0	0	2	0	0	0	0
0700	172	0	148	16	2	2	1	0	0	3	0	0	0	0
0800	315	0	296	13	4	0	0	0	1	1	0	0	0	0
0900	423	0	392	24	3	2	2	0	0	0	0	0	0	0
1000	473	0	443	22	3	5	0	0	0	0	0	0	0	0
1100	561	0	527	27	4	2	1	0	0	0	0	0	0	0
1200	432	1	411	18	0	1	1	0	0	0	0	0	0	0
1300	502	2	476	20	1	0	3	0	0	0	0	0	0	0
1400	516	3	492	12	3	1	3	2	0	0	0	0	0	0
1500	534	0	509	16	4	1	0	4	0	0	0	0	0	0
1600	568	2	549	9	2	1	4	1	0	0	0	0	0	0
1700	465	1	443	13	3	1	1	2	0	1	0	0	0	0
1800	433	2	414	11	2	1	3	0	0	0	0	0	0	0
1900	383	0	374	4	3	0	0	2	0	0	0	0	0	0
2000	269	1	257	5	3	0	2	0	0	0	1	0	0	0
2100	261	0	250	9	2	0	0	0	0	0	0	0	0	0
2200	230	0	224	4	1	0	1	0	0	0	0	0	0	0
2300	156	2	150	3	1	0	0	0	0	0	0	0	0	0
Total	7207	15	6827	250	48	19	24	12	1	10	1	0	0	0
	100.00%	0.21%	94.73%	3.47%	0.67%	0.26%	0.33%	0.17%	0.01%	0.14%	0.01%	0.00%	0.00%	0.00%

Job # 864\_013\_MM\_ATR 2

Area Somerville, MA

Location Bow Street NB, between Warren Avenue & Walnut Street

**Direction** Northbound

Thursday, March 3, 2022



Time	Total	Class	Class	Class	Class	Class	Class	Class	Class	Class	Class	Class	Class	Class
		1	2	3	4	5	6	7	8	9	10	11	12	13
		Motorcycle	Passenger Car	Vans, Pick up Trucks	Bus	2 Axle 6 Tires	3 Axle Unit	4 Axles or more Unit	3 or 4 Axle Trailer	5 Axle Trailer	6 Axle or more Trailer	5 Axle or less Multi-Trailer	6 Axle Multi- Trailer	7 Axle or more Multi-Trailer
0000	74	3	69	1	1	0	0	0	0	0	0	0	0	0
0100	35	1	29	3	2	0	0	0	0	0	0	0	0	0
0200	21	0	18	1	0	2	0	0	0	0	0	0	0	0
0300	26	0	16	5	0	2	1	0	0	2	0	0	0	0
0400	32	0	27	0	2	0	3	0	0	0	0	0	0	0
0500	103	0	74	14	9	4	1	0	0	1	0	0	0	0
0600	200	2	134	38	12	5	7	1	0	1	0	0	0	0
0700	410	6	330	51	6	10	3	3	0	1	0	0	0	0
0800	539	11	450	52	5	15	5	1	0	0	0	0	0	0
0900	448	11	344	62	8	14	6	1	0	1	0	0	0	1
1000	454	9	366	56	4	14	1	3	0	0	0	0	0	1
1100	532	12	440	57	4	7	4	6	0	1	0	0	0	1
1200	584	13	507	43	5	5	2	6	0	1	0	0	0	2
1300	538	17	453	51	7	5	2	1	0	0	0	0	0	2
1400	684	18	558	78	11	12	3	2	0	1	0	0	0	1
1500	735	16	647	57	4	4	2	2	0	1	0	0	0	2
1600	750	15	685	36	2	7	0	4	0	0	1	0	0	0
1700	797	19	724	46	4	3	0	1	0	0	0	0	0	0
1800	726	16	672	32	1	3	1	1	0	0	0	0	0	0
1900	537	8	504	13	9	1	0	0	0	1	0	0	0	1
2000	429	8	394	22	1	0	0	3	0	1	0	0	0	0
2100	326	8	303	14	1	0	0	0	0	0	0	0	0	0
2200	236	3	222	9	1	0	0	1	0	0	0	0	0	0
2300	130	1	120	8	1	0	0	0	0	0	0	0	0	0
Total	9346	197	8086	749	100	113	41	36	0	12	1	0	0	11
	100.00%	2.11%	86.52%	8.01%	1.07%	1.21%	0.44%	0.39%	0.00%	0.13%	0.01%	0.00%	0.00%	0.12%

Job # 864\_013\_MM\_ATR 2

Area Somerville, MA

Location Bow Street NB, between Warren Avenue & Walnut Street

**Direction** Northbound

Friday, March 4, 2022



Time	Total	Class	Class	Class	Class	Class	Class	Class	Class	Class	Class	Class	Class	Class
Tille	Total	Glass 4	Class 2	2	4	5	6	CidSS	Glass 8	9	10	11	12	13
			2	Vana Biak un	4	5	0	4 Avian au mana		9				
		Motorcycle	Passenger Car	Vans, Pick up Trucks	Bus	2 Axle 6 Tires	3 Axle Unit	4 Axles or more Unit	3 or 4 Axle Trailer	5 Axle Trailer	6 Axle or more Trailer	5 Axle or less Multi-Trailer	6 Axle Multi- Trailer	7 Axle or more Multi-Trailer
0000	90	0	80	4	2	3	1	0	0	0	0	0	0	0
0100	44	0	38	3	3	0	0	0	0	0	0	0	0	0
0200	23	0	19	1	0	2	0	0	0	1	0	0	0	0
0300	20	0	16	1	0	1	1	0	0	0	1	0	0	0
0400	35	0	22	4	4	2	2	0	0	1	0	0	0	0
0500	94	1	56	18	10	3	2	1	0	3	0	0	0	0
0600	203	2	140	28	17	7	7	1	0	1	0	0	0	0
0700	408	3	328	48	16	6	3	2	0	1	0	0	0	1
0800	589	14	494	50	6	12	7	2	0	4	0	0	0	0
0900	502	6	413	61	4	10	4	4	0	0	0	0	0	0
1000	527	13	435	55	7	8	4	5	0	0	0	0	0	0
1100	603	10	497	64	5	15	4	6	0	0	0	0	0	2
1200	657	24	537	62	14	9	4	4	0	3	0	0	0	0
1300	628	19	513	71	8	8	1	4	0	2	0	0	0	2
1400	614	10	524	51	12	8	0	6	0	0	0	0	0	3
1500	752	13	658	60	5	4	2	6	0	1	0	0	0	3
1600	789	22	710	42	4	5	0	5	0	1	0	0	0	0
1700	781	15	717	33	6	3	1	4	0	0	0	0	0	2
1800	694	24	635	22	7	3	0	1	0	2	0	0	0	0
1900	648	5	623	15	1	3	1	0	0	0	0	0	0	0
2000	499	4	471	16	2	1	3	2	0	0	0	0	0	0
2100	418	6	394	12	4	1	0	1	0	0	0	0	0	0
2200	269	5	250	11	2	1	0	0	0	0	0	0	0	0
2300	234	1	223	8	0	0	0	1	0	1	0	0	0	0
Total	10121	197	8793	740	139	115	47	55	0	21	1	0	0	13
	100.00%	1.95%	86.88%	7.31%	1.37%	1.14%	0.46%	0.54%	0.00%	0.21%	0.01%	0.00%	0.00%	0.13%

Job # 864\_013\_MM\_ATR 2

Area Somerville, MA

Location Bow Street NB, between Warren Avenue & Walnut Street

**Direction** Northbound

Saturday, March 5, 2022



Time	Total	Class	Class	Class	Class	Class	Class	Class	Class	Class	Class	Class	Class	Class
		1	2	3	4	5	6	7	8	9	10	11	12	13
		Motorcycle	Passenger Car	Vans, Pick up Trucks	Bus	2 Axle 6 Tires	3 Axle Unit	4 Axles or more Unit	3 or 4 Axle Trailer	5 Axle Trailer	6 Axle or more Trailer	5 Axle or less Multi-Trailer	6 Axle Multi- Trailer	7 Axle or more Multi-Trailer
0000	168	1	159	5	3	0	0	0	0	0	0	0	0	0
0100	106	1	96	6	2	0	0	0	0	1	0	0	0	0
0200	78	0	70	8	0	0	0	0	0	0	0	0	0	0
0300	34	1	32	1	0	0	0	0	0	0	0	0	0	0
0400	20	0	20	0	0	0	0	0	0	0	0	0	0	0
0500	69	0	50	11	2	4	1	0	0	1	0	0	0	0
0600	117	1	95	10	3	4	2	1	0	1	0	0	0	0
0700	230	2	187	29	7	2	1	0	0	2	0	0	0	0
0800	356	4	312	31	3	3	3	0	0	0	0	0	0	0
0900	489	9	424	43	6	2	2	1	0	0	0	0	0	2
1000	582	8	522	34	5	6	3	2	0	0	0	0	0	2
1100	606	24	541	21	4	5	5	3	0	2	0	0	0	1
1200	675	15	608	21	10	6	2	8	0	0	0	0	0	5
1300	648	21	573	26	15	6	2	4	0	0	0	0	0	1
1400	598	15	543	26	8	4	1	0	0	0	0	0	0	1
1500	621	7	574	21	9	3	1	5	0	1	0	0	0	0
1600	689	18	640	20	0	3	3	1	0	1	0	0	0	3
1700	665	18	600	28	8	2	0	4	0	0	0	0	0	5
1800	572	7	532	19	9	0	3	0	0	1	0	0	0	1
1900	575	16	540	15	1	0	0	0	0	2	0	0	0	1
2000	481	5	454	18	2	1	0	1	0	0	0	0	0	0
2100	385	5	361	16	1	1	1	0	0	0	0	0	0	0
2200	316	1	298	14	1	1	0	1	0	0	0	0	0	0
2300	255	4	239	10	1	0	0	0	0	0	0	0	0	1
Total	9335	183	8470	433	100	53	30	31	0	12	0	0	0	23
	100.00%	1.96%	90.73%	4.64%	1.07%	0.57%	0.32%	0.33%	0.00%	0.13%	0.00%	0.00%	0.00%	0.25%

Job 864\_013\_MM\_ATR 1 Area Somerville, MA

Location Somerville Avenue EB, between Hawkins Street & Bow Market Way

Dir Eastbound Thursday, March 3, 2022



PO BOX 1723, Framingham, MA 01701 Office: 978-746-1259 DataRequest@BostonTrafficData.com www.BostonTrafficData.com

Time	Total							Spee	d Bins (m	ph)							
		0	5	10	15	20	25	30	35	40	45	50	55	60	65	70	75
		5	10	15	20	25	30	35	40	45	50	55	60	65	70	75	80
0000	51	0	0	2	3	18	19	9	0	0	0	0	0	0	0	0	0
0100	25	0	0	0	0	8	10	6	1	0	0	0	0	0	0	0	0
0200	16	0	0	0	3	3	7	3	0	0	0	0	0	0	0	0	0
0300	13	0	0	0	1	6	4	2	0	0	0	0	0	0	0	0	0
0400	33	0	0	0	2	7	10	11	2	1	0	0	0	0	0	0	0
0500	77	0	0	1	5	18	33	16	4	0	0	0	0	0	0	0	0
0600	220	0	1	8	25	82	80	22	1	1	0	0	0	0	0	0	0
0700	448	0	6	18	115	206	95	8	0	0	0	0	0	0	0	0	0
0800	627	2	31	95	215	212	64	7	1	0	0	0	0	0	0	0	0
0900	494	0	21	72	158	172	62	6	3	0	0	0	0	0	0	0	0
1000	484	0	14	52	166	180	64	5	3	0	0	0	0	0	0	0	0
1100	483	0	11	46	202	181	39	3	1	0	0	0	0	0	0	0	0
1200	504	0	34	133	211	106	19	1	0	0	0	0	0	0	0	0	0
1300	495	1	29	111	191	125	33	3	2	0	0	0	0	0	0	0	0
1400	496	4	60	150	178	89	15	0	0	0	0	0	0	0	0	0	0
1500	514	0	12	48	211	191	46	5	1	0	0	0	0	0	0	0	0
1600	477	0	3	28	141	225	76	4	0	0	0	0	0	0	0	0	0
1700	524	0	2	26	167	263	61	4	1	0	0	0	0	0	0	0	0
1800	453	0	8	20	170	199	52	4	0	0	0	0	0	0	0	0	0
1900	416	1	10	28	144	184	44	4	1	0	0	0	0	0	0	0	0
2000	307	0	14	26	74	138	48	6	1	0	0	0	0	0	0	0	0
2100	257	0	2	7	68	120	52	8	0	0	0	0	0	0	0	0	0
2200	182	0	1	2	24	89	60	5	1	0	0	0	0	0	0	0	0
2300	114	0	0	2	13	46	45	7	1	0	0	0	0	0	0	0	0
Total	7710	8	259	875	2487	2868	1038	149	24	2	0	0	0	0	0	0	0

100.00% 0.10% 3.36% 11.35% 32.26% 37.20% 13.46% 1.93% 0.31% 0.03% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00%

Maximum = 42.3 mph, Minimum = 3.6 mph, Mean = 20.1 mph 85% Speed = 25.05 mph, 95% Speed = 27.91 mph, Median = 20.30 mph 10 mph Pace = 16 - 26, Number in Pace = 5209 (70.42%) Variance = 25.37, Standard Deviation = 5.04 mph

Job 864\_013\_MM\_ATR 1 Area Somerville, MA

Location Somerville Avenue EB, between Hawkins Street & Bow Market Way

Dir Eastbound Friday, March 4, 2022



PO BOX 1723, Framingham, MA 01701 Office: 978-746-1259 DataRequest@BostonTrafficData.com www.BostonTrafficData.com

Time	Total							Spee	d Bins (m	ph)							
		0	5	10	15	20	25	30	35 `	40	45	50	55	60	65	70	75
		5	10	15	20	25	30	35	40	45	50	55	60	65	70	75	80
0000	77	0	0	4	8	28	25	11	1	0	0	0	0	0	0	0	0
0100	39	0	1	0	4	12	17	4	1	0	0	0	0	0	0	0	0
0200	14	0	0	0	2	2	6	4	0	0	0	0	0	0	0	0	0
0300	15	0	0	0	1	5	4	4	1	0	0	0	0	0	0	0	0
0400	28	0	0	0	1	2	13	11	1	0	0	0	0	0	0	0	0
0500	89	0	0	2	6	22	38	20	1	0	0	0	0	0	0	0	0
0600	191	0	3	2	22	68	77	17	2	0	0	0	0	0	0	0	0
0700	455	0	1	29	125	211	74	13	2	0	0	0	0	0	0	0	0
0800	584	0	7	117	241	166	47	6	0	0	0	0	0	0	0	0	0
0900	509	1	9	49	155	233	55	6	1	0	0	0	0	0	0	0	0
1000	427	0	9	75	161	152	29	1	0	0	0	0	0	0	0	0	0
1100	569	3	12	93	265	165	28	3	0	0	0	0	0	0	0	0	0
1200	465	1	14	64	189	163	29	5	0	0	0	0	0	0	0	0	0
1300	606	0	47	263	193	97	6	0	0	0	0	0	0	0	0	0	0
1400	542	5	31	98	210	155	41	1	1	0	0	0	0	0	0	0	0
1500	481	0	13	75	187	153	47	6	0	0	0	0	0	0	0	0	0
1600	580	0	8	59	179	271	55	8	0	0	0	0	0	0	0	0	0
1700	564	2	24	92	194	203	45	3	1	0	0	0	0	0	0	0	0
1800	434	0	17	81	196	119	20	1	0	0	0	0	0	0	0	0	0
1900	487	0	27	85	221	129	22	2	1	0	0	0	0	0	0	0	0
2000	394	1	19	65	171	119	18	1	0	0	0	0	0	0	0	0	0
2100	290	3	8	47	130	85	16	0	1	0	0	0	0	0	0	0	0
2200	204	1	10	12	47	98	31	5	0	0	0	0	0	0	0	0	0
2300	176	0	4	17	55	79	21	0	0	0	0	0	0	0	0	0	0
Total	8220	17	264	1329	2963	2737	764	132	14	0	0	0	0	0	0	0	0

100.00% 0.21% 3.21% 16.17% 36.05% 33.30% 9.29% 1.61% 0.17% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00%

Maximum = 38.2 mph, Minimum = 3.3 mph, Mean = 19.2 mph 85% Speed = 24.10 mph, 95% Speed = 27.35 mph, Median = 19.35 mph 10 mph Pace = 14 - 24, Number in Pace = 5747 (69.91%) Variance = 24.96, Standard Deviation = 5.00 mph

Job 864\_013\_MM\_ATR 1 Area Somerville, MA

Location Somerville Avenue EB, between Hawkins Street & Bow Market Way

Dir Eastbound Saturday, March 5, 2022



PO BOX 1723, Framingham, MA 01701 Office: 978-746-1259 DataRequest@BostonTrafficData.com www.BostonTrafficData.com

Time	Total							Spee	d Bins (m	ph)							
		0	5	10	15	20	25	30	35	40	45	50	55	60	65	70	75
		5	10	15	20	25	30	35	40	45	50	55	60	65	70	75	80
0000	149	0	6	17	44	58	21	3	0	0	0	0	0	0	0	0	0
0100	113	0	2	5	19	49	29	8	1	0	0	0	0	0	0	0	0
0200	58	0	1	0	4	26	21	6	0	0	0	0	0	0	0	0	0
0300	24	0	0	0	2	9	11	2	0	0	0	0	0	0	0	0	0
0400	25	0	0	0	1	7	13	2	1	1	0	0	0	0	0	0	0
0500	39	0	1	2	5	10	16	5	0	0	0	0	0	0	0	0	0
0600	106	0	1	2	9	40	43	9	2	0	0	0	0	0	0	0	0
0700	172	0	0	1	12	84	62	13	0	0	0	0	0	0	0	0	0
0800	315	0	4	11	63	183	49	4	1	0	0	0	0	0	0	0	0
0900	423	0	13	49	176	157	21	6	1	0	0	0	0	0	0	0	0
1000	473	2	32	105	189	125	15	5	0	0	0	0	0	0	0	0	0
1100	561	3	20	88	250	171	27	1	1	0	0	0	0	0	0	0	0
1200	432	2	34	112	159	112	12	1	0	0	0	0	0	0	0	0	0
1300	502	1	24	104	250	117	5	1	0	0	0	0	0	0	0	0	0
1400	516	3	30	143	235	89	15	1	0	0	0	0	0	0	0	0	0
1500	534	1	17	143	255	100	16	2	0	0	0	0	0	0	0	0	0
1600	568	5	37	121	278	108	17	1	1	0	0	0	0	0	0	0	0
1700	465	2	25	70	234	121	12	1	0	0	0	0	0	0	0	0	0
1800	433	5	20	97	195	104	12	0	0	0	0	0	0	0	0	0	0
1900	383	3	13	88	179	87	11	1	0	1	0	0	0	0	0	0	0
2000	269	0	10	49	114	84	11	0	1	0	0	0	0	0	0	0	0
2100	261	0	8	32	112	95	11	3	0	0	0	0	0	0	0	0	0
2200	230	2	12	26	80	84	24	2	0	0	0	0	0	0	0	0	0
2300	156	0	5	9	37	89	14	2	0	0	0	0	0	0	0	0	0
Total	7207	29	315	1274	2902	2109	488	79	9	2	0	0	0	0	0	0	0

100.00% 0.40% 4.37% 17.68% 40.27% 29.26% 6.77% 1.10% 0.12% 0.03% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00%

Maximum = 41.9 mph, Minimum = 2.9 mph, Mean = 18.4 mph 85% Speed = 23.26 mph, 95% Speed = 26.34 mph, Median = 18.62 mph 10 mph Pace = 14 - 24, Number in Pace = 5154 (71.51%) Variance = 23.99, Standard Deviation = 4.90 mph

Job 864\_013\_MM\_ATR 2 Area Somerville, MA

Location Bow Street NB, between Warren Avenue & Walnut Street

Dir Northbound Thursday, March 3, 2022



PO BOX 1723, Framingham, MA 01701 Office: 978-746-1259 DataRequest@BostonTrafficData.com www.BostonTrafficData.com

Time	Total							Spee	d Bins (m	ıph)							
		0	5	10	15	20	25	30	35	40	45	50	55	60	65	70	75
		5	10	15	20	25	30	35	40	45	50	55	60	65	70	75	80
0000	74	0	3	12	8	30	17	4	0	0	0	0	0	0	0	0	0
0100	35	0	0	2	2	15	12	4	0	0	0	0	0	0	0	0	0
0200	21	0	0	1	2	11	5	2	0	0	0	0	0	0	0	0	0
0300	26	0	1	4	3	6	8	4	0	0	0	0	0	0	0	0	0
0400	32	0	2	0	7	5	13	5	0	0	0	0	0	0	0	0	0
0500	103	0	1	12	13	40	28	9	0	0	0	0	0	0	0	0	0
0600	200	1	9	27	45	67	48	3	0	0	0	0	0	0	0	0	0
0700	410	1	26	129	126	98	27	3	0	0	0	0	0	0	0	0	0
0800	539	3	48	189	180	104	15	0	0	0	0	0	0	0	0	0	0
0900	448	1	55	163	142	82	5	0	0	0	0	0	0	0	0	0	0
1000	454	6	55	160	181	48	4	0	0	0	0	0	0	0	0	0	0
1100	532	2	92	190	182	59	5	1	1	0	0	0	0	0	0	0	0
1200	584	21	145	200	174	43	1	0	0	0	0	0	0	0	0	0	0
1300	538	8	136	202	145	39	7	1	0	0	0	0	0	0	0	0	0
1400	684	26	197	274	142	40	5	0	0	0	0	0	0	0	0	0	0
1500	735	9	171	307	177	67	4	0	0	0	0	0	0	0	0	0	0
1600	750	2	91	276	292	79	7	3	0	0	0	0	0	0	0	0	0
1700	797	5	120	324	252	86	10	0	0	0	0	0	0	0	0	0	0
1800	726	3	97	253	283	83	6	0	1	0	0	0	0	0	0	0	0
1900	537	13	103	177	170	64	9	1	0	0	0	0	0	0	0	0	0
2000	429	0	31	120	142	116	19	0	1	0	0	0	0	0	0	0	0
2100	326	1	18	67	125	103	11	1	0	0	0	0	0	0	0	0	0
2200	236	0	8	36	61	107	22	1	1	0	0	0	0	0	0	0	0
2300	130	0	3	15	31	45	31	4	1	0	0	0	0	0	0	0	0
Total	9346	102	1412	3140	2885	1437	319	46	5	0	0	0	0	0	0	0	0

100.00% 1.09% 15.11% 33.60% 30.87% 15.38% 3.41% 0.49% 0.05% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00%

Maximum = 40.0 mph, Minimum = 2.2 mph, Mean = 15.4 mph 85% Speed = 20.92 mph, 95% Speed = 24.33 mph, Median = 15.04 mph 10 mph Pace = 9 - 19, Number in Pace = 6049 (64.72%) Variance = 27.56, Standard Deviation = 5.25 mph

Job 864\_013\_MM\_ATR 2 Area Somerville, MA

Location Bow Street NB, between Warren Avenue & Walnut Street

Dir Northbound Friday, March 4, 2022



PO BOX 1723, Framingham, MA 01701 Office: 978-746-1259 DataRequest@BostonTrafficData.com www.BostonTrafficData.com

Time	Total							0	al Dina /m	la \					StofffameData		
Time	Total		_	40	45	00	0.5		d Bins (m		45	<b>50</b>		00	0.5	70	7.5
		0	5	10	15	20	25	30	35	40	45	50	55	60	65	70	75
2222		5	10	15	20	25	30	35	40	45	50	55	60	65	70	75	80
0000	90	0	1	8	12	41	27	1	0	0	0	0	0	0	0	0	0
0100	44	0	0	2	8	21	12	1	0	0	0	0	0	0	0	0	0
0200	23	0	0	1	4	8	9	1	0	0	0	0	0	0	0	0	0
0300	20	0	0	1	4	6	6	2	1	0	0	0	0	0	0	0	0
0400	35	0	1	1	4	11	12	5	1	0	0	0	0	0	0	0	0
0500	94	0	3	7	12	27	31	12	2	0	0	0	0	0	0	0	0
0600	203	0	8	28	25	75	52	12	3	0	0	0	0	0	0	0	0
0700	408	0	29	128	117	98	33	2	0	1	0	0	0	0	0	0	0
0800	589	5	92	228	185	66	13	0	0	0	0	0	0	0	0	0	0
0900	502	0	61	182	150	99	10	0	0	0	0	0	0	0	0	0	0
1000	527	9	127	155	171	56	9	0	0	0	0	0	0	0	0	0	0
1100	603	5	137	213	197	46	4	1	0	0	0	0	0	0	0	0	0
1200	657	19	194	265	137	39	3	0	0	0	0	0	0	0	0	0	0
1300	628	45	173	191	163	53	3	0	0	0	0	0	0	0	0	0	0
1400	614	10	193	237	135	37	2	0	0	0	0	0	0	0	0	0	0
1500	752	9	179	293	211	53	7	0	0	0	0	0	0	0	0	0	0
1600	789	6	141	345	232	54	11	0	0	0	0	0	0	0	0	0	0
1700	781	10	221	315	196	37	2	0	0	0	0	0	0	0	0	0	0
1800	694	22	190	260	171	48	3	0	0	0	0	0	0	0	0	0	0
1900	648	4	131	243	172	84	11	3	0	0	0	0	0	0	0	0	0
2000	499	10	68	156	188	70	7	0	0	0	0	0	0	0	0	0	0
2100	418	3	51	132	156	71	5	0	0	0	0	0	0	0	0	0	0
2200	269	3	22	57	87	87	11	2	0	0	0	0	0	0	0	0	0
2300	234	0	10	31	60	99	29	4	1	0	0	0	0	0	0	0	0
Total	10121	160	2032	3479	2797	1286	312	46	8	1	0	0	0	0	0	0	0
Total	10121	100	2002	J-1 3	2131	1200	312	70	0		U	U	0	U	U	U	J

100.00% 1.58% 20.08% 34.37% 27.64% 12.71% 3.08% 0.45% 0.08% 0.01% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00%

Maximum = 41.5 mph, Minimum = 2.2 mph, Mean = 14.6 mph 85% Speed = 20.34 mph, 95% Speed = 24.05 mph, Median = 14.15 mph 10 mph Pace = 9 - 19, Number in Pace = 6486 (64.08%) Variance = 28.95, Standard Deviation = 5.38 mph

Job 864\_013\_MM\_ATR 2 Area Somerville, MA

Location Bow Street NB, between Warren Avenue & Walnut Street

Dir Northbound **Saturday, March 5, 2022** 



PO BOX 1723, Framingham, MA 01701 Office: 978-746-1259 DataRequest@BostonTrafficData.com www.BostonTrafficData.com

Time	Total	Speed Bins (mph)															
		0	5	10	15	20	25	30	35	40	45	50	55	60	65	70	75
		5	10	15	20	25	30	35	40	45	50	55	60	65	70	75	80
0000	168	2	6	26	30	72	25	6	0	1	0	0	0	0	0	0	0
0100	106	0	3	8	13	48	29	5	0	0	0	0	0	0	0	0	0
0200	78	0	0	4	5	29	31	8	1	0	0	0	0	0	0	0	0
0300	34	0	1	2	3	12	11	4	1	0	0	0	0	0	0	0	0
0400	20	0	0	0	4	4	10	2	0	0	0	0	0	0	0	0	0
0500	69	0	0	8	11	23	23	4	0	0	0	0	0	0	0	0	0
0600	117	0	3	20	26	38	28	2	0	0	0	0	0	0	0	0	0
0700	230	0	8	38	70	71	35	8	0	0	0	0	0	0	0	0	0
0800	356	3	38	106	127	65	15	2	0	0	0	0	0	0	0	0	0
0900	489	9	113	175	144	46	2	0	0	0	0	0	0	0	0	0	0
1000	582	19	199	205	115	42	2	0	0	0	0	0	0	0	0	0	0
1100	606	35	245	189	104	30	2	1	0	0	0	0	0	0	0	0	0
1200	675	37	280	231	112	12	3	0	0	0	0	0	0	0	0	0	0
1300	648	46	295	214	81	12	0	0	0	0	0	0	0	0	0	0	0
1400	598	31	228	219	99	17	4	0	0	0	0	0	0	0	0	0	0
1500	621	16	234	219	115	32	4	1	0	0	0	0	0	0	0	0	0
1600	689	40	283	238	109	18	1	0	0	0	0	0	0	0	0	0	0
1700	665	10	168	256	163	67	1	0	0	0	0	0	0	0	0	0	0
1800	572	20	156	214	139	38	5	0	0	0	0	0	0	0	0	0	0
1900	575	7	131	200	172	57	8	0	0	0	0	0	0	0	0	0	0
2000	481	2	60	186	164	63	5	1	0	0	0	0	0	0	0	0	0
2100	385	0	33	127	142	75	7	1	0	0	0	0	0	0	0	0	0
2200	316	1	12	60	109	110	24	0	0	0	0	0	0	0	0	0	0
2300	255	0	9	55	65	94	27	2	3	0	0	0	0	0	0	0	0
Total	9335	278	2505	3000	2122	1075	302	47	5	1	0	0	0	0	0	0	0

100.00% 2.98% 26.83% 32.14% 22.73% 11.52% 3.24% 0.50% 0.05% 0.01% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00% 0.00%

Maximum = 41.2 mph, Minimum = 1.6 mph, Mean = 13.8 mph 85% Speed = 20.11 mph, 95% Speed = 24.10 mph, Median = 13.14 mph 10 mph Pace = 6 - 16, Number in Pace = 5762 (61.72%) Variance = 33.11, Standard Deviation = 5.75 mph

## **APPENDIX B**

Traffic Projection Model

### TRAFFIC PROJECTION MODEL

Weekday Morning Peak Hour Liberty Cannabis

Somerville, MA

			2022 Counted Volumes	Seasonal Adjustment	Balancing Volumes	2022 Existing Volumes	New Vehicle Trips PERCENT	New Vehicle Trips ENTER	New Vehicle Trips PERCENT	New Vehicle Trips EXIT	New Vehicle Trips TOTAL	2022 Build Volumes	Background Growth 5 yrs (at 0.25%	2027 Design Year Build
Intersection	Dir.	Turn					ENTER		EXIT				per year)	Volumes
Somerville Avenue/Bow Street/	EB	L	50	2	0	52	5%	0			0	52	1	53
Webster Avenue at Washington Street/		T	196	6		202					0	202	3	205
Somerville Avenue		R	43	1		44					0	44	1	45
	WB	L	44	1		45					0	45	1	46
		T	221	7		228	5%	0			0	228	3	231
		R	186	6	2	194	20%	0			0	194	2	196
	NB	L	8	0		8	5%	0			0	8	0	8
		T	138	4	1	143	10%	0			0	143	2	145
		R	27	1		28					0	28	0	28
	SB	L	201	6	4	211			25%	0	0	211	2	213
		T	284	9	6	299			15%	0	0	299	4	303
		R	50	2	1	53			15%	0	0	53	1	54
Bow Street at	WB	Т	365	11		376	35%	0			0	376	5	381
Warren Avenue		R	13	0		13					0	13	0	13
	NB	L	157	5		162			45%	0	0	162	2	164
		T	46	1		47					0	47	1	48
	SB	R	64	2		66					0	66	1	67
Somerville Avenue at	EB	L	203	6		209			45%	0	0	209	3	212
Warren Avenue		Т	547	16		563			55%	0	0	563	7	570
Somerville Avenue at	EB	Т	680	20		700	80%	1			1	701	9	710
Hawkins Street	NB	R	70	2		72	20%	0			0	72	1	73

### TRAFFIC PROJECTION MODEL

Weekday Afternoon Peak Hour Liberty Cannabis

Somerville, MA

			2022 Counted Volumes	Seasonal Adjustment	Balancing Volumes	2022 Existing Volumes	New Vehicle Trips PERCENT	New Vehicle Trips ENTER	New Vehicle Trips PERCENT	New Vehicle Trips EXIT	New Vehicle Trips TOTAL	2022 Build Volumes	Background Growth 5 yrs (at 0.25%	2027 Design Year Build
Intersection	Dir.	Turn					ENTER		EXIT				per year)	Volumes
Somerville Avenue/Bow Street/	EB	L	75	2	1	78	5%	0			0	78	1	79
Webster Avenue at Washington Street/		T	214	6		220					0	220	3	223
Somerville Avenue		R	34	1		35					0	35	0	35
	WB	L	25	1		26					0	26	0	26
		T	211	6		217	5%	0			0	217	3	220
		R	232	7	5	244	20%	2			2	246	3	249
	NB	L	22	1		23	5%	0			0	23	0	23
		T	284	9	6	299	10%	1			1	300	4	304
		R	13	0		13					0	13	0	13
	SB	L	160	5		165			25%	2	2	167	2	169
		T	195	6		201			15%	1	1	202	3	205
		R	21	1		22			15%	1	1	23	0	23
Bow Street at	WB	Т	582	17		599	35%	3			3	602	8	610
Warren Avenue		R	21	1		22					0	22	0	22
	NB	L	179	5		184			45%	4	4	188	2	190
		T	49	1		50					0	50	1	51
	SB	R	57	2		59					0	59	1	60
Somerville Avenue at	EB	L	228	6		234			45%	4	4	238	3	241
Warren Avenue		Т	374	12	2	388			55%	4	4	392	5	397
Somerville Avenue at	EB	Т	530	16	2	548	80%	6			6	554	7	561
Hawkins Street	NB	R	72	2	0	74	20%	2			2	76	1	77

### TRAFFIC PROJECTION MODEL

Saturday Midday Peak Hour Liberty Cannabis

Somerville, MA

			2022 Counted	Seasonal Adjustment	Balancing Volumes	2022 Existing	New Vehicle Trips	2022 Build	Background Growth 5 yrs	2027 Design Year				
			Volumes			Volumes	PERCENT	ENTER	PERCENT	EXIT	TOTAL	Volumes	(at 0.25%	Build
Intersection	Dir.	Turn					ENTER		EXIT				per year)	Volumes
Somerville Avenue/Bow Street/	EB	L	62	2		64	5%	0			0	64	1	65
Webster Avenue at Washington Street/		T	203	6		209					0	209	3	212
Somerville Avenue		R	15	0		15					0	15	0	15
	WB	L	27	1		28					0	28	0	28
		T	158	5		163	5%	0			0	163	2	165
		R	263	8		271	20%	2			2	273	3	276
	NB	L	24	1		25	5%	0			0	25	0	25
		Т	193	6		199	10%	1			1	200	2	202
		R	21	1		22					0	22	0	22
	SB	L	237	7		244			25%	2	2	246	3	249
		T	196	6		202			15%	1	1	203	3	206
		R	59	2		61			15%	1	1	62	1	63
Bow Street at	WB	Т	483	14	5	502	35%	3			3	505	6	511
Warren Avenue		R	31	1	0	32					0	32	0	32
	NB	L	191	6		197			45%	3	3	200	2	202
		T	41	1		42					0	42	1	43
	SB	R	97	3		100					0	100	1	101
Somerville Avenue at	EB	L	232	7		239			45%	3	3	242	3	245
Warren Avenue		Т	480	14	13	507			55%	4	4	511	7	518
Somerville Avenue at	EB	Т	670	20	12	702	80%	7			7	709	9	718
Hawkins Street	NB	R	42	1	1	44	20%	2			2	46	1	47

**APPENDIX C** 

Crash Data

# **CRASH ANALYSIS**

# Liberty Cannabis Somerville, MA

Jonner Ville, IVIA	Somerville Ave/Bow St/			
	Webster Ave at			
	Washington St/ Somerville Ave	Somerville Ave/Bow St at Warren Ave	Somerville Ave at Hawkins St	Somerville Ave at Site Driveway
Year	Some ville Ave	vvarren Ave	Ji	Driveway
2015	0	0	0	0
2016	0	0	0	0
2017	4	3	0	0
2018	4	6	0	0
2019	0	5	2	0
Туре				
Angle	1	4	1	0
Rear-end	0	5	0	0
Sideswipe	3	2	1	0
Head-on	1	0	0	0
Pedestrian	2	0	0	0
Bicycle	0	0	0	0
Single Vehicle	1	2	0	0
Unknown	0	1	0	0
Severity				
Property Damage	5	9	2	0
Personal Injury	1	2	0	0
Fatality	0	0	0	0
Unknown	2	3	0	0
Weather				
Clear	6	8	2	0
Cloudy	0	2	0	0
Rain	2	3	0	0
Snow	0	0	0	0
Sleet	0	1	0	0
Road Surface				
Dry	6	10	1	0
Wet	2	4	0	0
Ice	0	0	0	0
Snow	0	0	0	0
Slush	0	0	1	0
Time				
7:00 AM to 9:00 AM	1	1	2	0
9:00 AM to 4:00 PM	4	5	0	0
4:00 PM to 6:00 PM	1	2	0	0
6:00 PM to 7:00 AM	2	6	0	0
Total	8	14	2	0
Crash Rate	0.22	0.48	0.11	0.00
State Average	0.78	0.78	0.57	0.57
District 4 Average	0.73	0.73	0.57	0.57

Source: MassDOT

#### **APPENDIX D**

Journey-to-Work Data

# **JOURNEY-TO-WORK DATA**

# Liberty Cannabis Somerville, MA

	Location of Residence of	Number of			
#	Somerville Workers	Workers	Percent	Assigned Route(s)	
1	Somerville	7,391	48.8%	Bow St/Somerville Ave to West	Washington St to West
2	Boston	2,236	14.8%	Bow St/Somerville Ave to West	Somerville Ave to East
3	Medford	1,358	9.0%	Bow St/Somerville Ave to West	Somerville Ave to East
4	Cambridge	988	6.5%	Washington St to West	Webster Ave to South
5	Malden	857	5.7%	Bow St/Somerville Ave to West	Somerville Ave to East
6	Everett	827	5.5%	Bow St/Somerville Ave to West	Somerville Ave to East
7	Arlington	427	2.8%	Bow St/Somerville Ave to West	
8	Waltham	391	2.6%	Washington St to West	Webster Ave to South
9	Revere	343	2.3%	Bow St/Somerville Ave to West	Somerville Ave to East
10	Billerica	318	2.1%	Bow St/Somerville Ave to West	Somerville Ave to East
	Total	15,136	100.0%		

Trip Distribution	% Of Total Workers	Trips Assigned
Washington St	14.3%	15%
Webster Ave	15.6%	15%
Somerville Ave to East	26.8%	25%
Bow St/Somerville to West	43.2%	45%
Total	100.0%	100%

#### **APPENDIX E**

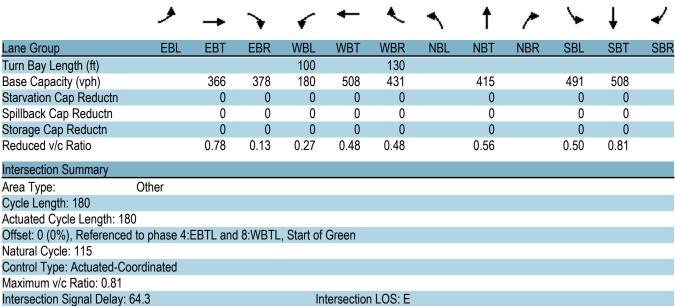
2022 Existing Capacity/Level-of-Service Analysis

# Liberty Cannabis 1: Webster Ave & Washington St & Somerville Ave

	۶	<b>→</b>	•	•	+	•	•	<b>†</b>	~	<b>/</b>	<b>+</b>	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ર્ન	7	ሻ	<b>†</b>	7		4		ሻ	f.	
Traffic Volume (vph)	52	202	44	45	228	194	8	143	28	211	299	53
Future Volume (vph)	52	202	44	45	228	194	8	143	28	211	299	53
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	100		130	0		0	0		0
Storage Lanes	0		1	1		1	0		0	1		0
Taper Length (ft)	25			25			25			25		
Satd. Flow (prot)	0	1755	1482	1583	1727	1495	0	1765	0	1703	1760	0
Flt Permitted		0.708		0.388				0.998		0.950		
Satd. Flow (perm)	0	1246	1284	612	1727	1495	0	1765	0	1703	1760	0
Right Turn on Red			No			No			Yes			No
Satd. Flow (RTOR)								5				
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		580			678			611			159	
Travel Time (s)		13.2			15.4			13.9			3.6	
Confl. Peds. (#/hr)	20		36	36					28			45
Confl. Bikes (#/hr)									2			28
Peak Hour Factor	0.89	0.89	0.89	0.93	0.93	0.93	0.77	0.77	0.77	0.86	0.86	0.86
Heavy Vehicles (%)	4%	8%	9%	14%	10%	8%	0%	4%	0%	6%	2%	4%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	285	49	48	245	209	0	232	0	245	410	0
Turn Type	Perm	NA	Perm	Perm	NA	Over	Split	NA		Split	NA	
Protected Phases		4			8	2	6	6		2	2	
Permitted Phases	4		4	8								
Detector Phase	4	4	4	8	8	2	6	6		2	2	
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0		10.0	10.0	
Minimum Split (s)	34.0	34.0	34.0	34.0	34.0	33.0	31.0	31.0		33.0	33.0	
Total Split (s)	59.0	59.0	59.0	59.0	59.0	58.0	48.0	48.0		58.0	58.0	
Total Split (%)	32.8%	32.8%	32.8%	32.8%	32.8%	32.2%	26.7%	26.7%		32.2%	32.2%	
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0		3.0	3.0	
All-Red Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Lost Time Adjust (s)		0.0	0.0	0.0	0.0	0.0		0.0		0.0	0.0	
Total Lost Time (s)		6.0	6.0	6.0	6.0	6.0		6.0		6.0	6.0	
Lead/Lag	Lag	Lag	Lag	Lag	Lag	Lag	Lag	Lag		Lag	Lag	
Lead-Lag Optimize?												
Recall Mode	C-Max		C-Max	C-Max	C-Max	Max	Max	Max		Max	Max	
Act Effct Green (s)		53.0	53.0	53.0	53.0	52.0		42.0		52.0	52.0	
Actuated g/C Ratio		0.29	0.29	0.29	0.29	0.29		0.23		0.29	0.29	
v/c Ratio		0.78	0.13	0.27	0.48	0.48		0.56		0.50	0.81	
Control Delay		74.0	47.9	53.7	56.1	57.5		65.5		57.4	72.8	
Queue Delay		0.0	0.0	0.0	0.0	0.0		0.0		0.0	0.0	
Total Delay		74.0	47.9	53.7	56.1	57.5		65.5		57.4	72.8	
LOS		Е	D	D	Е	E		Е		Е	E	
Approach Delay		70.1			56.4			65.5			67.1	
Approach LOS		Е			Е			Е			Е	
Queue Length 50th (ft)		310	43	44	240	206		239		243	453	
Queue Length 95th (ft)		432	80	88	335	297		280		321	561	
Internal Link Dist (ft)		500			598			531			79	

03/10/2022 McMahon Associates Synchro 10 Report Page 1

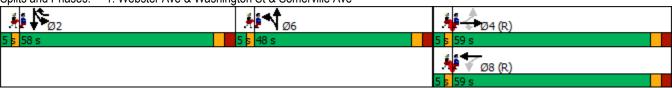
Lane Group	Ø1	Ø3	Ø5	Ø7
Lane Configurations				
Traffic Volume (vph)				
Future Volume (vph)				
Ideal Flow (vphpl)				
Storage Length (ft)				
Storage Lanes				
Taper Length (ft)				
Satd. Flow (prot)				
Flt Permitted				
Satd. Flow (perm)				
Right Turn on Red				
Satd. Flow (RTOR)				
Link Speed (mph)				
Link Distance (ft)				
Travel Time (s)				
Confl. Peds. (#/hr)				
Confl. Bikes (#/hr)				
Peak Hour Factor				
Heavy Vehicles (%)				
Shared Lane Traffic (%)				
Lane Group Flow (vph)				
Turn Type				
Protected Phases	1	3	5	7
Permitted Phases				
Detector Phase				
Switch Phase				
Minimum Initial (s)	3.0	3.0	3.0	3.0
Minimum Split (s)	5.0	5.0	5.0	5.0
,	5.0	5.0	5.0	5.0
Total Split (s)				
Total Split (%)	3%	3%	3%	3%
Yellow Time (s)	2.0	2.0	2.0	2.0
All-Red Time (s)	0.0	0.0	0.0	0.0
Lost Time Adjust (s)				
Total Lost Time (s)				
Lead/Lag	Lead	Lead	Lead	Lead
Lead-Lag Optimize?				
Recall Mode	Max	Max	Max	Max
Act Effct Green (s)				
Actuated g/C Ratio				
v/c Ratio				
Control Delay				
Queue Delay				
Total Delay				
LOS				
Approach Delay				
Approach LOS				
Queue Length 50th (ft)				
Queue Length 95th (ft)				
Internal Link Dist (ft)				
00/40/0000				



Intersection LOS: E Intersection Capacity Utilization 98.7% ICU Level of Service F

Analysis Period (min) 15

Splits and Phases: 1: Webster Ave & Washington St & Somerville Ave



Lane Group	Ø1	Ø3	Ø5	Ø7	
Turn Bay Length (ft)					
Base Capacity (vph)					
Starvation Cap Reductn					
Spillback Cap Reductn					
Storage Cap Reductn					
Reduced v/c Ratio					
Intersection Summary					

Synchro 10 Report Page 4 03/10/2022 McMahon Associates

User Entered Value

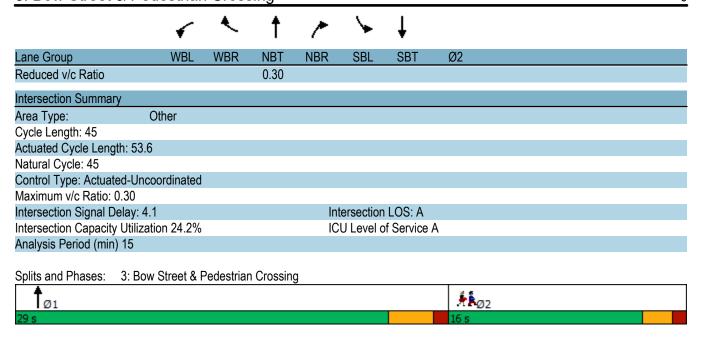
Z. Wallell Ave & D	ow oneer								ZOZZ EXISTING			
	•	<b>→</b>	•	•	<b>←</b>	•	•	<b>†</b>	<i>&gt;</i>	<b>/</b>	ļ	1
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations					ĵ.			ર્ન				77
Traffic Volume (veh/h)	0	0	0	0	376	13	162	47	0	0	0	66
Future Volume (Veh/h)	0	0	0	0	376	13	162	47	0	0	0	66
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.76	0.76	0.76
Hourly flow rate (vph)	0	0	0	0	409	14	176	51	0	0	0	87
Pedestrians											54	
Lane Width (ft)											12.0	
Walking Speed (ft/s)											3.5	
Percent Blockage											5	
Right turn flare (veh)												
Median type		None			None							
Median storage veh)		140110			110110							
Upstream signal (ft)					50							
pX, platoon unblocked	0.92				30		0.92	0.92		0.92	0.92	0.92
vC, conflicting volume	477			0			503	477	0	496	470	470
vC1, stage 1 conf vol	711			U			303	711	U	730	470	470
vC2, stage 2 conf vol												
vCu, unblocked vol	389			0			417	389	0	409	381	381
tC, single (s)	4.1			4.1			*5.5	*5.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)	4.1			4.1			5.5	5.5	0.2	7.1	0.5	0.2
	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
tF (s)	100			100			65	90	100	100	100	3.3 85
p0 queue free %				1623				534	1085		482	585
cM capacity (veh/h)	1021			1023			497	534	1005	430	462	200
Direction, Lane #	WB 1	NB 1	SB 1									
Volume Total	423	227	87									
Volume Left	0	176	0									
Volume Right	14	0	87									
cSH	1700	505	585									
Volume to Capacity	0.25	0.45	0.15									
Queue Length 95th (ft)	0	57	13									
Control Delay (s)	0.0	17.8	12.2									
Lane LOS		С	В									
Approach Delay (s)	0.0	17.8	12.2									
Approach LOS		С	В									
Intersection Summary												
Average Delay			6.9									
Intersection Capacity Utiliza	ation		46.2%	IC	CU Level o	of Service			Α			
Analysis Period (min)			15									

# Liberty Cannabis 3: Bow Street & Pedestrian Crossing

	•	•	<b>†</b>	<b>/</b>	<b>&gt;</b>	ļ		
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT	Ø2	
Lane Configurations			<b></b>					
Traffic Volume (vph)	0	0	389	0	0	0		
Future Volume (vph)	0	0	389	0	0	0		
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900		
Satd. Flow (prot)	0	0	1792	0	0	0		
Flt Permitted			1702					
Satd. Flow (perm)	0	0	1792	0	0	0		
Right Turn on Red		No	1102	No				
Satd. Flow (RTOR)		110		110				
Link Speed (mph)	30		30			30		
Link Distance (ft)	184		50			50		
Travel Time (s)	4.2		1.1			1.1		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92		
Heavy Vehicles (%)	2%	2%	6%	2%	2%	2%		
Shared Lane Traffic (%)	2 /0	2 /0	0 70	2 /0	2 /0	2 /0		
Lane Group Flow (vph)	0	0	423	0	0	0		
Turn Type	U	U	NA	0	U	U		
Protected Phases			1				2	
Permitted Phases							2	
Detector Phase			1					
Switch Phase								
Minimum Initial (s)			25.0				10.0	
Minimum Split (s)			29.0				16.0	
Total Split (s)			29.0				16.0	
Total Split (%)			64.4%				36%	
Yellow Time (s)			3.0				2.0	
All-Red Time (s)			1.0				1.0	
Lost Time Adjust (s)			0.0				1.0	
Total Lost Time (s)			4.0					
Lead/Lag			Lead				Lag	
Lead-Lag Optimize?			Leau				Lag	
Recall Mode			Min				None	
Act Effct Green (s)			42.9				NOTIC	
Actuated g/C Ratio			0.80					
v/c Ratio			0.30					
Control Delay			4.1					
Queue Delay			0.0					
Total Delay			4.1					
LOS			4.1 A					
Approach Delay			4.1					
Approach LOS			4.1 A					
Queue Length 50th (ft)			54					
Queue Length 95th (ft)			92					
Internal Link Dist (ft)	104		1			1		
Turn Bay Length (ft)	104		I					
Base Capacity (vph)			1433					
Starvation Cap Reductn			1433					
Spillback Cap Reductn			0					
Storage Cap Reductin			0					
- Corago Oap Neudolli			U					

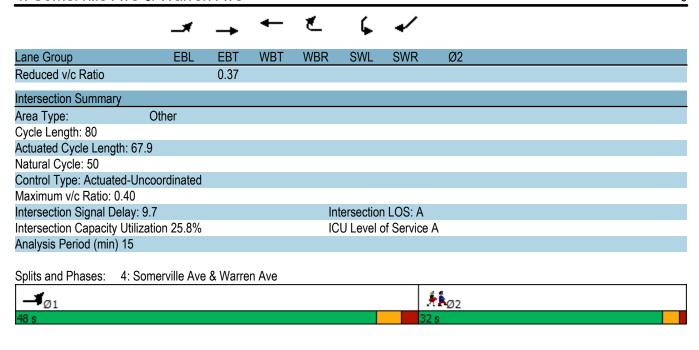
Synchro 10 Report 03/10/2022 Page 5 McMahon Associates

# 3: Bow Street & Pedestrian Crossing



	_≉	-	<b>←</b>	€_	Ĺ	✓		
Lane Group	EBL	EBT	WBT	WBR	SWL	SWR	Ø2	
Lane Configurations		414			0	• • • • • • • • • • • • • • • • • • • •	~-	
Traffic Volume (vph)	209	563	0	0	0	0		
Future Volume (vph)	209	563	0	0	0	0		
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900		
Satd. Flow (prot)	0	3453	0	0	0	0		
Flt Permitted	0	0.987	U	- U		- U		
Satd. Flow (perm)	0	3453	0	0	0	0		
Right Turn on Red	Yes	0400	U	Yes	U	Yes		
Satd. Flow (RTOR)	163	103		163		163		
Link Speed (mph)		30	30		30			
Link Distance (ft)		240	98		71			
Travel Time (s)		5.5	2.2		1.6			
	0.00			0.02		0.92		
Peak Hour Factor	0.90	0.90	0.92	0.92	0.92			
Heavy Vehicles (%)	1%	4%	2%	2%	2%	2%		
Shared Lane Traffic (%)		050	^	^	^	^		
Lane Group Flow (vph)	0	858	0	0	0	0		
Turn Type	Split	NA					•	
Protected Phases	1	1					2	
Permitted Phases								
Detector Phase	1	1						
Switch Phase								
Minimum Initial (s)	10.0	10.0					5.0	
Minimum Split (s)	15.0	15.0					32.0	
Total Split (s)	48.0	48.0					32.0	
Total Split (%)	60.0%	60.0%					40%	
Yellow Time (s)	3.0	3.0					2.0	
All-Red Time (s)	2.0	2.0					1.0	
Lost Time Adjust (s)		0.0						
Total Lost Time (s)		5.0						
Lead/Lag	Lead	Lead					Lag	
Lead-Lag Optimize?								
Recall Mode	Min	Min					None	
Act Effct Green (s)		41.8						
Actuated g/C Ratio		0.62						
v/c Ratio		0.40						
Control Delay		9.7						
Queue Delay		0.0						
Total Delay		9.7						
LOS		A						
Approach Delay		9.7						
Approach LOS		A						
Queue Length 50th (ft)		110						
Queue Length 95th (ft)		152						
Internal Link Dist (ft)		160	18		1			
Turn Bay Length (ft)		100	10					
Base Capacity (vph)		2303						
Starvation Cap Reductn								
		0						
Spillback Cap Reductn								
Storage Cap Reductn		0						

Synchro 10 Report Page 7 03/10/2022 McMahon Associates



	-	•	•	•	•	~	
Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	<b>†</b> †					7	
Traffic Volume (veh/h)	700	0	0	0	0	72	
Future Volume (Veh/h)	700	0	0	0	0	72	
Sign Control	Free			Free	Stop		
Grade	0%			0%	0%		
Peak Hour Factor	0.93	0.93	0.92	0.92	0.73	0.73	
Hourly flow rate (vph)	753	0	0	0	0	99	
Pedestrians							
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type	None			None			
Median storage veh)							
Upstream signal (ft)				240			
pX, platoon unblocked							
vC, conflicting volume			753		753	376	
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol			753		753	376	
tC, single (s)			4.1		6.8	7.0	
tC, 2 stage (s)							
tF (s)			2.2		3.5	3.3	
p0 queue free %			100		100	84	
cM capacity (veh/h)			853		346	618	
Direction, Lane #	EB 1	EB 2	NB 1				
Volume Total	376	376	99				
Volume Left	0	0	0				
Volume Right	0	0	99				
cSH	1700	1700	618				
Volume to Capacity	0.22	0.22	0.16				
Queue Length 95th (ft)	0	0	14				
Control Delay (s)	0.0	0.0	11.9				
Lane LOS	0.0	3.0	В				
Approach Delay (s)	0.0		11.9				
Approach LOS	0.0		В				
Intersection Summary							
Average Delay			1.4				
Intersection Capacity Utiliza	ation		30.5%	IC	U Level c	of Service	
Analysis Period (min)	audii		15	10	O LOVEI C	, OCIVICE	
Analysis i enou (min)			10				

	♪	<b>→</b>	•	•	<b>←</b>	•	•	<b>†</b>	<i>&gt;</i>	<b>/</b>	<b>↓</b>	-√
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ર્ન	7	ሻ	<b>†</b>	7		4		ች	f.	
Traffic Volume (vph)	78	220	35	26	217	244	23	299	13	165	201	22
Future Volume (vph)	78	220	35	26	217	244	23	299	13	165	201	22
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	100		130	0		0	0		0
Storage Lanes	0		1	1		1	0		0	1		0
Taper Length (ft)	25			25			25			25		
Satd. Flow (prot)	0	1803	1615	1612	1863	1568	0	1849	0	1770	1774	0
Flt Permitted		0.646		0.346				0.997		0.950		
Satd. Flow (perm)	0	1157	1405	560	1863	1568	0	1849	0	1770	1774	0
Right Turn on Red			No			No			Yes			No
Satd. Flow (RTOR)								1				
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		580			678			611			159	
Travel Time (s)		13.2			15.4			13.9			3.6	
Confl. Peds. (#/hr)	41		35	35					60			74
Confl. Bikes (#/hr)									26			8
Peak Hour Factor	0.95	0.95	0.95	0.94	0.94	0.94	0.94	0.94	0.94	0.96	0.96	0.96
Heavy Vehicles (%)	4%	4%	0%	12%	2%	3%	0%	1%	0%	2%	3%	0%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	314	37	28	231	260	0	356	0	172	232	0
Turn Type	Perm	NA	Perm	Perm	NA	Over	Split	NA		Split	NA	
Protected Phases		4			8	2	6	6		2	2	
Permitted Phases	4		4	8								
Detector Phase	4	4	4	8	8	2	6	6		2	2	
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0		10.0	10.0	
Minimum Split (s)	34.0	34.0	34.0	34.0	34.0	33.0	31.0	31.0		33.0	33.0	
Total Split (s)	59.0	59.0	59.0	59.0	59.0	58.0	48.0	48.0		58.0	58.0	
Total Split (%)	32.8%	32.8%	32.8%	32.8%	32.8%	32.2%	26.7%	26.7%		32.2%	32.2%	
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0		3.0	3.0	
All-Red Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Lost Time Adjust (s)		0.0	0.0	0.0	0.0	0.0		0.0		0.0	0.0	
Total Lost Time (s)		6.0	6.0	6.0	6.0	6.0		6.0		6.0	6.0	
Lead/Lag	Lag	Lag	Lag	Lag	Lag	Lag	Lag	Lag		Lag	Lag	
Lead-Lag Optimize?	J	J	J	Ŭ	Ŭ			J				
Recall Mode	C-Max	C-Max	C-Max	C-Max	C-Max	Max	Max	Max		Max	Max	
Act Effct Green (s)		53.0	53.0	53.0	53.0	52.0		42.0		52.0	52.0	
Actuated g/C Ratio		0.29	0.29	0.29	0.29	0.29		0.23		0.29	0.29	
v/c Ratio		0.92	0.09	0.17	0.42	0.58		0.82		0.34	0.45	
Control Delay		93.6	47.0	50.7	54.1	60.6		81.7		52.7	55.8	
Queue Delay		0.0	0.0	0.0	0.0	0.0		0.0		0.0	0.0	
Total Delay		93.6	47.0	50.7	54.1	60.6		81.7		52.7	55.8	
LOS		F	D	D	D	Е		F		D	Е	
Approach Delay		88.7			57.1			81.7			54.5	
Approach LOS		F			E			F			D	
Queue Length 50th (ft)		362	32	25	222	264		403		161	226	
Queue Length 95th (ft)		#562	66	57	311	369		#562		236	318	
Internal Link Dist (ft)		500			598			531			79	

03/10/2022 McMahon Associates

Lane Group	Ø1	Ø3	Ø5	Ø7
Lane Configurations				
Traffic Volume (vph)				
Future Volume (vph)				
Ideal Flow (vphpl)				
\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \				
Storage Length (ft)				
Storage Lanes				
Taper Length (ft)				
Satd. Flow (prot)				
Flt Permitted				
Satd. Flow (perm)				
Right Turn on Red				
Satd. Flow (RTOR)				
Link Speed (mph)				
Link Distance (ft)				
Travel Time (s)				
Confl. Peds. (#/hr)				
Confl. Bikes (#/hr)				
Peak Hour Factor				
Heavy Vehicles (%)				
Shared Lane Traffic (%)				
Lane Group Flow (vph)				
Turn Type				
Protected Phases	1	3	5	7
Permitted Phases	<u>'</u>			•
Detector Phase				
Switch Phase				
Minimum Initial (s)	3.0	3.0	3.0	3.0
Minimum Split (s)	5.0	5.0	5.0	5.0
	5.0		5.0	5.0
Total Split (s)		5.0		
Total Split (%)	3%	3%	3%	3%
Yellow Time (s)	2.0	2.0	2.0	2.0
All-Red Time (s)	0.0	0.0	0.0	0.0
Lost Time Adjust (s)				
Total Lost Time (s)				
Lead/Lag	Lead	Lead	Lead	Lead
Lead-Lag Optimize?				
Recall Mode	Max	Max	Max	Max
Act Effct Green (s)				
Actuated g/C Ratio				
v/c Ratio				
Control Delay				
Queue Delay				
Total Delay				
LOS				
Approach Delay				
Annroach I OS				
Approach LOS				
Queue Length 50th (ft)				

#### 1: Webster Ave & Washington St & Somerville Ave

	۶	-	$\rightarrow$	•	<b>←</b>	•	<b>1</b>	<b>†</b>	/	-	ţ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Bay Length (ft)				100		130						
Base Capacity (vph)		340	413	164	548	452		432		511	512	
Starvation Cap Reductn		0	0	0	0	0		0		0	0	
Spillback Cap Reductn		0	0	0	0	0		0		0	0	
Storage Cap Reductn		0	0	0	0	0		0		0	0	
Reduced v/c Ratio		0.92	0.09	0.17	0.42	0.58		0.82		0.34	0.45	

#### Intersection Summary

Area Type: Other

Cycle Length: 180 Actuated Cycle Length: 180

Offset: 0 (0%), Referenced to phase 4:EBTL and 8:WBTL, Start of Green

Natural Cycle: 115

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.92
Intersection Signal Delay: 68

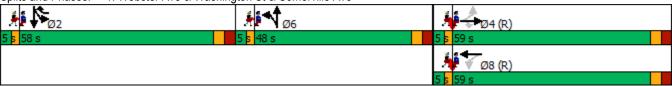
Intersection Signal Delay: 68.6 Intersection Capacity Utilization 98.1% ICU Level of Service F

Analysis Period (min) 15

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 1: Webster Ave & Washington St & Somerville Ave



Lane Group	Ø1	Ø3	Ø5	Ø7	
Turn Bay Length (ft)					
Base Capacity (vph)					
Starvation Cap Reductn					
Spillback Cap Reductn					
Storage Cap Reductn					
Reduced v/c Ratio					
Intersection Summary					

	٠	<b>→</b>	•	•	<b>←</b>	•	4	<b>†</b>	/	<b>&gt;</b>	ļ	1
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations					f)			4				7
Traffic Volume (veh/h)	0	0	0	0	599	22	184	50	0	0	0	59
Future Volume (Veh/h)	0	0	0	0	599	22	184	50	0	0	0	59
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.74	0.74	0.74	0.75	0.75	0.75
Hourly flow rate (vph)	0	0	0	0	651	24	249	68	0	0	0	79
Pedestrians											77	
Lane Width (ft)											12.0	
Walking Speed (ft/s)											3.5	
Percent Blockage											7	
Right turn flare (veh)											-	
Median type		None			None							
Median storage veh)		110110			110110							
Upstream signal (ft)					50							
pX, platoon unblocked	0.84						0.84	0.84		0.84	0.84	0.84
vC, conflicting volume	752			0			742	752	0	774	740	740
vC1, stage 1 conf vol	102			U			172	102	U	117	7 40	140
vC2, stage 2 conf vol												
vCu, unblocked vol	610			0			598	610	0	636	596	596
tC, single (s)	4.1			4.1			*5.5	*5.5	6.2	7.1	6.5	6.3
tC, 2 stage (s)	4.1			4.1			5.5	5.5	0.2	7.1	0.5	0.5
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.4
p0 queue free %	100			100			27	82	100	100	100	
•				1623								79 385
cM capacity (veh/h)	755			1023			343	380	1085	248	325	300
Direction, Lane #	WB 1	NB 1	SB 1									
Volume Total	675	317	79									
Volume Left	0	249	0									
Volume Right	24	0	79									
cSH	1700	350	385									
Volume to Capacity	0.40	0.90	0.21									
Queue Length 95th (ft)	0	225	19									
Control Delay (s)	0.0	62.0	16.7									
Lane LOS		F	С									
Approach Delay (s)	0.0	62.0	16.7									
Approach LOS		F	С									
Intersection Summary												
Average Delay			19.6									
Intersection Capacity Utiliza	ation		59.5%	IC	U Level o	of Service			В			
Analysis Period (min)			15									

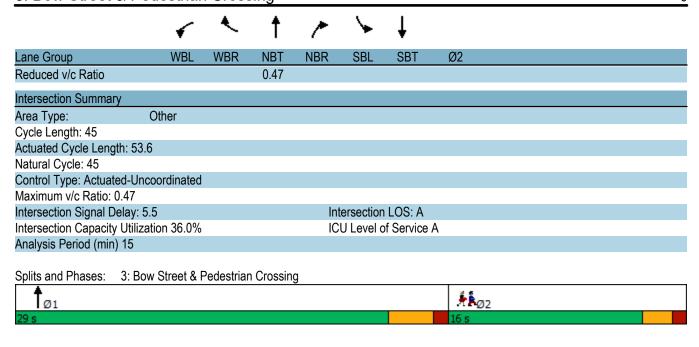
\* User Entered Value

# Liberty Cannabis 3: Bow Street & Pedestrian Crossing

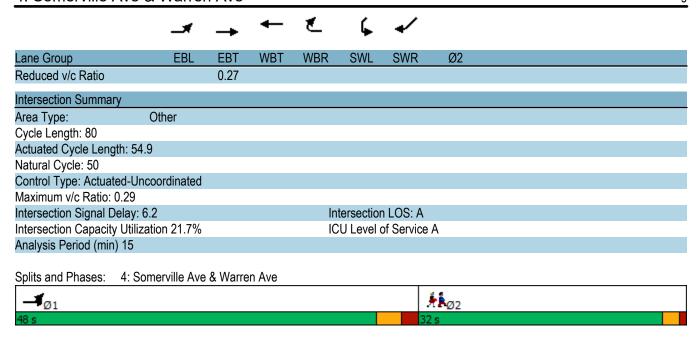
	•	•	<b>†</b>	<i>&gt;</i>	<b>&gt;</b>	ļ		
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT	Ø2	
Lane Configurations			<b>†</b>					
Traffic Volume (vph)	0	0	621	0	0	0		
Future Volume (vph)	0	0	621	0	0	0		
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900		
Satd. Flow (prot)	0	0	1792	0	0	0		
Flt Permitted								
Satd. Flow (perm)	0	0	1792	0	0	0		
Right Turn on Red		No		No		•		
Satd. Flow (RTOR)								
Link Speed (mph)	30		30			30		
Link Distance (ft)	184		50			50		
Travel Time (s)	4.2		1.1			1.1		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92		
Heavy Vehicles (%)	2%	2%	6%	2%	2%	2%		
Shared Lane Traffic (%)	Z /0	2 /0	0 /0	<b>2</b> /0	<b>2</b> /0	2 /0		
Lane Group Flow (vph)	0	0	675	0	0	0		
Turn Type	U	U	NA	U	U	U		
Protected Phases			1				2	
Permitted Phases							2	
Detector Phase			1					
Switch Phase								
Minimum Initial (s)			25.0				10.0	
Minimum Split (s)			29.0				16.0	
Total Split (s)			29.0				16.0	
Total Split (%)			64.4%				36%	
Yellow Time (s)			3.0				2.0	
All-Red Time (s)			1.0				1.0	
Lost Time Adjust (s)			0.0				1.0	
Total Lost Time (s)			4.0					
Lead/Lag			Lead				Lag	
Lead-Lag Optimize?			Load				Lug	
Recall Mode			Min				None	
Act Effct Green (s)			42.9				10.10	
Actuated g/C Ratio			0.80					
v/c Ratio			0.47					
Control Delay			5.5					
Queue Delay			0.0					
Total Delay			5.5					
LOS			A					
Approach Delay			5.5					
Approach LOS			A					
Queue Length 50th (ft)			105					
Queue Length 95th (ft)			178					
Internal Link Dist (ft)	104		1			1		
Turn Bay Length (ft)								
Base Capacity (vph)			1433					
Starvation Cap Reductn			0					
Spillback Cap Reductn			0					
Storage Cap Reductn			0					
			-					

Synchro 10 Report Page 5 03/10/2022 McMahon Associates

# 3: Bow Street & Pedestrian Crossing



	_#	-	<b>←</b>	€.	Ĺ	✓		
Lane Group	EBL	EBT	WBT	WBR	SWL	SWR	Ø2	
Lane Configurations		414				-		
Traffic Volume (vph)	234	388	0	0	0	0		
Future Volume (vph)	234	388	0	0	0	0		
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900		
Satd. Flow (prot)	0	3501	0	0	0	0		
Flt Permitted		0.982						
Satd. Flow (perm)	0	3501	0	0	0	0		
Right Turn on Red	Yes	0001		Yes		Yes		
Satd. Flow (RTOR)	100	243		100		100		
Link Speed (mph)		30	30		30			
Link Distance (ft)		240	98		71			
Travel Time (s)		5.5	2.2		1.6			
Peak Hour Factor	0.86	0.86	0.92	0.92	0.92	0.92		
Heavy Vehicles (%)	0%	2%	2%	2%	2%	2%		
Shared Lane Traffic (%)	0 70	270	270	270	270	270		
Lane Group Flow (vph)	0	723	0	0	0	0		
Turn Type	Split	NA	U	U	U	U		
Protected Phases	1	1					2	
Permitted Phases	l I	1					L	
Detector Phase	1	1						
Switch Phase								
Minimum Initial (s)	10.0	10.0					5.0	
Minimum Split (s)	15.0	15.0					32.0	
Total Split (s)	48.0	48.0					32.0	
Total Split (%)	60.0%	60.0%					40%	
Yellow Time (s)	3.0	3.0					2.0	
All-Red Time (s)	2.0	2.0					1.0	
Lost Time Adjust (s)	2.0	0.0					1.0	
Total Lost Time (s)		5.0						
Lead/Lag	Lead	Lead					Lag	
Lead-Lag Optimize?	Leau	Leau					Lay	
Recall Mode	Min	Min					None	
Act Effct Green (s)	IVIIII	37.8					None	
Actuated g/C Ratio		0.69						
•		0.09						
v/c Ratio		6.2						
Control Delay								
Queue Delay		0.0						
Total Delay		6.2						
LOS		A						
Approach Delay		6.2						
Approach LOS		A						
Queue Length 50th (ft)		63						
Queue Length 95th (ft)		90	40		4			
Internal Link Dist (ft)		160	18		1			
Turn Bay Length (ft)		0700						
Base Capacity (vph)		2722						
Starvation Cap Reductn		0						
Spillback Cap Reductn		0						
Storage Cap Reductn		0						



	<b>→</b>	•	•	<b>—</b>	1		
Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	<b>^</b>					7	
Traffic Volume (veh/h)	548	0	0	0	0	74	
Future Volume (Veh/h)	548	0	0	0	0	74	
Sign Control	Free			Free	Stop		
Grade	0%			0%	0%		
Peak Hour Factor	0.84	0.84	0.92	0.92	0.82	0.82	
Hourly flow rate (vph)	652	0	0	0	0	90	
Pedestrians							
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type	None			None			
Median storage veh)							
Upstream signal (ft)				240			
pX, platoon unblocked				2.0			
vC, conflicting volume			652		652	326	
vC1, stage 1 conf vol			002		002	020	
vC2, stage 2 conf vol							
vCu, unblocked vol			652		652	326	
tC, single (s)			4.1		6.8	6.9	
tC, 2 stage (s)					0.0	0.0	
tF (s)			2.2		3.5	3.3	
p0 queue free %			100		100	87	
cM capacity (veh/h)			930		401	673	
Direction, Lane #	EB 1	EB 2	NB 1				
Volume Total	326	326	90				
Volume Left	0	0	0				
	0	0	90				
Volume Right cSH	1700	1700	673				
	0.19						
Volume to Capacity		0.19	0.13				
Queue Length 95th (ft)	0	0	12				
Control Delay (s)	0.0	0.0	11.2				
Lane LOS	0.0		B				
Approach LOS	0.0		11.2				
Approach LOS			В				
Intersection Summary							
Average Delay			1.4				
Intersection Capacity Utilization	ation		26.4%	IC	U Level c	of Service	!
Analysis Period (min)			15				

	۶	<b>→</b>	•	•	<b>←</b>	•	•	<b>†</b>	~	<b>/</b>	<del> </del>	✓
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4	7	*	<b></b>	7		4		ች	f)	
Traffic Volume (vph)	64	209	15	28	163	271	25	199	22	244	202	61
Future Volume (vph)	64	209	15	28	163	271	25	199	22	244	202	61
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	100		130	0		0	0	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	0
Storage Lanes	0		1	1		1	0		0	1		0
Taper Length (ft)	25			25			25			25		
Satd. Flow (prot)	0	1821	1615	1736	1827	1568	0	1804	0	1752	1596	0
Flt Permitted		0.771		0.336				0.995		0.950		
Satd. Flow (perm)	0	1349	1339	577	1827	1568	0	1804	0	1752	1596	0
Right Turn on Red			No			No			Yes			No
Satd. Flow (RTOR)								3				
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		580			678			611			159	
Travel Time (s)		13.2			15.4			13.9			3.6	
Confl. Peds. (#/hr)	100		48	48					83			195
Confl. Bikes (#/hr)									16			31
Peak Hour Factor	0.85	0.85	0.85	0.94	0.94	0.94	0.92	0.92	0.92	0.91	0.91	0.91
Heavy Vehicles (%)	0%	4%	0%	4%	4%	3%	0%	0%	5%	3%	0%	0%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	321	18	30	173	288	0	267	0	268	289	0
Turn Type	Perm	NA	Perm	Perm	NA	Over	Split	NA		Split	NA	
Protected Phases		4			8	2	6	6		2	2	
Permitted Phases	4		4	8								
Detector Phase	4	4	4	8	8	2	6	6		2	2	
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0		10.0	10.0	
Minimum Split (s)	34.0	34.0	34.0	34.0	34.0	33.0	31.0	31.0		33.0	33.0	
Total Split (s)	59.0	59.0	59.0	59.0	59.0	58.0	48.0	48.0		58.0	58.0	
Total Split (%)	32.8%	32.8%	32.8%	32.8%	32.8%	32.2%	26.7%	26.7%		32.2%	32.2%	
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0		3.0	3.0	
All-Red Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Lost Time Adjust (s)		0.0	0.0	0.0	0.0	0.0		0.0		0.0	0.0	
Total Lost Time (s)		6.0	6.0	6.0	6.0	6.0		6.0		6.0	6.0	
Lead/Lag	Lag	Lag	Lag	Lag	Lag	Lag	Lag	Lag		Lag	Lag	
Lead-Lag Optimize?												
Recall Mode	C-Max	C-Max	C-Max	C-Max	C-Max	Max	Max	Max		Max	Max	
Act Effct Green (s)		53.0	53.0	53.0	53.0	52.0		42.0		52.0	52.0	
Actuated g/C Ratio		0.29	0.29	0.29	0.29	0.29		0.23		0.29	0.29	
v/c Ratio		0.81	0.05	0.18	0.32	0.64		0.63		0.53	0.63	
Control Delay		75.5	46.1	50.9	51.6	63.3		69.0		58.3	62.7	
Queue Delay		0.0	0.0	0.0	0.0	0.0		0.0		0.0	0.0	
Total Delay		75.5	46.1	50.9	51.6	63.3		69.0		58.3	62.7	
LOS		Е	D	D	D	Е		Е		Е	Е	
Approach Delay		74.0			58.4			69.0			60.6	
Approach LOS		Е			Е			Е			Е	
Queue Length 50th (ft)		354	15	27	161	299		284		268	299	
Queue Length 95th (ft)		453	36	59	236	413		392		369	413	
Internal Link Dist (ft)		500			598			531			79	

03/10/2022 McMahon Associates

Lane Group	Ø1	Ø3	Ø5	Ø7
Lane Configurations				
Traffic Volume (vph)				
Future Volume (vph)				
Ideal Flow (vphpl)				
( , , ,				
Storage Length (ft)				
Storage Lanes				
Taper Length (ft)				
Satd. Flow (prot)				
Flt Permitted				
Satd. Flow (perm)				
Right Turn on Red				
Satd. Flow (RTOR)				
Link Speed (mph)				
Link Distance (ft)				
Travel Time (s)				
Confl. Peds. (#/hr)				
Confl. Bikes (#/hr)				
Peak Hour Factor				
Heavy Vehicles (%)				
Shared Lane Traffic (%)				
Lane Group Flow (vph)				
Turn Type				
Protected Phases	1	3	5	7
Permitted Phases	ı	J	J	'
Detector Phase				
Switch Phase				
	2.0	2.0	2.0	2.0
Minimum Initial (s)	3.0	3.0	3.0	3.0
Minimum Split (s)	5.0	5.0	5.0	5.0
Total Split (s)	5.0	5.0	5.0	5.0
Total Split (%)	3%	3%	3%	3%
Yellow Time (s)	2.0	2.0	2.0	2.0
All-Red Time (s)	0.0	0.0	0.0	0.0
Lost Time Adjust (s)				
Total Lost Time (s)				
Lead/Lag	Lead	Lead	Lead	Lead
Lead-Lag Optimize?				
Recall Mode	Max	Max	Max	Max
Act Effct Green (s)				
Actuated g/C Ratio				
v/c Ratio				
Control Delay				
Queue Delay				
Total Delay				
LOS				
Approach Delay				
Approach LOS				
Queue Length 50th (ft)				
Queue Length 95th (ft)				
Internal Link Dist (ft)				
•				

	۶	<b>→</b>	•	•	←	4	4	<b>†</b>	/	<b>&gt;</b>	ļ	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Bay Length (ft)				100		130						
Base Capacity (vph)		397	394	169	537	452		423		506	461	
Starvation Cap Reductn		0	0	0	0	0		0		0	0	
Spillback Cap Reductn		0	0	0	0	0		0		0	0	
Storage Cap Reductn		0	0	0	0	0		0		0	0	
Reduced v/c Ratio		0.81	0.05	0.18	0.32	0.64		0.63		0.53	0.63	
Intersection Summary												
Area Type:	Other											
Cycle Length: 180												
Actuated Cycle Length: 180												
Offset: 0 (0%), Referenced t	to phase 4:	EBTL and	8:WBTL	., Start of	Green							
Natural Cycle: 115												
Control Type: Actuated-Coo	rdinated											
Maximum v/c Ratio: 0.81												
Intersection Signal Delay: 64	4.0			In	tersectior	n LOS: E						
Intersection Capacity Utiliza	tion 95.2%			IC	U Level	of Service	F					
Analysis Period (min) 15												
Splits and Phases: 1: Wel	bster Ave 8	. Washing	nton St &	Somervill	ο Δνο							
Splits and mases. 1. Wei	D3tG1 AVE C	· vvasiiiių	1011 Ot &	Sorrier viii	C AVC		*					
# <b>*</b> Ø2			77	Ø6			- 3	<del>*</del> Ø4(	R)			
2 g 20 S			3   <del>1</del> 0   S				56	59 S				
							A	₩ Ø8 (	R)			
							5 6	50 e				

Synchro 10 Report Page 3 03/10/2022 McMahon Associates

Lane Group	Ø1	Ø3	Ø5	Ø7	
Turn Bay Length (ft)					
Base Capacity (vph)					
Starvation Cap Reductn					
Spillback Cap Reductn					
Storage Cap Reductn					
Reduced v/c Ratio					
Intersection Summary					

Synchro 10 Report Page 4 03/10/2022 McMahon Associates

	٠	<b>→</b>	•	•	<b>←</b>	•	1	<b>†</b>	<i>&gt;</i>	<b>/</b>	<b>+</b>	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations					f)			4				7
Traffic Volume (veh/h)	0	0	0	0	502	32	197	42	0	0	0	100
Future Volume (Veh/h)	0	0	0	0	502	32	197	42	0	0	0	100
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.94	0.94	0.94	0.94	0.94	0.94	0.84	0.84	0.84
Hourly flow rate (vph)	0	0	0	0	534	34	210	45	0	0	0	119
Pedestrians											212	
Lane Width (ft)											12.0	
Walking Speed (ft/s)											3.5	
Percent Blockage											20	
Right turn flare (veh)												
Median type		None			None							
Median storage veh)												
Upstream signal (ft)					50							
pX, platoon unblocked	0.85						0.85	0.85		0.85	0.85	0.85
vC, conflicting volume	780			0			670	780	0	786	763	763
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	653			0			523	653	0	659	633	633
tC, single (s)	4.1			4.1			*5.5	*5.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	100			100			22	86	100	100	100	64
cM capacity (veh/h)	633			1623			270	317	1085	193	269	328
Direction, Lane #	WB 1	NB 1	SB 1									
Volume Total	568	255	119									
Volume Left	0	210	0									
Volume Right	34	0	119									
cSH	1700	277	328									
Volume to Capacity	0.33	0.92	0.36									
Queue Length 95th (ft)	0	212	40									
Control Delay (s)	0.0	75.6	22.1									
Lane LOS		F	С									
Approach Delay (s)	0.0	75.6	22.1									
Approach LOS		F	С									
Intersection Summary												
Average Delay			23.3									
Intersection Capacity Utiliza	ition		58.1%	IC	U Level o	of Service			В			
Analysis Period (min)			15									
* User Entered Value												

# Liberty Cannabis 3: Bow Street & Pedestrian Crossing

	•	•	<b>†</b>	<b>/</b>	<b>\</b>	ļ		
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT	Ø2	
Lane Configurations			<b>1</b>					
Traffic Volume (vph)	0	0	534	0	0	0		
Future Volume (vph)	0	0	534	0	0	0		
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900		
Satd. Flow (prot)	0	0	1863	0	0	0		
Flt Permitted	-			-	-			
Satd. Flow (perm)	0	0	1863	0	0	0		
Right Turn on Red		No		No				
Satd. Flow (RTOR)								
Link Speed (mph)	30		30			30		
Link Distance (ft)	184		50			50		
Travel Time (s)	4.2		1.1			1.1		
Peak Hour Factor	0.92	0.92	0.94	0.94	0.92	0.92		
Shared Lane Traffic (%)								
Lane Group Flow (vph)	0	0	568	0	0	0		
Turn Type			NA					
Protected Phases			1				2	
Permitted Phases								
Detector Phase			1					
Switch Phase			-					
Minimum Initial (s)			25.0				10.0	
Minimum Split (s)			29.0				16.0	
Total Split (s)			29.0				16.0	
Total Split (%)			64.4%				36%	
Yellow Time (s)			3.0				2.0	
All-Red Time (s)			1.0				1.0	
Lost Time Adjust (s)			0.0				·	
Total Lost Time (s)			4.0					
Lead/Lag			Lead				Lag	
Lead-Lag Optimize?							- 3	
Recall Mode			Min				None	
Act Effct Green (s)			40.0					
Actuated g/C Ratio			0.67					
v/c Ratio			0.46					
Control Delay			6.3					
Queue Delay			0.0					
Total Delay			6.3					
LOS			Α					
Approach Delay			6.3					
Approach LOS			Α					
Queue Length 50th (ft)			80					
Queue Length 95th (ft)			132					
Internal Link Dist (ft)	104		1			1		
Turn Bay Length (ft)								
Base Capacity (vph)			1242					
Starvation Cap Reductn			0					
Spillback Cap Reductn			0					
Storage Cap Reductn			0					
Reduced v/c Ratio			0.46					
			-					

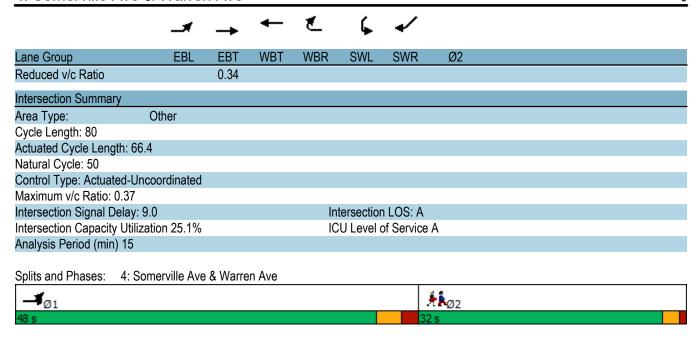
Synchro 10 Report Page 5 03/10/2022 McMahon Associates

Intersection Summary		
Area Type: Other		
Cycle Length: 45		
Actuated Cycle Length: 60		
Natural Cycle: 45		
Control Type: Actuated-Uncoordinated		
Maximum v/c Ratio: 0.46		
Intersection Signal Delay: 6.3	Intersection LOS: A	
Intersection Capacity Utilization 31.4%	ICU Level of Service A	
Analysis Period (min) 15		

Splits and Phases: 3: Bow Street & Pedestrian Crossing

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	_#	-	<b>←</b>	€_	Ĺ	✓		
Lane Group	EBL	EBT	WBT	WBR	SWL	SWR	Ø2	
Lane Configurations		414						
Traffic Volume (vph)	239	507	0	0	0	0		
Future Volume (vph)	239	507	0	0	0	0		
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900		
Satd. Flow (prot)	0	3528	0	0	0	0		
Flt Permitted		0.984						
Satd. Flow (perm)	0	3528	0	0	0	0		
Right Turn on Red	Yes	00_0		Yes	•	Yes		
Satd. Flow (RTOR)		151						
Link Speed (mph)		30	30		30			
Link Distance (ft)		240	98		71			
Travel Time (s)		5.5	2.2		1.6			
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92		
Heavy Vehicles (%)	0.32	1%	2%	2%	2%	2%		
Shared Lane Traffic (%)		1 /0	2 /0	2 /0	2 /0	2 /0		
Lane Group Flow (vph)	0	811	0	0	0	0		
Turn Type	Split	NA	U	U	· ·	0		
Protected Phases	1	1					2	
Permitted Phases		•					2	
Detector Phase	1	1						
Switch Phase								
Minimum Initial (s)	10.0	10.0					5.0	
Minimum Split (s)	15.0	15.0					32.0	
Total Split (s)	48.0	48.0					32.0	
Total Split (%)	60.0%	60.0%					40%	
Yellow Time (s)	3.0	3.0					2.0	
All-Red Time (s)	2.0	2.0					1.0	
Lost Time Adjust (s)	2.0	0.0					1.0	
Total Lost Time (s)		5.0						
Lead/Lag	Lead	Lead					Lag	
Lead-Lag Optimize?	LCau	LCau					Lag	
Recall Mode	Min	Min					None	
Act Effct Green (s)	IVIIII	39.9					Hono	
Actuated g/C Ratio		0.60						
v/c Ratio		0.00						
Control Delay		9.0						
Queue Delay		0.0						
Total Delay		9.0						
LOS		3.0 A						
Approach Delay		9.0						
Approach LOS		3.0 A						
Queue Length 50th (ft)		92						
Queue Length 95th (ft)		132						
Internal Link Dist (ft)		160	18		1			
Turn Bay Length (ft)		100	10					
Base Capacity (vph)		2398						
Starvation Cap Reductn		2390						
Spillback Cap Reductn		0						
Storage Cap Reductin		0						
Storage Cap Reductin		U						



→ <b>→ → → → →</b>
Movement EBT EBR WBL WBT NBL NBR
Lane Configurations ††
Traffic Volume (veh/h) 702 0 0 0 44
Future Volume (Veh/h) 702 0 0 0 44
Sign Control Free Stop
Grade 0% 0% 0%
Peak Hour Factor 0.93 0.93 0.92 0.92 0.81 0.81
Hourly flow rate (vph) 755 0 0 0 54
Pedestrians
Lane Width (ft)
Walking Speed (ft/s)
Percent Blockage
Right turn flare (veh)
Median type None None
Median storage veh)
Upstream signal (ft) 240
pX, platoon unblocked
vC, conflicting volume 755 755 378
vC1, stage 1 conf vol
vC2, stage 2 conf vol
vCu, unblocked vol 755 755 378
tC, single (s) 4.1 6.8 6.9
tC, 2 stage (s)
tF(s) 2.2 3.5 3.3
p0 queue free % 100 100 91
cM capacity (veh/h) 851 345 626
Direction, Lane # EB 1 EB 2 NB 1
Volume Total 378 378 54
Volume Left 0 0 0
Volume Right 0 0 54 cSH 1700 1700 626
Volume to Capacity 0.22 0.22 0.09
Queue Length 95th (ft) 0 0 7
Control Delay (s) 0.0 0.0 11.3
Lane LOS B
Approach Delay (s) 0.0 11.3
Approach LOS B
Intersection Summary
Average Delay 0.8
Intersection Capacity Utilization 29.4% ICU Level of Service
Analysis Period (min) 15

## APPENDIX F

2022 Build Capacity/Level-of-Service Analysis

## Liberty Cannabis 1: Webster Ave & Washington St & Somerville Ave

	۶	<b>→</b>	•	•	+	•	1	†	<i>&gt;</i>	<b>/</b>	<b>↓</b>	
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ર્ન	7	ሻ	<b></b>	7		4		ሻ	ĵ <sub>a</sub>	
Traffic Volume (vph)	52	202	44	45	228	194	8	143	28	211	299	53
Future Volume (vph)	52	202	44	45	228	194	8	143	28	211	299	53
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	100		130	0		0	0		0
Storage Lanes	0		1	1		1	0		0	1		0
Taper Length (ft)	25			25			25			25		
Satd. Flow (prot)	0	1755	1482	1583	1727	1495	0	1765	0	1703	1760	0
Flt Permitted		0.708		0.388				0.998		0.950		
Satd. Flow (perm)	0	1246	1284	612	1727	1495	0	1765	0	1703	1760	0
Right Turn on Red			No			No			Yes			No
Satd. Flow (RTOR)								5				
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		580			678			611			159	
Travel Time (s)		13.2			15.4			13.9			3.6	
Confl. Peds. (#/hr)	20		36	36					28			45
Confl. Bikes (#/hr)									2			28
Peak Hour Factor	0.89	0.89	0.89	0.93	0.93	0.93	0.77	0.77	0.77	0.86	0.86	0.86
Heavy Vehicles (%)	4%	8%	9%	14%	10%	8%	0%	4%	0%	6%	2%	4%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	285	49	48	245	209	0	232	0	245	410	0
Turn Type	Perm	NA	Perm	Perm	NA	Over	Split	NA		Split	NA	
Protected Phases		4			8	2	6	6		2	2	
Permitted Phases	4		4	8								
Detector Phase	4	4	4	8	8	2	6	6		2	2	
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0		10.0	10.0	
Minimum Split (s)	34.0	34.0	34.0	34.0	34.0	33.0	31.0	31.0		33.0	33.0	
Total Split (s)	59.0	59.0	59.0	59.0	59.0	58.0	48.0	48.0		58.0	58.0	
Total Split (%)	32.8%	32.8%	32.8%	32.8%	32.8%	32.2%	26.7%	26.7%		32.2%	32.2%	
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0		3.0	3.0	
All-Red Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Lost Time Adjust (s)		0.0	0.0	0.0	0.0	0.0		0.0		0.0	0.0	
Total Lost Time (s)		6.0	6.0	6.0	6.0	6.0		6.0		6.0	6.0	
Lead/Lag	Lag	Lag	Lag	Lag	Lag	Lag	Lag	Lag		Lag	Lag	
Lead-Lag Optimize?												
Recall Mode	C-Max			C-Max	C-Max	Max	Max	Max		Max	Max	
Act Effct Green (s)		53.0	53.0	53.0	53.0	52.0		42.0		52.0	52.0	
Actuated g/C Ratio		0.29	0.29	0.29	0.29	0.29		0.23		0.29	0.29	
v/c Ratio		0.78	0.13	0.27	0.48	0.48		0.56		0.50	0.81	
Control Delay		74.0	47.9	53.7	56.1	57.5		65.5		57.4	72.8	
Queue Delay		0.0	0.0	0.0	0.0	0.0		0.0		0.0	0.0	
Total Delay		74.0	47.9	53.7	56.1	57.5		65.5		57.4	72.8	
LOS		Е	D	D	Е	E		Е		Е	E	
Approach Delay		70.1			56.4			65.5			67.1	
Approach LOS		Е			Е			Е			Е	
Queue Length 50th (ft)		310	43	44	240	206		239		243	453	
Queue Length 95th (ft)		432	80	88	335	297		280		321	561	
Internal Link Dist (ft)		500			598			531			79	

03/10/2022 McMahon Associates Synchro 10 Report Page 1

Lane Group	Ø1	Ø3	Ø5	Ø7
Lane Configurations	~ .	~~	~~	~.
Traffic Volume (vph)				
Future Volume (vph)				
Ideal Flow (vphpl)				
Storage Length (ft)				
Storage Lanes				
Taper Length (ft)				
Satd. Flow (prot)				
Flt Permitted				
Satd. Flow (perm)				
Right Turn on Red				
Satd. Flow (RTOR)				
Link Speed (mph)				
Link Distance (ft)				
Travel Time (s)				
Confl. Peds. (#/hr)				
Confl. Bikes (#/hr)				
Peak Hour Factor				
Heavy Vehicles (%)				
Shared Lane Traffic (%)				
Lane Group Flow (vph)				
Turn Type				
Protected Phases	1	3	5	7
Permitted Phases	ı	J	J	'
Detector Phase				
Switch Phase	2.0	2.0	2.0	2.0
Minimum Initial (s)	3.0	3.0	3.0	3.0
Minimum Split (s)	5.0	5.0	5.0	5.0
Total Split (s)	5.0	5.0	5.0	5.0
Total Split (%)	3%	3%	3%	3%
Yellow Time (s)	2.0	2.0	2.0	2.0
All-Red Time (s)	0.0	0.0	0.0	0.0
Lost Time Adjust (s)				
Total Lost Time (s)				
Lead/Lag	Lead	Lead	Lead	Lead
Lead-Lag Optimize?				
Recall Mode	Max	Max	Max	Max
Act Effct Green (s)				
Actuated g/C Ratio				
v/c Ratio				
Control Delay				
Queue Delay				
Total Delay				
LOS				
Approach LOS				
Approach LOS				
Queue Length 50th (ft)				
Queue Length 95th (ft) Internal Link Dist (ft)				

## 1: Webster Ave & Washington St & Somerville Ave

2022 Build

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		<b>→</b>	*	•	•	`	7	ı		*	+	*
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Bay Length (ft)				100		130						
Base Capacity (vph)		366	378	180	508	431		415		491	508	
Starvation Cap Reductn		0	0	0	0	0		0		0	0	
Spillback Cap Reductn		0	0	0	0	0		0		0	0	
Storage Cap Reductn		0	0	0	0	0		0		0	0	
Reduced v/c Ratio		0.78	0.13	0.27	0.48	0.48		0.56		0.50	0.81	

Intersection Summary

Area Type: Other

Cycle Length: 180 Actuated Cycle Length: 180

Offset: 0 (0%), Referenced to phase 4:EBTL and 8:WBTL, Start of Green

Natural Cycle: 115

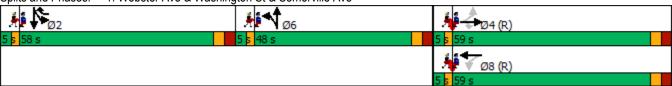
Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.81
Intersection Signal Delay: 64.3
Intersection Capacity Utilization 98.7%

Intersection LOS: E
ICU Level of Service F

Analysis Period (min) 15

Splits and Phases: 1: Webster Ave & Washington St & Somerville Ave



Lane Group	Ø1	Ø3	Ø5	Ø7	
Turn Bay Length (ft)					
Base Capacity (vph)					
Starvation Cap Reductn					
Spillback Cap Reductn					
Storage Cap Reductn					
Reduced v/c Ratio					
Intersection Summary					

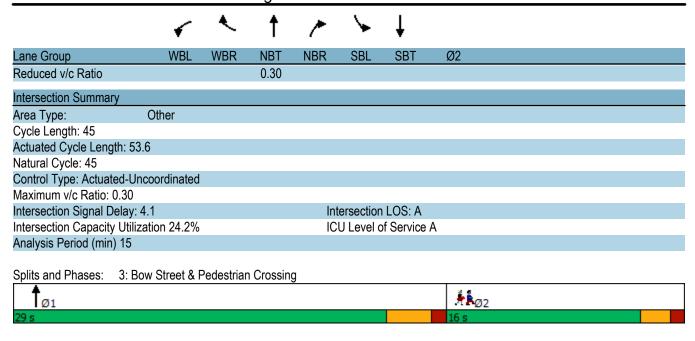
	۶	<b>→</b>	•	•	+	•	1	†	<i>&gt;</i>	<b>&gt;</b>	<b>+</b>	√
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations					f)			4				7
Traffic Volume (veh/h)	0	0	0	0	376	13	162	47	0	0	0	66
Future Volume (Veh/h)	0	0	0	0	376	13	162	47	0	0	0	66
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.76	0.76	0.76
Hourly flow rate (vph)	0	0	0	0	409	14	176	51	0	0	0	87
Pedestrians											54	
Lane Width (ft)											12.0	
Walking Speed (ft/s)											3.5	
Percent Blockage											5	
Right turn flare (veh)												
Median type		None			None							
Median storage veh)												
Upstream signal (ft)					50							
pX, platoon unblocked	0.92						0.92	0.92		0.92	0.92	0.92
vC, conflicting volume	477			0			503	477	0	496	470	470
vC1, stage 1 conf vol				-					-			
vC2, stage 2 conf vol												
vCu, unblocked vol	389			0			417	389	0	409	381	381
tC, single (s)	4.1			4.1			*5.5	*5.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)							0.0	0.0			0.0	V
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	100			100			65	90	100	100	100	85
cM capacity (veh/h)	1021			1623			497	534	1085	430	482	585
Direction, Lane #	WB 1	NB 1	SB 1									
Volume Total	423	227	87									
Volume Left	0	176	0									
Volume Right	14	0	87									
cSH	1700	505	585									
Volume to Capacity	0.25	0.45	0.15									
Queue Length 95th (ft)	0	57	13									
Control Delay (s)	0.0	17.8	12.2									
Lane LOS		С	В									
Approach Delay (s)	0.0	17.8	12.2									
Approach LOS		С	В									
Intersection Summary												
Average Delay			6.9									
Intersection Capacity Utiliza	tion		46.2%	IC	U Level	of Service			Α			
Analysis Period (min)			15									
* User Entered Value												

# Liberty Cannabis 3: Bow Street & Pedestrian Crossing

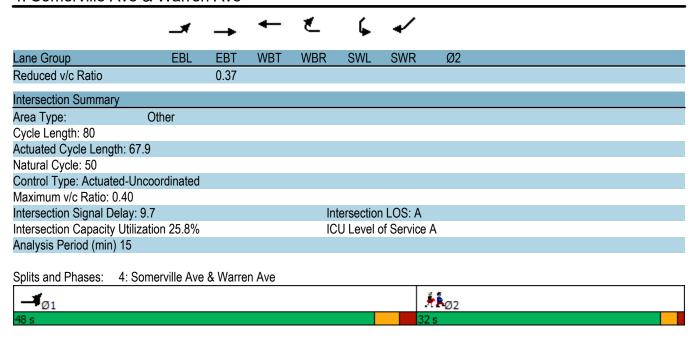
	•	•	<b>†</b>	<b>/</b>	<b>\</b>	<b>↓</b>		
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT	Ø2	
Lane Configurations	.,,,,		<u>↑</u>	.,_,,				
Traffic Volume (vph)	0	0	389	0	0	0		
Future Volume (vph)	0	0	389	0	0	0		
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900		
Satd. Flow (prot)	0	0	1792	0	0	0		
Flt Permitted	- U	- U	1752	0	0	U		
Satd. Flow (perm)	0	0	1792	0	0	0		
Right Turn on Red		No	1752	No	U	<u> </u>		
Satd. Flow (RTOR)		140		110				
Link Speed (mph)	30		30			30		
Link Distance (ft)	184		50			50		
Travel Time (s)	4.2		1.1			1.1		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92		
Heavy Vehicles (%)	2%	2%	6%	2%	2%	2%		
Shared Lane Traffic (%)	270	2/0	370	2/0	2/0	2/0		
Lane Group Flow (vph)	0	0	423	0	0	0		
Turn Type		- 0	NA					
Protected Phases			1				2	
Permitted Phases								
Detector Phase			1					
Switch Phase								
Minimum Initial (s)			25.0				10.0	
Minimum Split (s)			29.0				16.0	
Total Split (s)			29.0				16.0	
Total Split (%)			64.4%				36%	
Yellow Time (s)			3.0				2.0	
All-Red Time (s)			1.0				1.0	
Lost Time Adjust (s)			0.0				7.0	
Total Lost Time (s)			4.0					
Lead/Lag			Lead				Lag	
Lead-Lag Optimize?							_~9	
Recall Mode			Min				None	
Act Effct Green (s)			42.9				110110	
Actuated g/C Ratio			0.80					
v/c Ratio			0.30					
Control Delay			4.1					
Queue Delay			0.0					
Total Delay			4.1					
LOS			A					
Approach Delay			4.1					
Approach LOS			A					
Queue Length 50th (ft)			54					
Queue Length 95th (ft)			92					
Internal Link Dist (ft)	104		1			1		
Turn Bay Length (ft)	107							
Base Capacity (vph)			1433					
Starvation Cap Reductn			0					
Spillback Cap Reductn			0					
Storage Cap Reductn			0					
- Clorago Dap Modubili			U					

Synchro 10 Report Page 5 03/10/2022 McMahon Associates

## 3: Bow Street & Pedestrian Crossing



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Lane Group	EBL	EBT	WBT	WBR	SWL	SWR	Ø2	
Lane Configurations		414			0	• • • • • • • • • • • • • • • • • • • •	~-	
Traffic Volume (vph)	209	563	0	0	0	0		
Future Volume (vph)	209	563	0	0	0	0		
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900		
Satd. Flow (prot)	0	3453	0	0	0	0		
Flt Permitted	0	0.987	U	- U		- U		
Satd. Flow (perm)	0	3453	0	0	0	0		
Right Turn on Red	Yes	0400	U	Yes	U	Yes		
Satd. Flow (RTOR)	163	103		163		163		
Link Speed (mph)		30	30		30			
Link Distance (ft)		240	98		71			
Travel Time (s)		5.5	2.2		1.6			
	0.00			0.02		0.92		
Peak Hour Factor	0.90	0.90	0.92	0.92	0.92			
Heavy Vehicles (%)	1%	4%	2%	2%	2%	2%		
Shared Lane Traffic (%)		050	^	^	^	^		
Lane Group Flow (vph)	0	858	0	0	0	0		
Turn Type	Split	NA					•	
Protected Phases	1	1					2	
Permitted Phases								
Detector Phase	1	1						
Switch Phase								
Minimum Initial (s)	10.0	10.0					5.0	
Minimum Split (s)	15.0	15.0					32.0	
Total Split (s)	48.0	48.0					32.0	
Total Split (%)	60.0%	60.0%					40%	
Yellow Time (s)	3.0	3.0					2.0	
All-Red Time (s)	2.0	2.0					1.0	
Lost Time Adjust (s)		0.0						
Total Lost Time (s)		5.0						
Lead/Lag	Lead	Lead					Lag	
Lead-Lag Optimize?								
Recall Mode	Min	Min					None	
Act Effct Green (s)		41.8						
Actuated g/C Ratio		0.62						
v/c Ratio		0.40						
Control Delay		9.7						
Queue Delay		0.0						
Total Delay		9.7						
LOS		A						
Approach Delay		9.7						
Approach LOS		A						
Queue Length 50th (ft)		110						
Queue Length 95th (ft)		152						
Internal Link Dist (ft)		160	18		1			
Turn Bay Length (ft)		100	10					
Base Capacity (vph)		2303						
Starvation Cap Reductn								
		0						
Spillback Cap Reductn								
Storage Cap Reductn		0						



	-	$\rightarrow$	•	←	•	<b>/</b>
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>^</b>					7
Traffic Volume (veh/h)	701	0	0	0	0	72
Future Volume (Veh/h)	701	0	0	0	0	72
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.93	0.93	0.92	0.92	0.73	0.73
Hourly flow rate (vph)	754	0	0	0	0	99
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None			None		
Median storage veh)						
Upstream signal (ft)				240		
pX, platoon unblocked						
vC, conflicting volume			754		754	377
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			754		754	377
tC, single (s)			4.1		6.8	7.0
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.3
p0 queue free %			100		100	84
cM capacity (veh/h)			852		345	618
Direction, Lane #	EB 1	EB 2	NB 1			
Volume Total	377	377	99			
Volume Left	0	0	0			
Volume Right	0	0	99			
cSH	1700	1700	618			
Volume to Capacity	0.22	0.22	0.16			
Queue Length 95th (ft)	0.22	0.22	14			
Control Delay (s)	0.0	0.0	11.9			
	0.0	0.0	_			
Lane LOS Approach Delay (s)	0.0		11.9			
Approach LOS	0.0		11.9			
••			Ь			
Intersection Summary						
Average Delay			1.4			
Intersection Capacity Utiliz	ation		30.5%	IC	U Level c	f Service
Analysis Period (min)			15			

	ၨ	<b>→</b>	•	•	<b>←</b>	•	•	<b>†</b>	~	<b>/</b>	<b></b>	✓
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4	7	ሻ	<b>↑</b>	7		4		ች	1>	
Traffic Volume (vph)	78	220	35	26	217	246	23	300	13	167	202	23
Future Volume (vph)	78	220	35	26	217	246	23	300	13	167	202	23
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	1000	0	100	1000	130	0	1000	0	0	1000	0
Storage Lanes	0		1	1		1	0		0	1		0
Taper Length (ft)	25		•	25		•	25		•	25		•
Satd. Flow (prot)	0	1803	1615	1612	1863	1568	0	1849	0	1770	1772	0
Flt Permitted	•	0.646		0.346			•	0.997	•	0.950		•
Satd. Flow (perm)	0	1157	1405	560	1863	1568	0	1849	0	1770	1772	0
Right Turn on Red	•		No			No	•		Yes			No
Satd. Flow (RTOR)								1				
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		580			678			611			159	
Travel Time (s)		13.2			15.4			13.9			3.6	
Confl. Peds. (#/hr)	41		35	35					60		0.0	74
Confl. Bikes (#/hr)									26			8
Peak Hour Factor	0.95	0.95	0.95	0.94	0.94	0.94	0.94	0.94	0.94	0.96	0.96	0.96
Heavy Vehicles (%)	4%	4%	0%	12%	2%	3%	0%	1%	0%	2%	3%	0%
Shared Lane Traffic (%)	.,,	1,70	0,0	1270	2,0	0,0	0,0	1,0	0,0	_,,	0,0	0,70
Lane Group Flow (vph)	0	314	37	28	231	262	0	357	0	174	234	0
Turn Type	Perm	NA	Perm	Perm	NA	Over	Split	NA		Split	NA	
Protected Phases	1 01111	4	1 01111	1 01111	8	2	6	6		2	2	
Permitted Phases	4	•	4	8		_		•		_	_	
Detector Phase	4	4	4	8	8	2	6	6		2	2	
Switch Phase		•				_				_	_	
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0		10.0	10.0	
Minimum Split (s)	34.0	34.0	34.0	34.0	34.0	33.0	31.0	31.0		33.0	33.0	
Total Split (s)	59.0	59.0	59.0	59.0	59.0	58.0	48.0	48.0		58.0	58.0	
Total Split (%)	32.8%	32.8%	32.8%	32.8%	32.8%	32.2%	26.7%	26.7%		32.2%	32.2%	
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0		3.0	3.0	
All-Red Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Lost Time Adjust (s)		0.0	0.0	0.0	0.0	0.0		0.0		0.0	0.0	
Total Lost Time (s)		6.0	6.0	6.0	6.0	6.0		6.0		6.0	6.0	
Lead/Lag	Lag	Lag	Lag	Lag	Lag	Lag	Lag	Lag		Lag	Lag	
Lead-Lag Optimize?		- 5	- 3	- 5		- 5	- 5	- 5		- 5	- 0	
Recall Mode	C-Max	C-Max	C-Max	C-Max	C-Max	Max	Max	Max		Max	Max	
Act Effct Green (s)		53.0	53.0	53.0	53.0	52.0		42.0		52.0	52.0	
Actuated g/C Ratio		0.29	0.29	0.29	0.29	0.29		0.23		0.29	0.29	
v/c Ratio		0.92	0.09	0.17	0.42	0.58		0.83		0.34	0.46	
Control Delay		93.6	47.0	50.7	54.1	60.7		82.0		52.8	56.0	
Queue Delay		0.0	0.0	0.0	0.0	0.0		0.0		0.0	0.0	
Total Delay		93.6	47.0	50.7	54.1	60.7		82.0		52.8	56.0	
LOS		F	D	D	D	E		F		D	E	
Approach Delay		88.7			57.2			82.0			54.6	
Approach LOS		F			E			F			D	
Queue Length 50th (ft)		362	32	25	222	266		405		164	228	
Queue Length 95th (ft)		#562	66	57	311	372		#562		240	321	
Internal Link Dist (ft)		500			598			531			79	

03/10/2022 McMahon Associates Synchro 10 Report Page 1

Lane Group	Ø1	Ø3	Ø5	Ø7
Lane Configurations		20	20	ω,
Traffic Volume (vph)				
Future Volume (vph)				
Ideal Flow (vphpl)				
Storage Length (ft)				
Storage Lanes				
Taper Length (ft)				
Satd. Flow (prot)				
Flt Permitted				
Satd. Flow (perm)				
Right Turn on Red				
Satd. Flow (RTOR)				
Link Speed (mph)				
Link Distance (ft)				
Travel Time (s)				
Confl. Peds. (#/hr)				
Confl. Bikes (#/hr)				
Peak Hour Factor				
Heavy Vehicles (%)				
Shared Lane Traffic (%)				
Lane Group Flow (vph)				
Turn Type				
Protected Phases	1	3	5	7
Permitted Phases	ı	J	J	ı
Detector Phase				
Switch Phase				
	2.0	2.0	2.0	2.0
Minimum Initial (s)	3.0	3.0	3.0	3.0
Minimum Split (s)	5.0	5.0	5.0	5.0
Total Split (s)	5.0	5.0	5.0	5.0
Total Split (%)	3%	3%	3%	3%
Yellow Time (s)	2.0	2.0	2.0	2.0
All-Red Time (s)	0.0	0.0	0.0	0.0
Lost Time Adjust (s)				
Total Lost Time (s)				
Lead/Lag	Lead	Lead	Lead	Lead
Lead-Lag Optimize?				
Recall Mode	Max	Max	Max	Max
Act Effct Green (s)				
Actuated g/C Ratio				
v/c Ratio				
Control Delay				
Queue Delay				
Total Delay				
LOS				
Approach Delay				
Approach LOS				
Queue Length 50th (ft)				
Queue Length 95th (ft)				
Internal Link Dist (ft)				

## 1: Webster Ave & Washington St & Somerville Ave

2022 Build

	•	-	•	•	•	•	1	Ť		-	¥	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Bay Length (ft)				100		130						
Base Capacity (vph)		340	413	164	548	452		432		511	511	
Starvation Cap Reductn		0	0	0	0	0		0		0	0	
Spillback Cap Reductn		0	0	0	0	0		0		0	0	
Storage Cap Reductn		0	0	0	0	0		0		0	0	
Reduced v/c Ratio		0.92	0.09	0.17	0.42	0.58		0.83		0.34	0.46	

#### Intersection Summary

Area Type: Other

Cycle Length: 180 Actuated Cycle Length: 180

Offset: 0 (0%), Referenced to phase 4:EBTL and 8:WBTL, Start of Green

Natural Cycle: 115

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.92

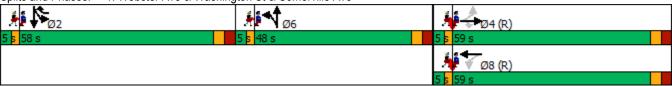
Intersection Signal Delay: 68.7 Intersection LOS: E
Intersection Capacity Utilization 98.1% ICU Level of Service F

Analysis Period (min) 15

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 1: Webster Ave & Washington St & Somerville Ave



Lane Group	Ø1	Ø3	Ø5	Ø7	
Turn Bay Length (ft)					
Base Capacity (vph)					
Starvation Cap Reductn					
Spillback Cap Reductn					
Storage Cap Reductn					
Reduced v/c Ratio					
Intersection Summary					

Synchro 10 Report Page 4 03/10/2022 McMahon Associates

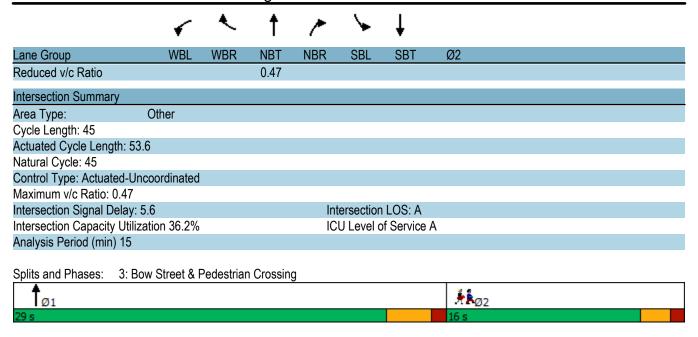
	٠	<b>→</b>	•	•	<b>←</b>	•	1	<b>†</b>	<i>&gt;</i>	<b>/</b>	<b>+</b>	√
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations					ĵ»			4				7
Traffic Volume (veh/h)	0	0	0	0	602	22	188	50	0	0	0	59
Future Volume (Veh/h)	0	0	0	0	602	22	188	50	0	0	0	59
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.74	0.74	0.74	0.75	0.75	0.75
Hourly flow rate (vph)	0	0	0	0	654	24	254	68	0	0	0	79
Pedestrians											77	
Lane Width (ft)											12.0	
Walking Speed (ft/s)											3.5	
Percent Blockage											7	
Right turn flare (veh)												
Median type		None			None							
Median storage veh)												
Upstream signal (ft)					50		0.01	2.24			2.21	2.2.1
pX, platoon unblocked	0.84			_			0.84	0.84		0.84	0.84	0.84
vC, conflicting volume	755			0			745	755	0	777	743	743
vC1, stage 1 conf vol												
vC2, stage 2 conf vol	040			0			004	040	0	000	<b>500</b>	500
vCu, unblocked vol	612			0			601	612	0	639	598	598
tC, single (s)	4.1			4.1			*5.5	*5.5	6.2	7.1	6.5	6.3
tC, 2 stage (s)	2.2			2.2			2.5	4.0	2.2	2.5	4.0	2.4
tF (s)	100			100			3.5 26	4.0 82	3.3 100	3.5	4.0	3.4
p0 queue free %	752			1623			341	379	1085	100 247	100 323	79 383
cM capacity (veh/h)				1023			341	3/9	1005	247	323	303
Direction, Lane #	WB 1	NB 1	SB 1									
Volume Total	678	322	79									
Volume Left	0	254	0									
Volume Right	24	0	79									
cSH	1700	349	383									
Volume to Capacity	0.40	0.92	0.21									
Queue Length 95th (ft)	0	236	19									
Control Delay (s)	0.0	65.9	16.8									
Lane LOS	0.0	F CF O	C									
Approach LOS	0.0	65.9 F	16.8 C									
Approach LOS		Г	C									
Intersection Summary												
Average Delay			20.9									
Intersection Capacity Utiliza	ation		59.9%	IC	U Level o	of Service			В			
Analysis Period (min)			15									
* User Entered Value												

# Liberty Cannabis 3: Bow Street & Pedestrian Crossing

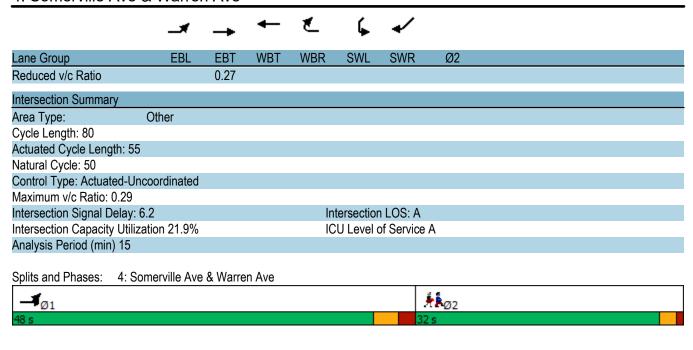
	•	•	<b>†</b>	<b>/</b>	<b>&gt;</b>	ţ	
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT	Ø2
Lane Configurations			<b>†</b>				<del> </del>
Traffic Volume (vph)	0	0	624	0	0	0	
Future Volume (vph)	0	0	624	0	0	0	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Satd. Flow (prot)	0	0	1792	0	0	0	
Flt Permitted		- U	1732			<u> </u>	
Satd. Flow (perm)	0	0	1792	0	0	0	
Right Turn on Red		No	1732	No		<u> </u>	
Satd. Flow (RTOR)		110		110			
Link Speed (mph)	30		30			30	
Link Distance (ft)	184		50			50	
Travel Time (s)	4.2		1.1			1.1	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Heavy Vehicles (%)	2%	2%	6%	2%	2%	2%	
Shared Lane Traffic (%)	2 /0	2 /0	0 /0	<b>2</b> /0	2 /0	<b>L</b> /0	
Lane Group Flow (vph)	0	0	678	0	0	0	
Turn Type	0	U	NA	- 0	- 0	- 0	
Protected Phases			1				2
Permitted Phases							2
Detector Phase			1				
Switch Phase							
Minimum Initial (s)			25.0				10.0
Minimum Split (s)			29.0				16.0
Total Split (s)			29.0				16.0
Total Split (%)			64.4%				36%
Yellow Time (s)			3.0				2.0
All-Red Time (s)			1.0				1.0
Lost Time Adjust (s)			0.0				1.0
Total Lost Time (s)			4.0				
Lead/Lag			Lead				Lag
Lead-Lag Optimize?			Leau				Lag
Recall Mode			Min				None
Act Effct Green (s)			42.9				None
Actuated g/C Ratio			0.80				
v/c Ratio			0.80				
			5.6				
Control Delay			0.0				
Queue Delay			5.6				
Total Delay LOS							
Approach Delay			A 5.6				
Approach LOS			A 106				
Queue Length 50th (ft)			180				
Queue Length 95th (ft)	104		180			1	
Internal Link Dist (ft)	104		I			T	
Turn Bay Length (ft)			1422				
Base Capacity (vph)			1433				
Starvation Cap Reductn			0				
Spillback Cap Reductn			0				
Storage Cap Reductn			0				

Synchro 10 Report Page 5 03/10/2022 McMahon Associates

## 3: Bow Street & Pedestrian Crossing



	_#	-	<b>←</b>	€_	6	✓		
Lane Group	EBL	EBT	WBT	WBR	SWL	SWR	Ø2	
Lane Configurations		414						
Traffic Volume (vph)	238	392	0	0	0	0		
Future Volume (vph)	238	392	0	0	0	0		
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900		
Satd. Flow (prot)	0	3498	0	0	0	0		
Flt Permitted		0.981						
Satd. Flow (perm)	0	3498	0	0	0	0		
Right Turn on Red	Yes	0100		Yes		Yes		
Satd. Flow (RTOR)	100	247		100		100		
Link Speed (mph)		30	30		30			
Link Distance (ft)		240	98		71			
Travel Time (s)		5.5	2.2		1.6			
Peak Hour Factor	0.86	0.86	0.92	0.92	0.92	0.92		
Heavy Vehicles (%)	0.00	2%	2%	2%	2%	2%		
Shared Lane Traffic (%)	0 70	<b>2</b> /0	2 /0	<b>Z</b> /0	2 /0	<b>Z</b> /0		
Lane Group Flow (vph)	0	733	0	0	0	0		
Turn Type	Split	NA	U	U	U	U		
Protected Phases	Split 1	1					2	
Permitted Phases	1						2	
Detector Phase	1	1						
Switch Phase								
Minimum Initial (s)	10.0	10.0					5.0	
Minimum Split (s)	15.0	15.0					32.0	
Total Split (s)	48.0	48.0					32.0	
Total Split (%)	60.0%	60.0%					40%	
Yellow Time (s)	3.0	3.0					2.0	
All-Red Time (s)	2.0	2.0					1.0	
Lost Time Adjust (s)	2.0	0.0					1.0	
Total Lost Time (s)		5.0						
Lead/Lag	Lead	Lead					Lag	
Lead-Lag Optimize?	Leau	Leau					Lag	
Recall Mode	Min	Min					None	
Act Effct Green (s)	IVIIII	37.9					NOTIC	
Actuated g/C Ratio		0.69						
v/c Ratio		0.09						
Control Delay		6.2						
Queue Delay		0.0						
Total Delay		6.2						
LOS		6.2 A						
Approach Delay		6.2						
Approach LOS		6.2 A						
Queue Length 50th (ft)		64						
Queue Length 95th (ft)		91						
Internal Link Dist (ft)		160	18		1			
		100	10					
Turn Bay Length (ft)		2710						
Base Capacity (vph)		2719						
Starvation Cap Reductn		0						
Spillback Cap Reductn		0						
Storage Cap Reductn		0						

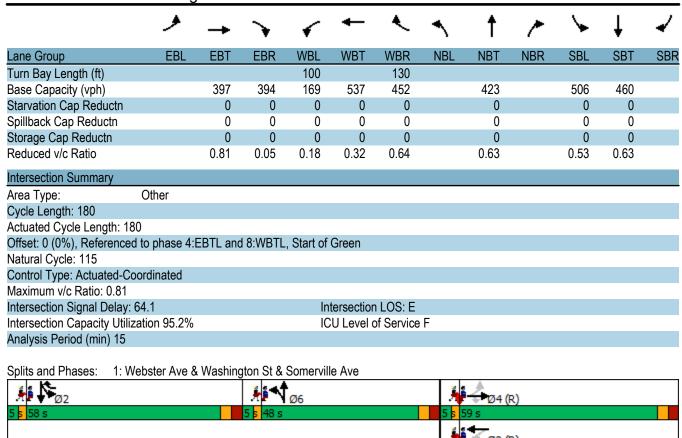


	-	•	•	•	•	~
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>^</b>					7
Traffic Volume (veh/h)	554	0	0	0	0	76
Future Volume (Veh/h)	554	0	0	0	0	76
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.84	0.84	0.92	0.92	0.82	0.82
Hourly flow rate (vph)	660	0	0	0	0	93
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None			None		
Median storage veh)						
Upstream signal (ft)				240		
pX, platoon unblocked						
vC, conflicting volume			660		660	330
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			660		660	330
tC, single (s)			4.1		6.8	6.9
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.3
p0 queue free %			100		100	86
cM capacity (veh/h)			924		396	669
Direction, Lane #	EB 1	EB 2	NB 1			
Volume Total	330	330	93			
Volume Left	0	0	0			
Volume Right	0	0	93			
cSH	1700	1700	669			
Volume to Capacity	0.19	0.19	0.14			
Queue Length 95th (ft)	0.10	0.10	12			
Control Delay (s)	0.0	0.0	11.3			
Lane LOS	0.0	0.0	В			
Approach Delay (s)	0.0		11.3			
Approach LOS	0.0		В			
Intersection Summary						
			1.4			
Average Delay Intersection Capacity Utiliza	otion			10	lll ovol s	f Consider
	สแบบ		26.7%	IC	U Level c	o Service
Analysis Period (min)			15			

	۶	<b>→</b>	•	•	+	•	•	<b>†</b>	~	<b>/</b>	<b>+</b>	-√
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4	7	*	<b>†</b>	7		4		ች	<b>1</b> >	
Traffic Volume (vph)	64	209	15	28	163	273	25	200	22	246	203	62
Future Volume (vph)	64	209	15	28	163	273	25	200	22	246	203	62
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	100		130	0		0	0		0
Storage Lanes	0		1	1		1	0		0	1		0
Taper Length (ft)	25			25			25			25		
Satd. Flow (prot)	0	1821	1615	1736	1827	1568	0	1804	0	1752	1594	0
Flt Permitted		0.771		0.336				0.995		0.950		
Satd. Flow (perm)	0	1349	1339	577	1827	1568	0	1804	0	1752	1594	0
Right Turn on Red			No			No			Yes			No
Satd. Flow (RTOR)								3				
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		580			678			611			159	
Travel Time (s)		13.2			15.4			13.9			3.6	
Confl. Peds. (#/hr)	100		48	48					83			195
Confl. Bikes (#/hr)									16			31
Peak Hour Factor	0.85	0.85	0.85	0.94	0.94	0.94	0.92	0.92	0.92	0.91	0.91	0.91
Heavy Vehicles (%)	0%	4%	0%	4%	4%	3%	0%	0%	5%	3%	0%	0%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	321	18	30	173	290	0	268	0	270	291	0
Turn Type	Perm	NA	Perm	Perm	NA	Over	Split	NA		Split	NA	
Protected Phases		4			8	2	6	6		2	2	
Permitted Phases	4		4	8								
Detector Phase	4	4	4	8	8	2	6	6		2	2	
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0	10.0	10.0	10.0		10.0	10.0	
Minimum Split (s)	34.0	34.0	34.0	34.0	34.0	33.0	31.0	31.0		33.0	33.0	
Total Split (s)	59.0	59.0	59.0	59.0	59.0	58.0	48.0	48.0		58.0	58.0	
Total Split (%)	32.8%	32.8%	32.8%	32.8%	32.8%	32.2%	26.7%	26.7%		32.2%	32.2%	
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0		3.0	3.0	
All-Red Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Lost Time Adjust (s)		0.0	0.0	0.0	0.0	0.0		0.0		0.0	0.0	
Total Lost Time (s)		6.0	6.0	6.0	6.0	6.0		6.0		6.0	6.0	
Lead/Lag	Lag	Lag	Lag	Lag	Lag	Lag	Lag	Lag		Lag	Lag	
Lead-Lag Optimize?												
Recall Mode	C-Max	C-Max	C-Max	C-Max	C-Max	Max	Max	Max		Max	Max	
Act Effct Green (s)		53.0	53.0	53.0	53.0	52.0		42.0		52.0	52.0	
Actuated g/C Ratio		0.29	0.29	0.29	0.29	0.29		0.23		0.29	0.29	
v/c Ratio		0.81	0.05	0.18	0.32	0.64		0.63		0.53	0.63	
Control Delay		75.5	46.1	50.9	51.6	63.5		69.1		58.4	63.0	
Queue Delay		0.0	0.0	0.0	0.0	0.0		0.0		0.0	0.0	
Total Delay		75.5	46.1	50.9	51.6	63.5		69.1		58.4	63.0	
LOS		Е	D	D	D	Е		Е		Е	Е	
Approach Delay		74.0			58.5			69.1			60.8	
Approach LOS		Е			Е			Е			Е	
Queue Length 50th (ft)		354	15	27	161	302		285		270	302	
Queue Length 95th (ft)		453	36	59	236	416		393		373	414	
Internal Link Dist (ft)		500			598			531			79	

03/10/2022 McMahon Associates Synchro 10 Report Page 1

Lane Group	Ø1	Ø3	Ø5	Ø7
Lane Configurations				
Traffic Volume (vph)				
Future Volume (vph)				
Ideal Flow (vphpl)				
\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \				
Storage Length (ft)				
Storage Lanes				
Taper Length (ft)				
Satd. Flow (prot)				
Flt Permitted				
Satd. Flow (perm)				
Right Turn on Red				
Satd. Flow (RTOR)				
Link Speed (mph)				
Link Distance (ft)				
Travel Time (s)				
Confl. Peds. (#/hr)				
Confl. Bikes (#/hr)				
Peak Hour Factor				
Heavy Vehicles (%)				
Shared Lane Traffic (%)				
Lane Group Flow (vph)				
Turn Type				
Protected Phases	1	3	5	7
Permitted Phases	<u>'</u>			•
Detector Phase				
Switch Phase				
Minimum Initial (s)	3.0	3.0	3.0	3.0
Minimum Split (s)	5.0	5.0	5.0	5.0
	5.0		5.0	5.0
Total Split (s)		5.0		
Total Split (%)	3%	3%	3%	3%
Yellow Time (s)	2.0	2.0	2.0	2.0
All-Red Time (s)	0.0	0.0	0.0	0.0
Lost Time Adjust (s)				
Total Lost Time (s)				
Lead/Lag	Lead	Lead	Lead	Lead
Lead-Lag Optimize?				
Recall Mode	Max	Max	Max	Max
Act Effct Green (s)				
Actuated g/C Ratio				
v/c Ratio				
Control Delay				
Queue Delay				
Total Delay				
LOS				
Approach Delay				
Annroach I OS				
Approach LOS				
Queue Length 50th (ft)				



Lane Group	Ø1	Ø3	Ø5	Ø7	
Turn Bay Length (ft)					
Base Capacity (vph)					
Starvation Cap Reductn					
Spillback Cap Reductn					
Storage Cap Reductn					
Reduced v/c Ratio					
Intersection Summary					

	۶	<b>→</b>	•	•	<b>←</b>	•	1	<b>†</b>	<i>&gt;</i>	<b>/</b>	<b></b>	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations					f)			4				7
Traffic Volume (veh/h)	0	0	0	0	505	32	200	42	0	0	0	100
Future Volume (Veh/h)	0	0	0	0	505	32	200	42	0	0	0	100
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.94	0.94	0.94	0.94	0.94	0.94	0.84	0.84	0.84
Hourly flow rate (vph)	0	0	0	0	537	34	213	45	0	0	0	119
Pedestrians											212	
Lane Width (ft)											12.0	
Walking Speed (ft/s)											3.5	
Percent Blockage											20	
Right turn flare (veh)												
Median type		None			None							
Median storage veh)												
Upstream signal (ft)					50							
pX, platoon unblocked	0.85						0.85	0.85		0.85	0.85	0.85
vC, conflicting volume	783			0			673	783	0	788	766	766
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	655			0			526	655	0	662	635	635
tC, single (s)	4.1			4.1			*5.5	*5.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												<u> </u>
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	100			100			21	86	100	100	100	64
cM capacity (veh/h)	631			1623			268	315	1085	192	268	326
Direction, Lane #	WB 1	NB 1	SB 1									
Volume Total	571	258	119									
Volume Left	0	213	0									
Volume Right	34	0	119									
cSH	1700	275	326									
Volume to Capacity	0.34	0.94	0.36									
Queue Length 95th (ft)	0	220	40									
Control Delay (s)	0.0	79.5	22.2									
Lane LOS		F	С									
Approach Delay (s)	0.0	79.5	22.2									
Approach LOS		F	С									
Intersection Summary												
Average Delay			24.4									
Intersection Capacity Utiliza	ition		58.5%	IC	U Level	of Service			В			
Analysis Period (min)			15									
* User Entered Value												

## Liberty Cannabis 3: Bow Street & Pedestrian Crossing

	•	•	<b>†</b>	<b>/</b>	<b>&gt;</b>	ţ	
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT	Ø2
Lane Configurations			<b></b>				
Traffic Volume (vph)	0	0	537	0	0	0	
Future Volume (vph)	0	0	537	0	0	0	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Satd. Flow (prot)	0	0	1863	0	0	0	
Flt Permitted							
Satd. Flow (perm)	0	0	1863	0	0	0	
Right Turn on Red		No		No	-	-	
Satd. Flow (RTOR)							
Link Speed (mph)	30		30			30	
Link Distance (ft)	184		50			50	
Travel Time (s)	4.2		1.1			1.1	
Peak Hour Factor	0.92	0.92	0.94	0.94	0.92	0.92	
Shared Lane Traffic (%)	0.32	0.32	0.34	0.34	0.32	0.32	
. ,	0	0	E71	٥	0	0	
Lane Group Flow (vph)	0	0	571	0	0	0	
Turn Type			NA				0
Protected Phases			1				2
Permitted Phases							
Detector Phase			1				
Switch Phase							
Minimum Initial (s)			25.0				10.0
Minimum Split (s)			29.0				16.0
Total Split (s)			29.0				16.0
Total Split (%)			64.4%				36%
Yellow Time (s)			3.0				2.0
All-Red Time (s)			1.0				1.0
Lost Time Adjust (s)			0.0				
Total Lost Time (s)			4.0				
Lead/Lag			Lead				Lag
Lead-Lag Optimize?							
Recall Mode			Min				None
Act Effct Green (s)			40.0				
Actuated g/C Ratio			0.67				
v/c Ratio			0.46				
Control Delay			6.3				
Queue Delay			0.0				
Total Delay			6.3				
LOS			Α				
Approach Delay			6.3				
Approach LOS			0.5 A				
Queue Length 50th (ft)			80				
Queue Length 95th (ft)			133				
Internal Link Dist (ft)	104		1			1	
	104		ı			ı	
Turn Bay Length (ft)			1010				
Base Capacity (vph)			1242				
Starvation Cap Reductn			0				
Spillback Cap Reductn			0				
Storage Cap Reductn			0				
Reduced v/c Ratio			0.46				

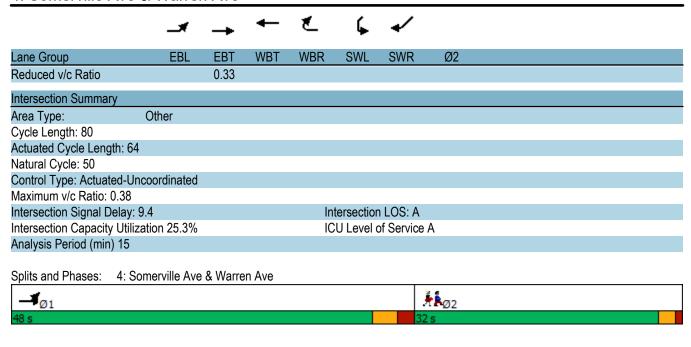
Synchro 10 Report 03/10/2022 Page 5 McMahon Associates

## Liberty Cannabis 3: Bow Street & Pedestrian Crossing

Intersection Summary Area Type: Other		
Cycle Length: 45		
Actuated Cycle Length: 60		
Natural Cycle: 45		
Control Type: Actuated-Uncoordinated		
Maximum v/c Ratio: 0.46		
Intersection Signal Delay: 6.3	Intersection LOS: A	
Intersection Capacity Utilization 31.6%	ICU Level of Service A	
Analysis Period (min) 15		
Splits and Phases: 3: Bow Street & Pedestrian Crossing		

**↑**ø1

	_#	-	<b>←</b>	€_	Ĺ	✓		
Lane Group	EBL	EBT	WBT	WBR	SWL	SWR	Ø2	
Lane Configurations		41					<del> </del>	
Traffic Volume (vph)	242	511	0	0	0	0		
Future Volume (vph)	242	511	0	0	0	0		
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900		
Satd. Flow (prot)	0	3528	0	0	0	0		
Flt Permitted		0.984						
Satd. Flow (perm)	0	3528	0	0	0	0		
Right Turn on Red	Yes	0020		Yes		Yes		
Satd. Flow (RTOR)	100	152		. 00		100		
Link Speed (mph)		30	30		30			
Link Distance (ft)		240	98		71			
Travel Time (s)		5.5	2.2		1.6			
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92		
Heavy Vehicles (%)	0%	1%	2%	2%	2%	2%		
Shared Lane Traffic (%)	- 0 /0	1 /0	270	£ /0	2 /0	2/0		
Lane Group Flow (vph)	0	818	0	0	0	0		
Turn Type	Split	NA	U	U	· ·	· ·		
Protected Phases	1	1					2	
Permitted Phases							2	
Detector Phase	1	1						
Switch Phase								
Minimum Initial (s)	10.0	10.0					5.0	
Minimum Split (s)	15.0	15.0					32.0	
Total Split (s)	48.0	48.0					32.0	
Total Split (%)	60.0%	60.0%					40%	
Yellow Time (s)	3.0	3.0					2.0	
All-Red Time (s)	2.0	2.0					1.0	
Lost Time Adjust (s)	2.0	0.0					1.0	
Total Lost Time (s)		5.0						
Lead/Lag	Lead	Lead					Lag	
Lead-Lag Optimize?	Load	LCau					Lag	
Recall Mode	Min	Min					None	
Act Effct Green (s)	IVIIII	37.6					NOTIC	
Actuated g/C Ratio		0.59						
v/c Ratio		0.38						
Control Delay		9.4						
Queue Delay		0.0						
Total Delay		9.4						
LOS		9.4 A						
Approach Delay		9.4						
Approach LOS		3. <del>4</del>						
Queue Length 50th (ft)		93						
Queue Length 95th (ft)		135						
Internal Link Dist (ft)		160	18		1			
Turn Bay Length (ft)		100	10					
Base Capacity (vph)		2445						
Starvation Cap Reductn		0						
Spillback Cap Reductn		0						
Storage Cap Reductn		0						
Ciorage Cap Neducin		U						



	<b>→</b>	•	•	<b>←</b>	4	<b>/</b>
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>†</b> †					7
Traffic Volume (veh/h)	709	0	0	0	0	46
Future Volume (Veh/h)	709	0	0	0	0	46
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.93	0.93	0.92	0.92	0.81	0.81
Hourly flow rate (vph)	762	0	0	0	0	57
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None			None		
Median storage veh)						
Upstream signal (ft)				240		
pX, platoon unblocked						
vC, conflicting volume			762		762	381
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			762		762	381
tC, single (s)			4.1		6.8	6.9
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.3
p0 queue free %			100		100	91
cM capacity (veh/h)			846		341	623
Direction, Lane #	EB 1	EB 2	NB 1			
Volume Total	381	381	57			
Volume Left	0	0	0			
Volume Right	0	0	57			
cSH	1700	1700	623			
Volume to Capacity	0.22	0.22	0.09			
Queue Length 95th (ft)	0	0	8			
Control Delay (s)	0.0	0.0	11.4			
Lane LOS			В			
Approach Delay (s)	0.0		11.4			
Approach LOS			В			
Intersection Summary						
Average Delay 0.8						
		29.6%	IC	U Level c	f Service	
Analysis Period (min)			15	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,		2200
analysis i shou (illiii)			10			

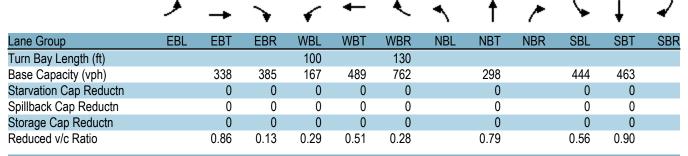
## **APPENDIX G**

2027 Design Year Build Capacity/Level-of-Service Analysis

	۶	<b>→</b>	*	•	<b>←</b>	•	1	†	~	<b>/</b>	<b>+</b>	-√
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4	7	*	<b>+</b>	7		4		, j	ĵ.	
Traffic Volume (vph)	53	205	45	46	231	196	8	145	28	213	303	54
Future Volume (vph)	53	205	45	46	231	196	8	145	28	213	303	54
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	100		130	0		0	0		0
Storage Lanes	0		1	1		1	0		0	1		0
Taper Length (ft)	25			25			25			25		
Satd. Flow (prot)	0	1755	1482	1583	1727	1495	0	1769	0	1703	1776	0
Flt Permitted		0.676		0.368				0.998		0.950		
Satd. Flow (perm)	0	1193	1360	592	1727	1401	0	1769	0	1703	1776	0
Right Turn on Red			No			No			Yes			No
Satd. Flow (RTOR)								4				
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		580			678			611			159	
Travel Time (s)		13.2			15.4			13.9			3.6	
Confl. Peds. (#/hr)	20		36	36		48			28			45
Confl. Bikes (#/hr)									2			28
Peak Hour Factor	0.89	0.89	0.89	0.93	0.93	0.93	0.77	0.77	0.77	0.86	0.86	0.86
Heavy Vehicles (%)	4%	8%	9%	14%	10%	8%	0%	4%	0%	6%	2%	4%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	290	51	49	248	211	0	234	0	248	415	0
Turn Type	Perm	NA	Perm	Perm	NA	custom	Split	NA		Split	NA	
Protected Phases		4			8		6	6		2	2	
Permitted Phases	4		4	8		28						
Detector Phase	4	4	4	8	8	28	6	6		2	2	
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0		10.0	10.0		10.0	10.0	
Minimum Split (s)	16.0	16.0	16.0	16.0	16.0		16.0	16.0		16.0	16.0	
Total Split (s)	57.0	57.0	57.0	57.0	57.0		36.0	36.0		53.0	53.0	
Total Split (%)	31.7%	31.7%	31.7%	31.7%	31.7%		20.0%	20.0%		29.4%	29.4%	
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0		3.0	3.0		3.0	3.0	
All-Red Time (s)	3.0	3.0	3.0	3.0	3.0		3.0	3.0		3.0	3.0	
Lost Time Adjust (s)		0.0	0.0	0.0	0.0			0.0		0.0	0.0	
Total Lost Time (s)		6.0	6.0	6.0	6.0			6.0		6.0	6.0	
Lead/Lag												
Lead-Lag Optimize?		0.14	0.11	0.14	0.14							
Recall Mode	C-Max		C-Max	C-Max	C-Max	00.0	Max	Max		Max	Max	
Act Effct Green (s)		51.0	51.0	51.0	51.0	98.0		30.0		47.0	47.0	
Actuated g/C Ratio		0.28	0.28	0.28	0.28	0.54		0.17		0.26	0.26	
v/c Ratio		0.86	0.13	0.29	0.51	0.28		0.79		0.56	0.90	
Control Delay		84.7	49.3	56.3	58.3	12.1		89.3		63.2	86.5	
Queue Delay		0.0	0.0	0.0	0.0	0.0		0.0		0.0	0.0	
Total Delay		84.7	49.3	56.3	58.3	12.1		89.3		63.2	86.5	
LOS		F 70.4	D	E	Е	В		F		E	F	
Approach Delay		79.4			38.9			89.3			77.8	
Approach LOS		E	4.5	40	D	00		F		050	E	
Queue Length 50th (ft)		327	45	46	247	62		265		256	478	
Queue Length 95th (ft)		#489	85	91	346	90		310		338	#621	
Internal Link Dist (ft)		500			598			531			79	

03/10/2022 McMahon Associates Synchro 10 Report Page 1

Lane Group	Ø9	
Lane Configurations		
Traffic Volume (vph)		
Future Volume (vph)		
Ideal Flow (vphpl)		
Storage Length (ft)		
Storage Lanes		
Taper Length (ft)		
Satd. Flow (prot)		
Flt Permitted		
Satd. Flow (perm)		
Right Turn on Red		
Satd. Flow (RTOR)		
Link Speed (mph)		
Link Distance (ft)		
Travel Time (s)		
Confl. Peds. (#/hr)		
Confl. Bikes (#/hr) Peak Hour Factor		
Heavy Vehicles (%)		
Shared Lane Traffic (%)		
Lane Group Flow (vph)		
Turn Type	0	
Protected Phases	9	
Permitted Phases		
Detector Phase		
Switch Phase	0.0	
Minimum Initial (s)	9.0	
Minimum Split (s)	34.0	
Total Split (s)	34.0	
Total Split (%)	19%	
Yellow Time (s)	2.0	
All-Red Time (s)	3.0	
Lost Time Adjust (s)		
Total Lost Time (s)		
Lead/Lag		
Lead-Lag Optimize?		
Recall Mode	Max	
Act Effct Green (s)		
Actuated g/C Ratio		
v/c Ratio		
Control Delay		
Queue Delay		
Total Delay		
LOS		
Approach Delay		
Approach LOS		
Queue Length 50th (ft)		
Queue Length 95th (ft)		
Internal Link Dist (ft)		



#### Intersection Summary

Area Type: Other

Cycle Length: 180 Actuated Cycle Length: 180

Offset: 0 (0%), Referenced to phase 4:EBTL and 8:WBTL, Start of Green

Natural Cycle: 115

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.90 Intersection Signal Delay: 68.4

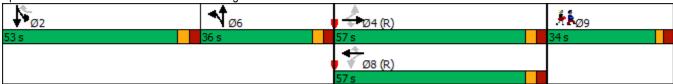
Intersection Signal Delay: 68.4 Intersection LOS: E
Intersection Capacity Utilization 75.8% ICU Level of Service D

Analysis Period (min) 15

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 1: Webster Ave & Washington St & Somerville Ave



Lane Group	Ø9		
Turn Bay Length (ft)			
Base Capacity (vph)			
Starvation Cap Reductn			
Spillback Cap Reductn			
Storage Cap Reductn			
Reduced v/c Ratio			
Intersection Summary			

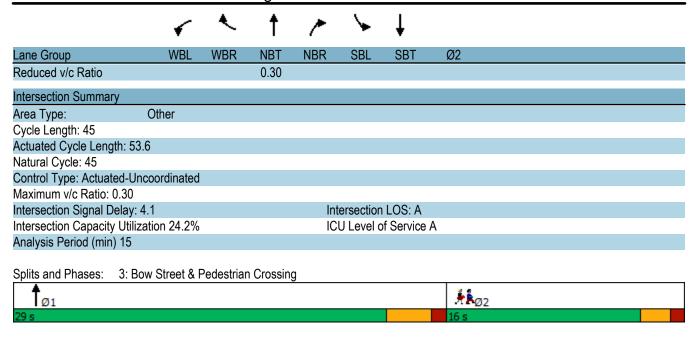
	۶	<b>→</b>	•	•	<b>←</b>	•	4	<b>†</b>	<i>&gt;</i>	<b>&gt;</b>	ļ	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations					<b>₽</b>			4				7
Traffic Volume (veh/h)	0	0	0	0	381	13	164	48	0	0	0	67
Future Volume (Veh/h)	0	0	0	0	381	13	164	48	0	0	0	67
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.76	0.76	0.76
Hourly flow rate (vph)	0	0	0	0	414	14	178	52	0	0	0	88
Pedestrians											54	
Lane Width (ft)											12.0	
Walking Speed (ft/s)											3.5	
Percent Blockage											5	
Right turn flare (veh)												
Median type		None			None							
Median storage veh)												
Upstream signal (ft)					50							
pX, platoon unblocked	0.92						0.92	0.92		0.92	0.92	0.92
vC, conflicting volume	482			0			509	482	0	501	475	475
vC1, stage 1 conf vol	102						000	102		001	170	170
vC2, stage 2 conf vol												
vCu, unblocked vol	393			0			422	393	0	413	385	385
tC, single (s)	4.1			4.1			*5.5	*5.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)	7.1			7.1			0.0	0.0	0.2	7.1	0.0	0.2
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	100			100			64	90	100	100	100	85
cM capacity (veh/h)	1017			1623			492	532	1085	426	479	582
Direction, Lane #	WB 1	NB 1	SB 1									
Volume Total	428	230	88									
Volume Left	0	178	0									
Volume Right	14	0	88									
cSH	1700	501	582									
Volume to Capacity	0.25	0.46	0.15									
Queue Length 95th (ft)	0.23	60	13									
Control Delay (s)	0.0	18.2	12.3									
Lane LOS	0.0	C	12.3 B									
Approach Delay (s)	0.0	18.2	12.3									
Approach LOS	0.0	C	12.3 B									
Intersection Summary												
Average Delay			7.0									
Intersection Capacity Utilizat	tion		46.7%	IC	יון אים נ	of Service			Α			
Analysis Period (min)	uon		15	10	O LEVEL	DI GELVICE			A			
Alialysis Feliou (IIIIII)			10									
* User Entered Value												

# Liberty Cannabis 3: Bow Street & Pedestrian Crossing

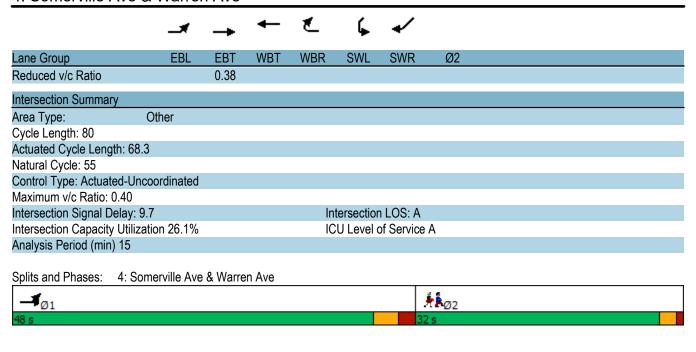
	•	•	<b>†</b>	<b>/</b>	<b>\</b>	ļ		
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT	Ø2	
Lane Configurations	.,,		<u> </u>	.,,,,,				
Traffic Volume (vph)	0	0	394	0	0	0		
Future Volume (vph)	0	0	394	0	0	0		
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900		
Satd. Flow (prot)	0	0	1792	0	0	0		
Flt Permitted			1702					
Satd. Flow (perm)	0	0	1792	0	0	0		
Right Turn on Red		No	1702	No				
Satd. Flow (RTOR)		110		110				
Link Speed (mph)	30		30			30		
Link Distance (ft)	184		50			50		
Travel Time (s)	4.2		1.1			1.1		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92		
Heavy Vehicles (%)	2%	2%	6%	2%	2%	2%		
Shared Lane Traffic (%)			3,0			/0		
Lane Group Flow (vph)	0	0	428	0	0	0		
Turn Type			NA					
Protected Phases			1				2	
Permitted Phases			•				<del>-</del>	
Detector Phase			1					
Switch Phase			•					
Minimum Initial (s)			25.0				10.0	
Minimum Split (s)			29.0				16.0	
Total Split (s)			29.0				16.0	
Total Split (%)			64.4%				36%	
Yellow Time (s)			3.0				2.0	
All-Red Time (s)			1.0				1.0	
Lost Time Adjust (s)			0.0					
Total Lost Time (s)			4.0					
Lead/Lag			Lead				Lag	
Lead-Lag Optimize?								
Recall Mode			Min				None	
Act Effct Green (s)			42.9					
Actuated g/C Ratio			0.80					
v/c Ratio			0.30					
Control Delay			4.1					
Queue Delay			0.0					
Total Delay			4.1					
LOS			Α					
Approach Delay			4.1					
Approach LOS			Α					
Queue Length 50th (ft)			55					
Queue Length 95th (ft)			93					
Internal Link Dist (ft)	104		1			1		
Turn Bay Length (ft)								
Base Capacity (vph)			1433					
Starvation Cap Reductn			0					
Spillback Cap Reductn			0					
Storage Cap Reductn			0					

Synchro 10 Report Page 5 03/10/2022 McMahon Associates

### 3: Bow Street & Pedestrian Crossing



	_#	<b>→</b>	<b>+</b>	٤	4	<b>√</b>	
Lane Group	EBL	EBT	WBT	WBR	SWL	SWR	Ø2
Lane Configurations		414			0	•	~-
Traffic Volume (vph)	212	570	0	0	0	0	
Future Volume (vph)	212	570	0	0	0	0	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Satd. Flow (prot)	0	3453	0	0	0	0	
Flt Permitted		0.987	•		•		
Satd. Flow (perm)	0	3453	0	0	0	0	
Right Turn on Red	Yes	0.00	•	Yes	•	Yes	
Satd. Flow (RTOR)		104					
Link Speed (mph)		30	30		30		
Link Distance (ft)		240	98		71		
Travel Time (s)		5.5	2.2		1.6		
Peak Hour Factor	0.90	0.90	0.92	0.92	0.92	0.92	
Heavy Vehicles (%)	1%	4%	2%	2%	2%	2%	
Shared Lane Traffic (%)	170	170	270	270	270	270	
Lane Group Flow (vph)	0	869	0	0	0	0	
Turn Type	Split	NA	· ·	U	· ·	U	
Protected Phases	1	1					2
Permitted Phases	'	•					<b>L</b>
Detector Phase	1	1					
Switch Phase	'	•					
Minimum Initial (s)	10.0	10.0					5.0
Minimum Split (s)	15.0	15.0					32.0
Total Split (s)	48.0	48.0					32.0
Total Split (%)	60.0%	60.0%					40%
Yellow Time (s)	3.0	3.0					2.0
All-Red Time (s)	2.0	2.0					1.0
Lost Time Adjust (s)	2.0	0.0					1.0
Total Lost Time (s)		5.0					
Lead/Lag	Lead	Lead					Lag
Lead-Lag Optimize?	Loud	Loud					
Recall Mode	Min	Min					None
Act Effct Green (s)	141111	42.2					Hono
Actuated g/C Ratio		0.62					
v/c Ratio		0.40					
Control Delay		9.7					
Queue Delay		0.0					
Total Delay		9.7					
LOS		A					
Approach Delay		9.7					
Approach LOS		A					
Queue Length 50th (ft)		112					
Queue Length 95th (ft)		154					
Internal Link Dist (ft)		160	18		1		
Turn Bay Length (ft)		100	10				
Base Capacity (vph)		2296					
Starvation Cap Reductn		0					
Spillback Cap Reductn		0					
Storage Cap Reductin		0					
Ciorage Cap Neudolli		U					



	-	•	•	•	•	~	
Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	<b>†</b> †					7	
Traffic Volume (veh/h)	710	0	0	0	0	73	
Future Volume (Veh/h)	710	0	0	0	0	73	
Sign Control	Free			Free	Stop		
Grade	0%			0%	0%		
Peak Hour Factor	0.93	0.93	0.92	0.92	0.73	0.73	
Hourly flow rate (vph)	763	0	0	0	0	100	
Pedestrians							
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type	None			None			
Median storage veh)							
Upstream signal (ft)				240			
pX, platoon unblocked							
vC, conflicting volume			763		763	382	
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol			763		763	382	
tC, single (s)			4.1		6.8	7.0	
tC, 2 stage (s)							
tF (s)			2.2		3.5	3.3	
p0 queue free %			100		100	84	
cM capacity (veh/h)			845		341	614	
	ED 4	ED 0					
Direction, Lane # Volume Total	EB 1 382	EB 2	NB 1				
		382	100				
Volume Left	0	0	100				
Volume Right	0	0	100				
cSH	1700	1700	614				
Volume to Capacity	0.22	0.22	0.16				
Queue Length 95th (ft)	0	0	14				
Control Delay (s)	0.0	0.0	12.0				
Lane LOS			В				
Approach Delay (s)	0.0		12.0				
Approach LOS			В				
Intersection Summary							
Average Delay			1.4				
Intersection Capacity Utiliz	ation		30.8%	IC	U Level c	of Service	)
Analysis Period (min)			15				
J = = = = = = = = = = = = = = = = = = =							

### Liberty Cannabis 1: Webster Ave & Washington St & Somerville Ave

	۶	<b>→</b>	*	•	<b>←</b>	•	1	†	<i>&gt;</i>	<b>/</b>	<b>+</b>	
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ર્ન	7	ሻ	<b></b>	7		4		ሻ	<b>f</b> a	
Traffic Volume (vph)	79	223	35	26	220	249	23	304	13	169	205	23
Future Volume (vph)	79	223	35	26	220	249	23	304	13	169	205	23
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	100		130	0		0	0		0
Storage Lanes	0		1	1		1	0		0	1		0
Taper Length (ft)	25			25			25			25		
Satd. Flow (prot)	0	1803	1615	1612	1863	1568	0	1854	0	1770	1776	0
Flt Permitted		0.677		0.370				0.997		0.950		
Satd. Flow (perm)	0	1224	1497	610	1863	1378	0	1854	0	1770	1776	0
Right Turn on Red			No			No			Yes			No
Satd. Flow (RTOR)								1				
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		580			678			611			159	
Travel Time (s)		13.2			15.4			13.9			3.6	
Confl. Peds. (#/hr)	41		35	35		101			60			74
Confl. Bikes (#/hr)									26			8
Peak Hour Factor	0.95	0.95	0.95	0.94	0.94	0.94	0.94	0.94	0.94	0.96	0.96	0.96
Heavy Vehicles (%)	4%	4%	0%	12%	2%	3%	0%	1%	0%	2%	3%	0%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	318	37	28	234	265	0	361	0	176	238	0
Turn Type	Perm	NA	Perm	Perm	NA	custom	Split	NA		Split	NA	
Protected Phases		4			8		6	6		2	2	
Permitted Phases	4		4	8		28						
Detector Phase	4	4	4	8	8	28	6	6		2	2	
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0		10.0	10.0		10.0	10.0	
Minimum Split (s)	16.0	16.0	16.0	16.0	16.0		16.0	16.0		16.0	16.0	
Total Split (s)	64.0	64.0	64.0	64.0	64.0		46.0	46.0		36.0	36.0	
Total Split (%)	35.6%	35.6%	35.6%	35.6%	35.6%		25.6%	25.6%		20.0%	20.0%	
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0		3.0	3.0		3.0	3.0	
All-Red Time (s)	3.0	3.0	3.0	3.0	3.0		3.0	3.0		3.0	3.0	
Lost Time Adjust (s)		0.0	0.0	0.0	0.0			0.0		0.0	0.0	
Total Lost Time (s)		6.0	6.0	6.0	6.0			6.0		6.0	6.0	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	C-Max		C-Max	C-Max	C-Max		Max	Max		Max	Max	
Act Effct Green (s)		58.0	58.0	58.0	58.0	88.0		40.0		30.0	30.0	
Actuated g/C Ratio		0.32	0.32	0.32	0.32	0.49		0.22		0.17	0.17	
v/c Ratio		0.81	0.08	0.14	0.39	0.39		0.88		0.60	0.80	
Control Delay		72.7	43.1	46.0	49.7	16.8		89.3		78.7	92.5	
Queue Delay		0.0	0.0	0.0	0.0	0.0		0.0		0.0	0.0	
Total Delay		72.7	43.1	46.0	49.7	16.8		89.3		78.7	92.5	
LOS		Е	D	D	D	В		F		Е	F	
Approach Delay		69.6			32.9			89.3			86.6	
Approach LOS		Е			С			F			F	
Queue Length 50th (ft)		346	31	24	216	105		416		195	275	
Queue Length 95th (ft)		#506	63	54	302	148		#598		287	#411	
Internal Link Dist (ft)		500			598			531			79	

03/10/2022 McMahon Associates Synchro 10 Report Page 1

Lane Group	Ø9	
Lane Configurations		
Traffic Volume (vph)		
Future Volume (vph)		
Ideal Flow (vphpl)		
Storage Length (ft)		
Storage Lanes		
Taper Length (ft)		
Satd. Flow (prot)		
Flt Permitted		
Satd. Flow (perm)		
Right Turn on Red		
Satd. Flow (RTOR)		
Link Speed (mph)		
Link Distance (ft)		
Travel Time (s)		
Confl. Peds. (#/hr)		
Confl. Bikes (#/hr)		
Peak Hour Factor		
Heavy Vehicles (%)		
Shared Lane Traffic (%)		
Lane Group Flow (vph)		
Turn Type		
Protected Phases	9	
Permitted Phases	<u> </u>	
Detector Phase		
Switch Phase		
Minimum Initial (s)	9.0	
Minimum Split (s)	34.0	
Total Split (s)	34.0	
Total Split (%)	19%	
Yellow Time (s)	2.0	
All-Red Time (s)	3.0	
Lost Time Adjust (s)	0.0	
Total Lost Time (s)		
Lead/Lag		
Lead-Lag Optimize?		
Recall Mode	Max	
Act Effct Green (s)	Widi	
Actuated g/C Ratio		
v/c Ratio		
Control Delay		
Queue Delay		
Total Delay		
LOS		
Approach Delay		
Approach LOS		
Queue Length 50th (ft)		
Queue Length 95th (ft)		
Internal Link Dist (ft)		
(/		

Lane Group

Turn Bay Length (ft)

Base Capacity (vph)

Starvation Cap Reductn

Spillback Cap Reductn

Storage Cap Reductn

Reduced v/c Ratio

#### 1: Webster Ave & Washington St & Somerville Ave

**EBL** 

**EBR** 

482

0

0

0.08

**WBL** 

100

196

0

0

0

0.14

**WBT** 

600

0

0

0

0.39

WBR

130

673

0

0

0

0.39

**EBT** 

394

0

0

0

0.81

^	<b>†</b>	/	<b>&gt;</b>	ţ	1
NBL	NBT	NBR	SBL	SBT	SBR
	412		295	296	
	0		0	0	
	Λ		Λ	Λ	

0.60

0

0.80

0

0.88

#### Intersection Summary

Area Type: Other

Cycle Length: 180 Actuated Cycle Length: 180

Offset: 0 (0%), Referenced to phase 4:EBTL and 8:WBTL, Start of Green

Natural Cycle: 115

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.88 Intersection Signal Delay: 66.5

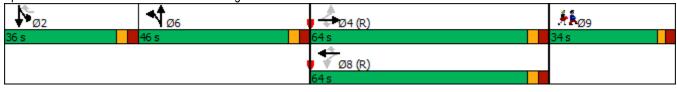
Intersection LOS: E Intersection Capacity Utilization 78.6% ICU Level of Service D

Analysis Period (min) 15

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 1: Webster Ave & Washington St & Somerville Ave



Lane Group	Ø9			
Turn Bay Length (ft)				
Base Capacity (vph)				
Starvation Cap Reductn				
Spillback Cap Reductn				
Storage Cap Reductn				
Reduced v/c Ratio				
Intersection Summary				

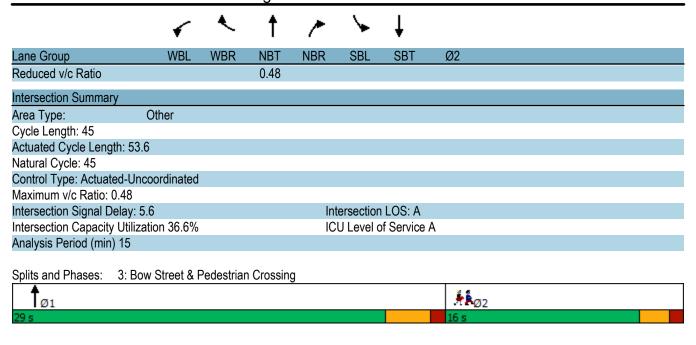
Lane Configurations		•	<b>→</b>	•	•	<b>←</b>	•	•	<b>†</b>	<i>&gt;</i>	<b>&gt;</b>	ļ	1
Traffic Volume (veh/h) 0 0 0 0 610 22 190 51 0 0 0 60 60 60 60 60 60 60 60 60 60 60	Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Future Volume (Vehi/h) 0 0 0 0 610 22 190 51 0 0 0 60 Sign Control Free Free Stop Stop Stop Stop Own	Lane Configurations					î»			4				7
Sign Control   Free	Traffic Volume (veh/h)	0	0	0	0		22	190	51	0	0	0	
Grade 0,% 0,% 0,% 0,% 0,% 0,% 0,% 0,% 0,% 0,%	Future Volume (Veh/h)	0	0	0	0	610	22	190	51	0	0	0	60
Peak Hour Factor 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92	Sign Control		Free			Free			Stop			Stop	
Hourly flow rate (vph) 0 0 0 0 663 24 257 69 0 0 0 80 Pedestrians 77 Lane Width (ft) 12.0 Malking Speed (ft/s) 3.5 Percent Blockage 77 Right turn flare (veh) Median type None None Median storage veh) Upstream signal (ft) 50 PMS, platoon unblocked 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84	Grade		0%			0%			0%			0%	
Pedestrians	Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.74	0.74	0.74	0.75	0.75	0.75
Pedestrians	Hourly flow rate (vph)	0	0	0	0	663	24	257	69	0	0	0	80
Walking Speed (fit/s) Percent Blockage Right turn flare (veh) Median tyre Median tyre None Median storage veh) Upstream signal (fit) pX, platoon unblocked 0,84 0,04 0,84 0,84 0,84 0,84 0,84 0,84	Pedestrians											77	
Walking Speed (ft/s) Percent Blockage Right turn flare (veh) Median type None None Median storage veh) Upstream signal (ft) pX, platoon unblocked VC, conflicting volume VC1, stage 1 conf vol VC2, stage 2 conf vol VC2, stage 2 conf vol VC2, stage 1 conf vol VC3, stage 1 conf vol VC4, stage 1 conf vol VC5, stage 2 conf vol VC9, stage 3 conf vol VC9, stage 3 conf vol VC9, stage 4 conf vol VC9, stage 5 conf vol VC9, stage 6 conf vol VC9, stage 8 conf vol VC9, stage 8 conf vol VC9, stage 8 conf vol VC9, stage 9 conf vol VC9, stage 9 conf vol VC9, stage 1 conf vol VC9, stage 2 conf vol VC9, stage 2 conf vol VC9, stage 2 conf vol VC9, stage 1 conf vol VC9, stage 2 conf vol VC9, stage 2 conf vol VC9, stage 1 conf vol VC9, stage 2 conf vol VC9, stage 2 conf vol VC9, stage 1 conf vol VC9, stage 2 conf vol VC9, stage 1 conf vol VC1, stage 1 conf vol VC2, stage 2 conf vol VC2, stage 1 conf vol VC1, stage 1 conf vol VC1, stage 1 conf vol VC2, stage 1 conf vol VC1, stage 1 conf vol VC1, stage 1 conf vol VC2, stage 1 conf vol VC2, stage 1 conf vol VC1, stage 1 conf vol VC2, stage 1 conf vol VC1, stage 1 conf vol VC2, stage 1 conf vol VC2, stage 1 conf vol VC1, st	Lane Width (ft)											12.0	
Percent Blockage   Right turn flare (veh)   None   None   None   Median storage veh   Upstream signal (ft)   50   50   755   764   0   786   752   752   752   752   752   752   754   755   764   0   786   752   752   752   752   752   752   752   753   764   755   764   755   764   755   755   764   755	. ,											3.5	
Right turn flare (veh) Median storage veh) Upstream signal (ft) pX, platoon unblocked 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84													
Median type         None         None           Median storage veh)         Upstream signal (ft)         50           pX, platoon unblocked         0.84													
Median storage veh)     Upstream signal (ft)     50       Dx, platoon unblocked     0.84     0.81     0.81     0.81     0.81     0.81     0.81     0.81     0.81 </td <td></td> <td></td> <td>None</td> <td></td> <td></td> <td>None</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td>			None			None							
Upstream signal (ft)													
pX, platoon unblocked 0.84 0.84 0.84 0.84 0.84 0.84 0.84 vC, conflicting volume 764 0 765 764 0 786 752 752 752 vC1, stage 1 conf vol vC2, stage 2 conf vol vC2, stage 2 conf vol vC2, stage 2 conf vol vC2, stage 3 d. 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1						50							
vC, conflicting volume 764 0 755 764 0 786 752 752 vC1, stage 1 conf vol vC2, stage 2 conf vol vC2, stage 2 conf vol vC2, stage 2 conf vol vC2, unblocked vol 619 0 609 619 0 646 605 605 CC, single (s) 4.1 4.1 *5.5 *5.5 6.2 7.1 6.5 6.3 tC, 2 stage (s) tF (s) 2.2 2.2 3.5 4.0 3.3 3.5 4.0 3.4 p0 queue free % 100 100 23 82 100 100 100 79 cM capacity (veh/h) 744 1623 335 374 1085 242 319 378 Direction, Lane # WB 1 NB 1 SB 1 Volume Total 687 326 80 Volume Total 687 326 80 Volume Right 24 0 80 cSH 1700 342 378 Volume to Capacity 0.40 0.95 0.21 Queue Length 95th (ft) 0 252 20 Control Delay (s) 0.0 72.8 17.0 Lane LOS F C Approach Delay (s) 0.0 72.8 17.0 Approach LOS F C Intersection Summary  Average Delay 23.0 Intersection Capacity Utilization 60.5% ICU Level of Service B Analysis Period (min) 15		0.84				00		0.84	0.84		0.84	0.84	0.84
vC1, stage 1 conf vol vC2, stage 2 conf vol vCu, unblocked vol 619 0 609 619 0 646 605 605 tC, single (s) 4.1 4.1 *5.5 *5.5 6.2 7.1 6.5 6.3 tC, 2 stage (s) tF (s) 2.2 2.2 3.5 4.0 3.3 3.5 4.0 3.4 p0 queue free % 100 100 23 82 100 100 100 79 cM capacity (veh/h) 744 1623 335 374 1085 242 319 378  Direction, Lane # WB 1 NB 1 SB 1  Volume Total 687 326 80 Volume Left 0 257 0 Volume Right 24 0 80 cSH 1700 342 378  Volume to Capacity 0.40 0.95 0.21 Queue Length 95th (ft) 0 252 20 Control Delay (s) 0.0 72.8 17.0 Lane LOS F C Approach Delay (s) 0.0 72.8 17.0 Approach Delay (s) 0.0 72.8 17.0 Approach LOS F C  Intersection Summary  Average Delay 23.0 Intersection Capacity Utilization 60.5% ICU Level of Service B  Analysis Period (min) 15					0					0			
VCQ, stage 2 conf vol VCQ, unblocked vol 619 0 609 619 0 646 605 605 tC, single (s) 4.1 4.1 *5.5 *5.5 6.2 7.1 6.5 6.3 tC, 2 stage (s)  tF (s) 2.2 2.2 3.5 4.0 3.3 3.5 4.0 3.4 p0 queue free % 100 100 23 82 100 100 100 79 cM capacity (veh/h) 744 1623 335 374 1085 242 319 378  Direction, Lane # WB 1 NB 1 SB 1  Volume Total 687 326 80 Volume Left 0 257 0 Volume Right 24 0 80 cSH 1700 342 378  Volume to Capacity 0.40 0.95 0.21 Queue Length 95th (ft) 0 252 20 Control Delay (s) 0.0 72.8 17.0 Lane LOS F C Approach Delay (s) 0.0 72.8 17.0 Approach LOS F C  Intersection Summary  Average Delay 23.0 Intersection Capacity Utilization 60.5% ICU Level of Service B Analysis Period (min) 15		701						700	701		700	702	102
vCu, unblocked vol 619 0 609 619 0 646 605 605 tC, single (s) 4.1 4.1 *5.5 *5.5 *5.5 6.2 7.1 6.5 6.3 tC, 2 stage (s)  tF (s) 2.2 2.2 3.5 4.0 3.3 3.5 4.0 3.4 p0 queue free % 100 100 23 82 100 100 100 79 dc Ac													
tC, single (s) 4.1 4.1 *5.5 *5.5 6.2 7.1 6.5 6.3 tC, 2 stage (s) tC, 2 stage (s) tF (s) 2.2 2.2 3.5 4.0 3.3 3.5 4.0 3.4 p0 queue free % 100 100 23 82 100 100 100 79 cM capacity (veh/h) 744 1623 335 374 1085 242 319 378 Direction, Lane # WB 1 NB 1 SB 1  Volume Total 687 326 80 Volume Right 24 0 80 cSH 1700 342 378 Volume to Capacity 0.40 0.95 0.21 Queue Length 95th (ft) 0 252 20 Control Delay (s) 0.0 72.8 17.0 Lane LOS F C Approach Delay (s) 0.0 72.8 17.0 Approach LOS F C Intersection Summary  Average Delay 23.0 ICU Level of Service B Analysis Period (min) 15		619			Λ			609	619	0	646	605	605
tC, 2 stage (s) tF (s) 2.2 2.2 3.5 4.0 3.3 3.5 4.0 3.4 p0 queue free % 100 100 23 82 100 100 100 79 cM capacity (veh/h) 744 1623 335 374 1085 242 319 378  Direction, Lane # WB 1 NB 1 SB 1  Volume Total 687 326 80  Volume Left 0 257 0  Volume Right 24 0 80 cSH 1700 342 378  Volume to Capacity 0.40 0.95 0.21  Queue Length 95th (ft) 0 252 20  Control Delay (s) 0.0 72.8 17.0  Lane LOS F C  Approach Delay (s) 0.0 72.8 17.0  Approach LOS F C  Intersection Summary  Average Delay  Intersection Capacity Utilization 60.5% ICU Level of Service B  Analysis Period (min) 15													
tF (s) 2.2 2.2 3.5 4.0 3.3 3.5 4.0 3.4 p0 queue free % 100 100 100 23 82 100 100 100 79 cM capacity (veh/h) 744 1623 335 374 1085 242 319 378		7.1			7.1			0.0	0.0	0.2	7.1	0.0	0.0
p0 queue free % 100 100 23 82 100 100 100 79 cM capacity (veh/h) 744 1623 335 374 1085 242 319 378      Direction, Lane # WB 1 NB 1 SB 1		2.2			2.2			3.5	4.0	3 3	3.5	4.0	3./
CM capacity (veh/h)   744   1623   335   374   1085   242   319   378													
Direction, Lane # WB 1 NB 1 SB 1													
Volume Total         687         326         80           Volume Left         0         257         0           Volume Right         24         0         80           cSH         1700         342         378           Volume to Capacity         0.40         0.95         0.21           Queue Length 95th (ft)         0         252         20           Control Delay (s)         0.0         72.8         17.0           Lane LOS         F         C           Approach Delay (s)         0.0         72.8         17.0           Approach LOS         F         C           Intersection Summary         Volume to Capacity Utilization         60.5%         ICU Level of Service         B           Analysis Period (min)         15			NR 1	SR 1									
Volume Left         0         257         0           Volume Right         24         0         80           cSH         1700         342         378           Volume to Capacity         0.40         0.95         0.21           Queue Length 95th (ft)         0         252         20           Control Delay (s)         0.0         72.8         17.0           Lane LOS         F         C           Approach Delay (s)         0.0         72.8         17.0           Approach LOS         F         C           Intersection Summary         Average Delay         23.0           Intersection Capacity Utilization         60.5%         ICU Level of Service         B           Analysis Period (min)         15													
Volume Right         24         0         80           cSH         1700         342         378           Volume to Capacity         0.40         0.95         0.21           Queue Length 95th (ft)         0         252         20           Control Delay (s)         0.0         72.8         17.0           Lane LOS         F         C           Approach Delay (s)         0.0         72.8         17.0           Approach LOS         F         C           Intersection Summary         23.0         Intersection Capacity Utilization         60.5%         ICU Level of Service         B           Analysis Period (min)         15         ICU Level of Service         B													
cSH         1700         342         378           Volume to Capacity         0.40         0.95         0.21           Queue Length 95th (ft)         0         252         20           Control Delay (s)         0.0         72.8         17.0           Lane LOS         F         C           Approach Delay (s)         0.0         72.8         17.0           Approach LOS         F         C           Intersection Summary         Summary         23.0           Average Delay         23.0         ICU Level of Service         B           Analysis Period (min)         15													
Volume to Capacity         0.40         0.95         0.21           Queue Length 95th (ft)         0         252         20           Control Delay (s)         0.0         72.8         17.0           Lane LOS         F         C           Approach Delay (s)         0.0         72.8         17.0           Approach LOS         F         C           Intersection Summary         Value of Service         B           Average Delay         23.0         ICU Level of Service         B           Analysis Period (min)         15													
Queue Length 95th (ft)         0         252         20           Control Delay (s)         0.0         72.8         17.0           Lane LOS         F         C           Approach Delay (s)         0.0         72.8         17.0           Approach LOS         F         C           Intersection Summary         Value of Service         B           Average Delay         23.0         ICU Level of Service         B           Analysis Period (min)         15													
Control Delay (s)         0.0         72.8         17.0           Lane LOS         F         C           Approach Delay (s)         0.0         72.8         17.0           Approach LOS         F         C           Intersection Summary         23.0           Average Delay         23.0           Intersection Capacity Utilization         60.5%         ICU Level of Service         B           Analysis Period (min)         15													
Lane LOS F C Approach Delay (s) 0.0 72.8 17.0 Approach LOS F C  Intersection Summary  Average Delay 23.0 Intersection Capacity Utilization 60.5% ICU Level of Service B  Analysis Period (min) 15													
Approach Delay (s) 0.0 72.8 17.0 Approach LOS F C  Intersection Summary  Average Delay 23.0 Intersection Capacity Utilization 60.5% ICU Level of Service B  Analysis Period (min) 15		0.0											
Approach LOS F C  Intersection Summary  Average Delay 23.0 Intersection Capacity Utilization 60.5% ICU Level of Service B  Analysis Period (min) 15		0.0											
Intersection Summary  Average Delay 23.0 Intersection Capacity Utilization 60.5% ICU Level of Service B  Analysis Period (min) 15		0.0											
Average Delay 23.0 Intersection Capacity Utilization 60.5% ICU Level of Service B Analysis Period (min) 15				U									
Intersection Capacity Utilization 60.5% ICU Level of Service B Analysis Period (min) 15													
Analysis Period (min) 15													
		ation			IC	CU Level of	of Service			В			
* User Entered Value	Analysis Period (min)			15									
	* User Entered Value												

## Liberty Cannabis 3: Bow Street & Pedestrian Crossing

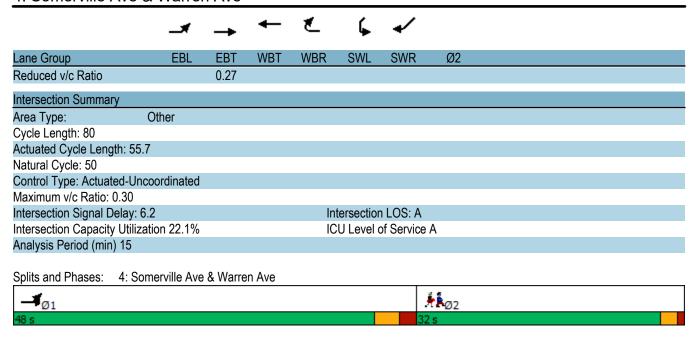
	•	•	<b>†</b>	<b>/</b>	<b>\</b>	ļ		
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT	Ø2	
Lane Configurations			<b>1</b>					
Traffic Volume (vph)	0	0	632	0	0	0		
Future Volume (vph)	0	0	632	0	0	0		
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900		
Satd. Flow (prot)	0	0	1792	0	0	0		
Flt Permitted								
Satd. Flow (perm)	0	0	1792	0	0	0		
Right Turn on Red		No		No				
Satd. Flow (RTOR)								
Link Speed (mph)	30		30			30		
Link Distance (ft)	184		50			50		
Travel Time (s)	4.2		1.1			1.1		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92		
Heavy Vehicles (%)	2%	2%	6%	2%	2%	2%		
Shared Lane Traffic (%)			3,0					
Lane Group Flow (vph)	0	0	687	0	0	0		
Turn Type			NA					
Protected Phases			1				2	
Permitted Phases								
Detector Phase			1					
Switch Phase								
Minimum Initial (s)			25.0				10.0	
Minimum Split (s)			29.0				16.0	
Total Split (s)			29.0				16.0	
Total Split (%)			64.4%				36%	
Yellow Time (s)			3.0				2.0	
All-Red Time (s)			1.0				1.0	
Lost Time Adjust (s)			0.0					
Total Lost Time (s)			4.0					
Lead/Lag			Lead				Lag	
Lead-Lag Optimize?								
Recall Mode			Min				None	
Act Effct Green (s)			42.9					
Actuated g/C Ratio			0.80					
v/c Ratio			0.48					
Control Delay			5.6					
Queue Delay			0.0					
Total Delay			5.6					
LOS			A					
Approach Delay			5.6					
Approach LOS			A					
Queue Length 50th (ft)			109					
Queue Length 95th (ft)	101		184			1		
Internal Link Dist (ft)	104		1			1		
Turn Bay Length (ft)			1/22					
Base Capacity (vph) Starvation Cap Reductn			1433 0					
Spillback Cap Reductn			0					
Storage Cap Reductn			0					
Ciorage Cap Neudolii			U					

Synchro 10 Report Page 5 03/10/2022 McMahon Associates

### 3: Bow Street & Pedestrian Crossing



	_#	-	•	€_	6	1		
Lane Group	EBL	EBT	WBT	WBR	SWL	SWR	Ø2	
Lane Configurations		414	,,,,,	11211	0112	OTTI	~-	
Traffic Volume (vph)	241	397	0	0	0	0		
Future Volume (vph)	241	397	0	0	0	0		
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900		
Satd. Flow (prot)	0	3498	0	0	0	0		
Flt Permitted	U	0.981	U	U	U	U		
Satd. Flow (perm)	0	3498	0	0	0	0		
Right Turn on Red	Yes	3490	U	Yes	U	Yes		
Satd. Flow (RTOR)	165	245		165		165		
		30	30		30			
Link Speed (mph)			98		71			
Link Distance (ft)		240						
Travel Time (s)	0.00	5.5	2.2	0.00	1.6	0.00		
Peak Hour Factor	0.86	0.86	0.92	0.92	0.92	0.92		
Heavy Vehicles (%)	0%	2%	2%	2%	2%	2%		
Shared Lane Traffic (%)		7.10	^			•		
Lane Group Flow (vph)	0	742	0	0	0	0		
Turn Type	Split	NA						
Protected Phases	1	1					2	
Permitted Phases								
Detector Phase	1	1						
Switch Phase								
Minimum Initial (s)	10.0	10.0					5.0	
Minimum Split (s)	15.0	15.0					32.0	
Total Split (s)	48.0	48.0					32.0	
Total Split (%)	60.0%	60.0%					40%	
Yellow Time (s)	3.0	3.0					2.0	
All-Red Time (s)	2.0	2.0					1.0	
Lost Time Adjust (s)		0.0						
Total Lost Time (s)		5.0						
Lead/Lag	Lead	Lead					Lag	
Lead-Lag Optimize?								
Recall Mode	Min	Min					None	
Act Effct Green (s)		38.6						
Actuated g/C Ratio		0.69						
v/c Ratio		0.30						
Control Delay		6.2						
Queue Delay		0.0						
Total Delay		6.2						
LOS		Α						
Approach Delay		6.2						
Approach LOS		Α						
Queue Length 50th (ft)		65						
Queue Length 95th (ft)		93						
Internal Link Dist (ft)		160	18		1			
Turn Bay Length (ft)								
Base Capacity (vph)		2704						
Starvation Cap Reductn		0						
Spillback Cap Reductn		0						
Storage Cap Reductn		0						
Otorage Cap Neductii		U						

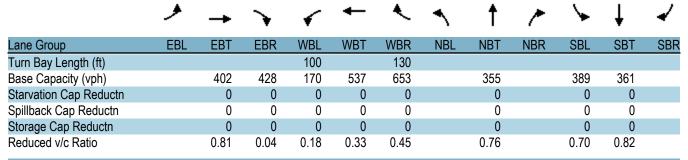


	-	•	•	•	•	~	
Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	<b>^</b>					7	
Traffic Volume (veh/h)	561	0	0	0	0	77	
Future Volume (Veh/h)	561	0	0	0	0	77	
Sign Control	Free			Free	Stop		
Grade	0%			0%	0%		
Peak Hour Factor	0.84	0.84	0.92	0.92	0.82	0.82	
Hourly flow rate (vph)	668	0	0	0	0	94	
Pedestrians							
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type	None			None			
Median storage veh)							
Upstream signal (ft)				240			
pX, platoon unblocked							
vC, conflicting volume			668		668	334	
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol			668		668	334	
tC, single (s)			4.1		6.8	6.9	
tC, 2 stage (s)							
tF (s)			2.2		3.5	3.3	
p0 queue free %			100		100	86	
cM capacity (veh/h)			918		391	665	
Direction, Lane #	EB 1	EB 2	NB 1				
Volume Total	334	334	94				
Volume Left	0	0	0				
Volume Right	0	0	94				
cSH	1700	1700	665				
Volume to Capacity	0.20	0.20	0.14				
Queue Length 95th (ft)	0.20	0.20	12				
Control Delay (s)	0.0	0.0	11.3				
Lane LOS	0.0	0.0	В				
Approach Delay (s)	0.0		11.3				
Approach LOS	0.0		Н.5				
• •			Ь				
Intersection Summary							
Average Delay			1.4				
Intersection Capacity Utiliz	zation		26.9%	IC	U Level c	of Service	)
Analysis Period (min)			15				

	۶	<b>→</b>	•	•	<b>+</b>	•	•	<b>†</b>	~	<b>/</b>	<b></b>	✓
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4	7	ሻ	<b>†</b>	7		4		*	f)	
Traffic Volume (vph)	65	212	15	28	165	276	25	202	22	249	206	63
Future Volume (vph)	65	212	15	28	165	276	25	202	22	249	206	63
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	100		130	0		0	0		0
Storage Lanes	0		1	1		1	0		0	1		0
Taper Length (ft)	25			25			25			25		
Satd. Flow (prot)	0	1821	1615	1736	1827	1568	0	1818	0	1752	1628	0
Flt Permitted		0.763		0.330				0.995		0.950		
Satd. Flow (perm)	0	1366	1454	578	1827	1264	0	1818	0	1752	1628	0
Right Turn on Red			No			No			Yes			No
Satd. Flow (RTOR)								2				
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		580			678			611			159	
Travel Time (s)		13.2			15.4			13.9			3.6	
Confl. Peds. (#/hr)	100		48	48		183			83			195
Confl. Bikes (#/hr)									16			31
Peak Hour Factor	0.85	0.85	0.85	0.94	0.94	0.94	0.92	0.92	0.92	0.91	0.91	0.91
Heavy Vehicles (%)	0%	4%	0%	4%	4%	3%	0%	0%	5%	3%	0%	0%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	325	18	30	176	294	0	271	0	274	295	0
Turn Type	Perm	NA	Perm	Perm	NA	custom	Split	NA		Split	NA	
Protected Phases		4			8		6	6		2	2	
Permitted Phases	4		4	8		28						
Detector Phase	4	4	4	8	8	28	6	6		2	2	
Switch Phase												
Minimum Initial (s)	10.0	10.0	10.0	10.0	10.0		10.0	10.0		10.0	10.0	
Minimum Split (s)	16.0	16.0	16.0	16.0	16.0		16.0	16.0		16.0	16.0	
Total Split (s)	59.0	59.0	59.0	59.0	59.0		41.0	41.0		46.0	46.0	
Total Split (%)	32.8%	32.8%	32.8%	32.8%	32.8%		22.8%	22.8%		25.6%	25.6%	
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0		3.0	3.0		3.0	3.0	
All-Red Time (s)	3.0	3.0	3.0	3.0	3.0		3.0	3.0		3.0	3.0	
Lost Time Adjust (s)		0.0	0.0	0.0	0.0			0.0		0.0	0.0	
Total Lost Time (s)		6.0	6.0	6.0	6.0			6.0		6.0	6.0	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	C-Max	C-Max		C-Max	C-Max		Max	Max		Max	Max	
Act Effct Green (s)		53.0	53.0	53.0	53.0	93.0		35.0		40.0	40.0	
Actuated g/C Ratio		0.29	0.29	0.29	0.29	0.52		0.19		0.22	0.22	
v/c Ratio		0.81	0.04	0.18	0.33	0.45		0.76		0.70	0.82	
Control Delay		75.3	45.9	50.8	51.7	16.3		83.0		75.6	84.9	
Queue Delay		0.0	0.0	0.0	0.0	0.0		0.0		0.0	0.0	
Total Delay		75.3	45.9	50.8	51.7	16.3		83.0		75.6	84.9	
LOS		Е	D	D	D	В		F		Е	F	
Approach Delay		73.8			30.8			83.0			80.4	
Approach LOS		Е			С			F			F	
Queue Length 50th (ft)		358	15	27	164	109		305		302	335	
Queue Length 95th (ft)		457	36	59	240	154		421		416	#484	
Internal Link Dist (ft)		500			598			531			79	

03/10/2022 McMahon Associates Synchro 10 Report Page 1

1 0	<b>CO</b>	
Lane Group	Ø9	
Lane Configurations		
Traffic Volume (vph)		
Future Volume (vph)		
Ideal Flow (vphpl)		
Storage Length (ft)		
Storage Lanes		
Taper Length (ft)		
Satd. Flow (prot)		
Flt Permitted		
Satd. Flow (perm)		
Right Turn on Red		
Satd. Flow (RTOR)		
Link Speed (mph)		
Link Distance (ft)		
Travel Time (s)		
Confl. Peds. (#/hr)		
Confl. Bikes (#/hr)		
Peak Hour Factor		
Heavy Vehicles (%)		
Shared Lane Traffic (%)		
Lane Group Flow (vph)		
Turn Type		
Protected Phases	9	
Permitted Phases	<u> </u>	
Detector Phase		
Switch Phase		
Minimum Initial (s)	9.0	
Minimum Split (s)	34.0	
Total Split (s)	34.0	
	19%	
Total Split (%)		
Yellow Time (s)	2.0	
All-Red Time (s)	3.0	
Lost Time Adjust (s)		
Total Lost Time (s)		
Lead/Lag		
Lead-Lag Optimize?		
Recall Mode	Max	
Act Effct Green (s)		
Actuated g/C Ratio		
v/c Ratio		
Control Delay		
Queue Delay		
Total Delay		
LOS		
Approach Delay		
Approach LOS		
Queue Length 50th (ft)		
Queue Length 95th (ft)		
Internal Link Dist (ft)		



#### Intersection Summary

Area Type: Other

Cycle Length: 180 Actuated Cycle Length: 180

Offset: 0 (0%), Referenced to phase 4:EBTL and 8:WBTL, Start of Green

Natural Cycle: 115

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.82 Intersection Signal Delay: 64.8 Intersection Capacity Utilization 73.7%

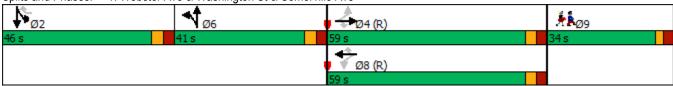
Intersection LOS: E
ICU Level of Service D

Analysis Period (min) 15

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 1: Webster Ave & Washington St & Somerville Ave



Lane Group	Ø9
Turn Bay Length (ft)	
Base Capacity (vph)	
Starvation Cap Reductn	
Spillback Cap Reductn	
Storage Cap Reductn	
Reduced v/c Ratio	
Intersection Summary	

	۶	<b>→</b>	•	•	<b>←</b>	4	1	<b>†</b>	<i>&gt;</i>	<b>/</b>	<b>+</b>	✓
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations					f)			ર્ન				7
Traffic Volume (veh/h)	0	0	0	0	511	32	202	43	0	0	0	101
Future Volume (Veh/h)	0	0	0	0	511	32	202	43	0	0	0	101
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.94	0.94	0.94	0.94	0.94	0.94	0.84	0.84	0.84
Hourly flow rate (vph)	0	0	0	0	544	34	215	46	0	0	0	120
Pedestrians											212	
Lane Width (ft)											12.0	
Walking Speed (ft/s)											3.5	
Percent Blockage											20	
Right turn flare (veh)												
Median type		None			None							
Median storage veh)												
Upstream signal (ft)					50							
pX, platoon unblocked	0.85						0.85	0.85	•	0.85	0.85	0.85
vC, conflicting volume	790			0			681	790	0	796	773	773
vC1, stage 1 conf vol												
vC2, stage 2 conf vol	004			^			<b>500</b>	004	0	000	044	044
vCu, unblocked vol	661			0			532	661	0	668	641	641
tC, single (s)	4.1			4.1			*5.5	*5.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)	2.2			2.2			2.5	4.0	2.2	2.5	4.0	2.2
tF (s)	100			100			3.5 18	4.0 85	3.3 100	3.5 100	4.0 100	3.3 63
p0 queue free %	626			1623			263	313	1085	189	265	323
cM capacity (veh/h)				1023			203	313	1000	109	205	323
Direction, Lane #	WB 1	NB 1	SB 1									
Volume Total	578	261	120									
Volume Left	0	215	0									
Volume Right	34	0	120									
cSH	1700	270	323									
Volume to Capacity	0.34	0.97	0.37 42									
Queue Length 95th (ft)	0.0	233										
Control Delay (s)	0.0	87.2 F	22.6 C									
Lane LOS Approach Delay (s)	0.0	87.2	22.6									
Approach LOS	0.0	07.Z	22.0 C									
		Г	C									
Intersection Summary												
Average Delay			26.6									
Intersection Capacity Utiliza	ition		59.0%	IC	U Level o	of Service			В			
Analysis Period (min)			15									
* User Entered Value												

## Liberty Cannabis 3: Bow Street & Pedestrian Crossing

	•	•	<b>†</b>	<b>/</b>	<b>/</b>	ţ	
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT	Ø2
Lane Configurations			<b>^</b>				
Traffic Volume (vph)	0	0	543	0	0	0	
Future Volume (vph)	0	0	543	0	0	0	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Satd. Flow (prot)	0	0	1863	0	0	0	
Flt Permitted	•						
Satd. Flow (perm)	0	0	1863	0	0	0	
Right Turn on Red		No		No			
Satd. Flow (RTOR)							
Link Speed (mph)	30		30			30	
Link Distance (ft)	184		50			50	
Travel Time (s)	4.2		1.1			1.1	
Peak Hour Factor	0.92	0.92	0.94	0.94	0.92	0.92	
Shared Lane Traffic (%)	0.52	0.52	0.54	0.54	0.52	0.52	
Lane Group Flow (vph)	0	0	578	0	0	0	
Turn Type	U	U	NA	U	U	U	
Protected Phases			1				2
Permitted Phases			Į.				۷
Detector Phases			1				
			ı				
Switch Phase			05.0				40.0
Minimum Initial (s)			25.0				10.0
Minimum Split (s)			29.0				16.0
Total Split (s)			29.0				16.0
Total Split (%)			64.4%				36%
Yellow Time (s)			3.0				2.0
All-Red Time (s)			1.0				1.0
Lost Time Adjust (s)			0.0				
Total Lost Time (s)			4.0				
Lead/Lag			Lead				Lag
Lead-Lag Optimize?							
Recall Mode			Min				None
Act Effct Green (s)			40.0				
Actuated g/C Ratio			0.67				
v/c Ratio			0.47				
Control Delay			6.3				
Queue Delay			0.0				
Total Delay			6.3				
LOS			Α				
Approach Delay			6.3				
Approach LOS			Α				
Queue Length 50th (ft)			82				
Queue Length 95th (ft)			136				
Internal Link Dist (ft)	104		1			1	
Turn Bay Length (ft)							
Base Capacity (vph)			1242				
Starvation Cap Reductn			0				
Spillback Cap Reductn			0				
Storage Cap Reductn			0				
Reduced v/c Ratio			0.47				

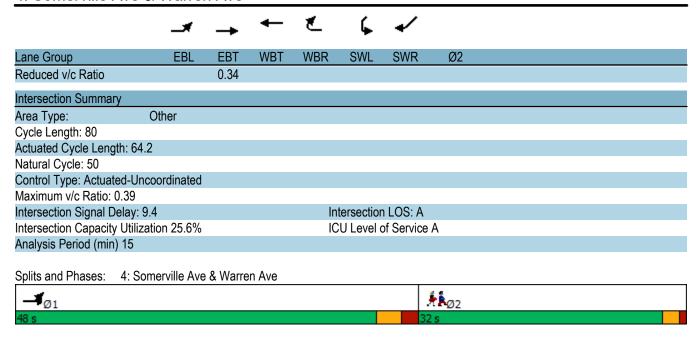
Synchro 10 Report 03/10/2022 Page 5 McMahon Associates

## Liberty Cannabis 3: Bow Street & Pedestrian Crossing

Intersection Summary		
Area Type: Other		
Cycle Length: 45		
Actuated Cycle Length: 60		
Natural Cycle: 45		
Control Type: Actuated-Uncoordinated		
Maximum v/c Ratio: 0.47		
Intersection Signal Delay: 6.3	Intersection LOS: A	
Intersection Capacity Utilization 31.9%	ICU Level of Service A	
Analysis Period (min) 15		
Splits and Phases: 3: Bow Street & Pedestrian Crossing		
A		

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	_#	-	<b>←</b>	۲	Ĺ	✓		
Lane Group	EBL	EBT	WBT	WBR	SWL	SWR	Ø2	
Lane Configurations		414						
Traffic Volume (vph)	245	518	0	0	0	0		
Future Volume (vph)	245	518	0	0	0	0		
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900		
Satd. Flow (prot)	0	3528	0	0	0	0		
Flt Permitted		0.984						
Satd. Flow (perm)	0	3528	0	0	0	0		
Right Turn on Red	Yes	00_0	•	Yes	•	Yes		
Satd. Flow (RTOR)		152						
Link Speed (mph)		30	30		30			
Link Distance (ft)		240	98		71			
Travel Time (s)		5.5	2.2		1.6			
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92		
Heavy Vehicles (%)	0.52	1%	2%	2%	2%	2%		
Shared Lane Traffic (%)	0 /0	1 /0	270	<u>_</u> /0	2 /0	2/0		
Lane Group Flow (vph)	0	829	0	0	0	0		
Turn Type	Split	NA	U	· ·	· ·	· ·		
Protected Phases	1	1					2	
Permitted Phases	•	•					2	
Detector Phase	1	1						
Switch Phase								
Minimum Initial (s)	10.0	10.0					5.0	
Minimum Split (s)	15.0	15.0					32.0	
Total Split (s)	48.0	48.0					32.0	
Total Split (%)	60.0%	60.0%					40%	
Yellow Time (s)	3.0	3.0					2.0	
All-Red Time (s)	2.0	2.0					1.0	
Lost Time Adjust (s)	2.0	0.0					1.0	
Total Lost Time (s)		5.0						
Lead/Lag	Lead	Lead					Lag	
Lead-Lag Optimize?	Leau	Leau					Lag	
Recall Mode	Min	Min					None	
Act Effct Green (s)	IVIIII	37.8					NOTIC	
Actuated g/C Ratio		0.59						
v/c Ratio		0.39						
Control Delay		9.4						
Queue Delay		0.0						
Total Delay		9.4						
LOS		9.4 A						
Approach Delay		9.4						
Approach LOS		A 95						
Queue Length 50th (ft)		137						
Queue Length 95th (ft)		160	18		1			
Internal Link Dist (ft)		100	10					
Turn Bay Length (ft)		2440						
Base Capacity (vph)								
Starvation Cap Reducts		0						
Spillback Cap Reductn		0						
Storage Cap Reductn		0						



	-	•	•	<b>←</b>	•	<b>/</b>
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>^</b>					7
Traffic Volume (veh/h)	718	0	0	0	0	47
Future Volume (Veh/h)	718	0	0	0	0	47
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.93	0.93	0.92	0.92	0.81	0.81
Hourly flow rate (vph)	772	0	0	0	0	58
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None			None		
Median storage veh)						
Upstream signal (ft)				240		
pX, platoon unblocked						
vC, conflicting volume			772		772	386
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			772		772	386
tC, single (s)			4.1		6.8	6.9
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.3
p0 queue free %			100		100	91
cM capacity (veh/h)			839		336	618
Direction, Lane #	EB 1	EB 2	NB 1			
Volume Total	386	386	58			
Volume Left	0	0	0			
Volume Right	0	0	58			
cSH	1700	1700	618			
Volume to Capacity	0.23	0.23	0.09			
Queue Length 95th (ft)	0	0	8			
Control Delay (s)	0.0	0.0	11.4			
Lane LOS			В			
Approach Delay (s)	0.0		11.4			
Approach LOS			В			
Intersection Summary						
Average Delay			0.8			
Intersection Capacity Utilizat	ion		29.8%	IC	U Level c	f Service
Analysis Period (min)			15			

#### **APPENDIX H**

Capacity/Level-of-Service Analysis Summary

### **CAPACITY ANALYSIS SUMMARY**

Weekday Morning Peak Hour Liberty Cannabis Somerville, MA

			2022 Existing		2	022 Bui	ld	2027 Build			
Intersection	Mover	nent	LOS <sup>1</sup>	Delay <sup>2</sup>	V/C <sup>3</sup>	LOS	Delay	V/C	LOS	Delay	V/C
Somerville Avenue at	EB	LT	Е	74.0	0.78	E	74.0	0.78	F	84.7	0.86
Bow Street/Webster Avenue and		R	D	47.9	0.13	D	47.9	0.13	D	49.3	0.13
Washington Street	WB	L	D	53.7	0.27	D	53.7	0.27	Ε	56.3	0.29
		Т	Ε	56.1	0.48	Ε	56.1	0.48	Ε	58.3	0.51
		R	Ε	57.5	0.48	Ε	57.5	0.48	В	12.1	0.28
	NB	LTR	Ε	65.5	0.56	Ε	65.5	0.56	F	89.3	0.79
	SB	L	Ε	57.4	0.50	Ε	57.4	0.50	Ε	63.2	0.56
		TR	Ε	72.8	0.81	Ε	72.8	0.81	F	86.5	0.90
	Ove	erall	Ε	64.3	0.99	Ε	64.3	0.99	Ε	68.4	0.76
Bow Street at	WB	TR	Α	4.1	0.30	Α	4.1	0.30	Α	4.1	0.30
Warren Avenue	NB	LT	С	17.8	0.45	С	17.8	0.45	С	18.2	0.46
	SB	R	В	12.2	0.15	В	12.2	0.15	В	12.3	0.15
Somerville Avenue at Warren Avenue	ЕВ	LT	Α	9.7	0.40	Α	9.7	0.40	Α	9.7	0.40
Somerville Avenue at	EB	Т	Α	0.0	0.22	Α	0.0	0.22	Α	0.0	0.22
Hawkins Street	NB	R	В	11.9	0.16	В	11.9	0.16	В	12.0	0.16

<sup>1</sup> Level-of-Service

<sup>2</sup> Average vehicle delay in seconds

<sup>3</sup> Volume to capacity ratio; intersection capacity utilization reported for overall

### **QUEUE SUMMARY**

Weekday Morning Peak Hour Liberty Cannabis Somerville, MA

			2022 E	xisting	2022	Build	2027	Build	
Intersection	Movem	nent	50th Queue <sup>1</sup>	95th Queue <sup>2</sup>	50th Queue	95th Queue	50th Queue	95th Queue	
Somerville Avenue at	EB	LT	310	432	310	432	327	489	
Bow Street/Webster Avenue and		R	43	80	43	80	45	85	
Washington Street	WB	L	44	88	44	88	46	91	
		Т	240	335	240	335	247	346	
		R	206	297	206	297	62	90	
	NB	LTR	239	280	239	280	265	310	
	SB	L	243	321	243	321	256	338	
		TR	453	561	453	561	478	621	
Bow Street at	WB	TR	54	92	54	92	55	93	
Warren Avenue	NB	LT	n/a	57	n/a	57	n/a	60	
	SB	R	n/a	13	n/a	13	n/a	13	
Somerville Avenue at Warren Avenue	ЕВ	LT	110	152	110	152	112	154	
Somerville Avenue at	EB	Т	n/a	0	n/a	0	n/a	0	
Hawkins Street	NB	R	n/a	14	n/a	14	n/a	14	

<sup>1 50</sup>th percentile queue, in feet

<sup>2 95</sup>th percentile queue, in feet

n/a Not applicable

### **CAPACITY ANALYSIS SUMMARY**

### Weekday Afternoon Peak Hour Liberty Cannabis Somerville, MA

			2022 Existing		2	022 Bui	ld	2027 Build			
Intersection	Mover	nent	LOS <sup>1</sup>	Delay <sup>2</sup>	V/C <sup>3</sup>	LOS	Delay	V/C	LOS	Delay	V/C
Somerville Avenue at	EB	LT	F	93.6	0.92	F	93.6	0.92	E	72.7	0.81
Bow Street/Webster Avenue and		R	D	47.0	0.09	D	47.0	0.09	D	43.1	0.08
Washington Street	WB	L	D	50.7	0.17	D	50.7	0.17	D	46.0	0.14
		Т	D	54.1	0.42	D	54.1	0.42	D	49.7	0.39
		R	Ε	60.6	0.58	Ε	60.7	0.58	В	16.8	0.39
	NB	LTR	F	81.7	0.82	F	82.0	0.83	F	89.3	0.88
	SB	L	D	52.7	0.34	D	52.8	0.34	Ε	78.7	0.60
		TR	Ε	55.8	0.45	Ε	56.0	0.46	F	92.5	0.80
	Ove	erall	Ε	68.6	0.98	Ε	68.7	0.98	Ε	66.5	0.79
Bow Street at	WB	TR	Α	5.5	0.47	Α	5.6	0.47	Α	5.6	0.48
Warren Avenue	NB	LT	F	62.0	0.90	F	65.9	0.92	F	72.8	0.95
	SB	R	С	16.7	0.21	С	16.8	0.21	С	17.0	0.21
Somerville Avenue at Warren Avenue	EB	LT	Α	6.2	0.29	Α	6.2	0.29	Α	6.2	0.30
Somerville Avenue at	EB	Т	Α	0.0	0.19	Α	0.0	0.19	Α	0.0	0.20
Hawkins Street	NB	R	В	11.2	0.13	В	11.3	0.14	В	11.3	0.14

<sup>1</sup> Level-of-Service

<sup>2</sup> Average vehicle delay in seconds

<sup>3</sup> Volume to capacity ratio; intersection capacity utilization reported for overall

### **QUEUE SUMMARY**

Weekday Afternoon Peak Hour Liberty Cannabis Somerville, MA

			2022 E	xisting	2022	Build	2027	Build
Intersection	Movem	ent	50th Queue <sup>1</sup>	95th Queue <sup>2</sup>	50th Queue	95th Queue	50th Queue	95th Queue
Somerville Avenue at	EB	LT	362	562	362	562	346	506
Bow Street/Webster Avenue and		R	32	66	32	66	31	63
Washington Street	WB	L	25	57	25	57	24	54
		T	222	311	222	311	216	302
		R	264	369	266	372	105	148
	NB	LTR	403	562	405	562	416	598
	SB	L	161	236	164	240	195	287
		TR	226	318	228	321	275	411
Bow Street at	WB	TR	105	178	106	180	109	184
Warren Avenue	NB	LT	n/a	225	n/a	236	n/a	252
	SB	R	n/a	19	n/a	19	n/a	20
Somerville Avenue at Warren Avenue	ЕВ	LT	63	90	64	91	65	93
Somerville Avenue at	EB	Т	n/a	0	n/a	0	n/a	0
Hawkins Street	NB	R	n/a	12	n/a	12	n/a	12

<sup>1 50</sup>th percentile queue, in feet

<sup>2 95</sup>th percentile queue, in feet

n/a Not applicable

### **CAPACITY ANALYSIS SUMMARY**

### Saturday Midday Peak Hour Liberty Cannabis Somerville, MA

			2022 Existing			2	022 Bui	ld	2027 Build		
Intersection	Movem	nent	LOS <sup>1</sup>	Delay <sup>2</sup>	V/C <sup>3</sup>	LOS	Delay	V/C	LOS	Delay	V/C
Somerville Avenue at	EB	LT	Е	75.5	0.81	Е	75.5	0.81	Е	75.3	0.81
Bow Street/Webster Avenue and		R	D	46.1	0.05	D	46.1	0.05	D	45.9	0.04
Washington Street	WB	L	D	50.9	0.18	D	50.9	0.18	D	50.8	0.18
		Т	D	51.6	0.32	D	51.6	0.32	D	51.7	0.33
		R	Ε	63.3	0.64	Ε	63.5	0.64	В	16.3	0.45
	NB	LTR	Ε	69.0	0.63	Ε	69.1	0.63	F	83.0	0.76
	SB	L	Ε	58.3	0.53	Ε	58.4	0.53	Ε	75.6	0.70
		TR	Ε	62.7	0.63	Ε	63.0	0.63	F	84.9	0.82
	Ove	rall	Ε	64.0	0.95	Ε	64.1	0.95	Ε	64.8	0.74
Bow Street at	WB	TR	Α	6.3	0.46	Α	6.3	0.46	Α	6.3	0.47
Warren Avenue	NB	LT	F	75.6	0.92	F	79.5	0.94	F	87.2	0.97
	SB	R	С	22.1	0.36	С	22.2	0.36	С	22.6	0.37
Somerville Avenue at Warren Avenue	EB	LT	Α	9.0	0.37	Α	9.4	0.38	Α	9.4	0.39
Somerville Avenue at	EB	Т	Α	0.0	0.22	Α	0.0	0.22	Α	0.0	0.23
Hawkins Street	NB	R	В	11.3	0.09	В	11.4	0.09	В	11.4	0.09

<sup>1</sup> Level-of-Service

<sup>2</sup> Average vehicle delay in seconds

<sup>3</sup> Volume to capacity ratio; intersection capacity utilization reported for overall

### **QUEUE SUMMARY**

Saturday Midday Peak Hour Liberty Cannabis Somerville, MA

			2022 I	xisting	2022	Build	2027 Build		
Intersection	Moven	nent	50th Queue <sup>1</sup>	95th Queue <sup>2</sup>	50th Queue	95th Queue	50th Queue	95th Queue	
Somerville Avenue at	EB	LT	354	453	354	453	358	457	
Bow Street/Webster Avenue and		R	15	36	15	36	15	36	
Washington Street	WB	L	27	59	27	59	27	59	
		T	161	236	161	236	164	240	
		R	299	413	302	416	109	154	
	NB	LTR	284	392	285	393	305	421	
	SB	L	268	369	270	373	302	416	
		TR	299	413	302	414	335	484	
Bow Street at	WB	TR	80	132	80	133	82	136	
Warren Avenue	NB	LT	n/a	212	n/a	220	n/a	233	
	SB	R	n/a	40	n/a	40	n/a	42	
Somerville Avenue at Warren Avenue	ЕВ	LT	92	132	93	135	95	137	
Somerville Avenue at	EB	Т	n/a	0	n/a	0	n/a	0	
Hawkins Street	NB	R	n/a	7	n/a	8	n/a	8	

<sup>1 50</sup>th percentile queue, in feet

<sup>2 95</sup>th percentile queue, in feet

n/a Not applicable